DA Form 1 – Development application details

Approved form (version 1.3 effective 28 September 2020) made under section 282 of the Planning Act 2016.

This form **must** be used to make a development application **involving code assessment or impact assessment**, except when applying for development involving only building work.

For a development application involving building work only, use DA Form 2 – Building work details.

For a development application involving building work associated with any other type of assessable development (i.e. material change of use, operational work or reconfiguring a lot), use this form (*DA Form 1*) and parts 4 to 6 of *DA Form 2 – Building work details.*

Unless stated otherwise, all parts of this form **must** be completed in full and all required supporting information **must** accompany the development application.

One or more additional pages may be attached as a schedule to this development application if there is insufficient space on the form to include all the necessary information.

This form and any other form relevant to the development application must be used to make a development application relating to strategic port land and Brisbane core port land under the *Transport Infrastructure Act 1994*, and airport land under the *Airport Assets (Restructuring and Disposal) Act 2008.* For the purpose of assessing a development application relating to strategic port land and Brisbane core port land, any reference to a planning scheme is taken to mean a land use plan for the strategic port land, Brisbane port land use plan for Brisbane core port land, or a land use plan for airport land.

Note: All terms used in this form have the meaning given under the Planning Act 2016, the Planning Regulation 2017, or the Development Assessment Rules (DA Rules).

1) Applicant details	
Applicant name(s) (individual or company full name)	Debell Property Pty Ltd as trustee for Debell Property Trust
Contact name (only applicable for companies)	Brendan Ferris
Postal address (P.O. Box or street address)	C/- Town Planning Alliance PO Box 7657
Suburb	East Brisbane
State	Qld
Postcode	4169
Country	Australia
Contact number	07 3361 9999
Email address (non-mandatory)	eda@tpalliance.com.au
Mobile number (non-mandatory)	
Fax number (non-mandatory)	
Applicant's reference number(s) (if applicable)	22-0117

PART 1 – APPLICANT DETAILS

2) Owner's consent

2.1) Is written consent of the owner required for this development application?

Yes – the written consent of the owner(s) is attached to this development application

 \boxtimes No – proceed to 3)



PART 2 – LOCATION DETAILS

 Location of the premises (complete 3.1) or 3.2), and 3.3) as applicable) Note: Provide details below and attach a site plan for any or all premises part of the development application. For further information, see <u>DA</u> Forms Guide: Relevant plans. 							
3.1) Street address and lot on plan							
Str	eet address	AND lo	ot on plan (a	Il lots must be liste	ed), or		
				or an adjoining g. jetty, pontoon. A			e premises (appropriate for development in
Unit No. Street No.				eet Name and	Туре		Suburb
a)	-	12990	C Mo	ssman Daintre	e Road	k	Rocky Point
α)	Postcode	Lot No	o. Pla	an Type and Nu	umber	(e.g. RP, SP)	Local Government Area(s)
	4874	4	SP	SP238245			Douglas Shire Council
	Unit No.	Street	No. Str	eet Name and	Туре		Suburb
b)	-	1299A	Mo	ssman Daintre	e Road	ł	Rocky Point
0)	Postcode	Lot No	o. Pla	an Type and Nu	umber	(e.g. RP, SP)	Local Government Area(s)
	4874	2	SP	238245			Douglas Shire Council
е.	oordinates o g. channel drec lace each set o	lging in N	loreton Bay)		ent in rer	note areas, over part of	a lot or in water not adjoining or adjacent to land
	ordinates of	premis	es by longit	ude and latitud	le		
Longit	ude(s)		Latitude(s)	Datu	m	Local Government Area(s) (if applicable)
						GS84	
						DA94	_
	ordinates of	premis	es by eastir	ng and northing		ther:	
Eastin		North	•	Zone Ref.	Datu	m	Local Government Area(s) (if applicable)
			0()	54		GS84	
				55	G	DA94	
				56	0	ther:	
3.3) A	dditional pre	mises					
atta				to this develop relopment appl		oplication and the c	letails of these premises have been
4) Ider	ntify any of t	ne follo	wing that a	only to the prer	nises a	nd provide any rele	evant details
						bove an aquifer	
	of water boo		•				
		•		Transport Infras	structur	ο Δct 1994	
	• •				structur		
	Lot on plan description of strategic port land: Name of port authority for the lot:						
	-						
	In a tidal area						
	Name of local government for the tidal area <i>(if applicable)</i> :						
Name of port authority for tidal area (if applicable): On airport land under the Airport Assets (Restructuring and Disposal) Act 2008							
		under		กออยเอ (กษรแน	ciuring		2000
name	Name of airport:						

Listed on the Environmental Management Register (EMR) under the Environmental Protection Act 1994			
EMR site identification:			
Listed on the Contaminated Land Register (CLR) under the Environmental Protection Act 1994			
CLR site identification:			

5) Are there any existing easements over the premises?

Note: Easement uses vary throughout Queensland and are to be identified correctly and accurately. For further information on easements and how they may affect the proposed development, see <u>DA Forms Guide</u>.

Yes – All easement locations, types and dimensions are included in plans submitted with this development application

🗌 No

PART 3 – DEVELOPMENT DETAILS

Section 1 – Aspects of development

6.1) Provide details about the first development aspect
a) What is the type of development? (tick only one box)
Material change of use Reconfiguring a lot Operational work Building work
b) What is the approval type? (tick only one box)
Development permit Preliminary approval Preliminary approval that includes a variation approva
c) What is the level of assessment?
Code assessment Impact assessment (requires public notification)
d) Provide a brief description of the proposal (e.g. 6 unit apartment building defined as multi-unit dwelling, reconfiguration of 1 lot into 3 lots):
Dwelling house
e) Relevant plans Note : Relevant plans are required to be submitted for all aspects of this development application. For further information, see <u>DA Forms guide:</u> <u>Relevant plans</u> .
\boxtimes Relevant plans of the proposed development are attached to the development application
6.2) Provide details about the second development aspect
a) What is the type of development? (tick only one box)
Material change of use Reconfiguring a lot Operational work Building work
b) What is the approval type? (tick only one box)
Development permit Preliminary approval Preliminary approval that includes a variation approva
c) What is the level of assessment?
Code assessment Impact assessment (requires public notification)
d) Provide a brief description of the proposal (e.g. 6 unit apartment building defined as multi-unit dwelling, reconfiguration of 1 lot into s lots):
e) Relevant plans Note: Relevant plans are required to be submitted for all aspects of this development application. For further information, see <u>DA Forms Guide:</u> <u>Relevant plans.</u>
Relevant plans of the proposed development are attached to the development application
6.3) Additional aspects of development
Additional aspects of development are relevant to this development application and the details for these aspects that would be required under Part 3 Section 1 of this form have been attached to this development application
Not required

Section 2 – Further development details

7) Does the proposed development application involve any of the following?		
Material change of use	$oxed{i}$ Yes – complete division 1 if assessable against a local planning instrument	
Reconfiguring a lot	Yes – complete division 2	
Operational work	Yes – complete division 3	
Building work	Yes – complete DA Form 2 – Building work details	

Division 1 – Material change of use

Note: This division is only required to be completed if any part of the development application involves a material change of use assessable against a local planning instrument.

8.1) Describe the proposed material change of use				
Provide a general description of the proposed use	Provide the planning scheme definition (include each definition in a new row)	Number of dwelling units (if applicable)	Gross floor area (m²) <i>(if applicable)</i> n/a	
Dwelling	Dwelling house	n/a		
8.2) Does the proposed use involve the use of existing buildings on the premises?				
☐ Yes				
🛛 No				

Division 2 – Reconfiguring a lot

Note: This division is only required to be completed if any part of the development application involves reconfiguring a lot.

9.1) What is the total number of existing lots making up the premises?

9.2) What is the nature of the lot reconfiguration? (tick all applicable boxes)				
Subdivision (complete 10))	Dividing land into parts by agreement (complete 11))			
Boundary realignment (complete 12))	Creating or changing an easement giving access to a lot from a constructed road (complete 13))			

10) Subdivision				
10.1) For this development, how many lots are being created and what is the intended use of those lots:				
Intended use of lots created	Commercial	Industrial	Other, please specify:	
Number of lots created				
10.2) Will the subdivision be stag	ged?			
Yes – provide additional deta	ils below			
□ No				
How many stages will the works				
What stage(s) will this developm apply to?				

11) Dividing land into parts by agreement – how many parts are being created and what is the intended use of the parts?				
Intended use of parts created	Residential	Commercial	Industrial	Other, please specify:
Number of parts created				

12) Boundary realignment				
12.1) What are the current a	nd proposed areas for each lo	t comprising the premises?		
Curre	ent lot	Proposed lot		
Lot on plan description Area (m ²)		Lot on plan description	Area (m ²)	
12.2) What is the reason for the boundary realignment?				

13) What are the dimensions and nature of any existing easements being changed and/or any proposed easement? (attach schedule if there are more than two easements)				
Existing or proposed?	Width (m)	Length (m)	Purpose of the easement? (e.g. pedestrian access)	Identify the land/lot(s) benefitted by the easement

Division 3 – Operational work

Note: This division is only required to be completed if any part of the development application involves operational work.

	14.1) What is the nature of the operational work?				
	Road work	Stormwater	Water infrastructure		
	Drainage work	Earthworks	Sewage infrastructure		
ļ	Landscaping	Signage	Clearing vegetation		
	Other – please specify:				
	14.2) Is the operational work nec	essary to facilitate the creation of	new lots? (e.g. subdivision)		
	Yes – specify number of new	lots:			
	🗌 No				
	14.3) What is the monetary value	e of the proposed operational work	(include GST, materials and labour)		
	\$				

PART 4 – ASSESSMENT MANAGER DETAILS

15) Identify the assessment manager(s) who will be assessing this development application
Douglas Shire Council
16) Has the local government agreed to apply a superseded planning scheme for this development application?
Yes – a copy of the decision notice is attached to this development application
The local government is taken to have agreed to the superseded planning scheme request – relevant documents
attached
No No

PART 5 – REFERRAL DETAILS

17) Does this development application include any aspects that have any referral requirements? Note: A development application will require referral if prescribed by the Planning Regulation 2017. No, there are no referral requirements relevant to any development aspects identified in this development application - proceed to Part 6 Matters requiring referral to the Chief Executive of the Planning Act 2016: Clearing native vegetation Contaminated land (unexploded ordnance) Environmentally relevant activities (ERA) (only if the ERA has not been devolved to a local government) Fisheries – aquaculture Fisheries – declared fish habitat area Fisheries – marine plants Fisheries – waterway barrier works Hazardous chemical facilities Heritage places – Queensland heritage place (on or near a Queensland heritage place) Infrastructure-related referrals – designated premises Infrastructure-related referrals – state transport infrastructure Infrastructure-related referrals – State transport corridor and future State transport corridor Infrastructure-related referrals – State-controlled transport tunnels and future state-controlled transport tunnels Infrastructure-related referrals – near a state-controlled road intersection Koala habitat in SEQ region – interfering with koala habitat in koala habitat areas outside koala priority areas Koala habitat in SEQ region – key resource areas Ports – Brisbane core port land – near a State transport corridor or future State transport corridor Ports – Brisbane core port land – environmentally relevant activity (ERA) Ports – Brisbane core port land – tidal works or work in a coastal management district Ports – Brisbane core port land – hazardous chemical facility Ports – Brisbane core port land – taking or interfering with water Ports – Brisbane core port land – referable dams Ports – Brisbane core port land – fisheries Ports – Land within Port of Brisbane's port limits (below high-water mark) SEQ development area SEQ regional landscape and rural production area or SEQ rural living area – tourist activity or sport and recreation activity SEQ regional landscape and rural production area or SEQ rural living area – community activity SEQ regional landscape and rural production area or SEQ rural living area – indoor recreation SEQ regional landscape and rural production area or SEQ rural living area – urban activity SEQ regional landscape and rural production area or SEQ rural living area – combined use Tidal works or works in a coastal management district Reconfiguring a lot in a coastal management district or for a canal Erosion prone area in a coastal management district Urban design Water-related development – taking or interfering with water Water-related development – removing quarry material (from a watercourse or lake) Water-related development – referable dams Water-related development –levees (category 3 levees only) Wetland protection area Matters requiring referral to the local government: Airport land Environmentally relevant activities (ERA) (only if the ERA has been devolved to local government)

Heritage places – Local heritage places

Matters requiring referral to the Chief Executive of the distribution entity or transmission entity:

Infrastructure-related referrals – Electricity infrastructure

Matters requiring referral to:

- The Chief Executive of the holder of the licence, if not an individual
- The holder of the licence, if the holder of the licence is an individual

Infrastructure-related referrals - Oil and gas infrastructure

Matters requiring referral to the Brisbane City Council:

Ports – Brisbane core port land

Matters requiring referral to the Minister responsible for administering the Transport Infrastructure Act 1994:

Ports – Brisbane core port land (where inconsistent with the Brisbane port LUP for transport reasons)

Ports – Strategic port land

Matters requiring referral to the relevant port operator, if applicant is not port operator:

Ports - Land within Port of Brisbane's port limits (below high-water mark)

Matters requiring referral to the **Chief Executive of the relevant port authority**:

Ports - Land within limits of another port (below high-water mark)

Matters requiring referral to the **Gold Coast Waterways Authority:**

Tidal works or work in a coastal management district (in Gold Coast waters)

Matters requiring referral to the Queensland Fire and Emergency Service:

Tidal works or work in a coastal management district (involving a marina (more than six vessel berths))

18) Has any referral agency provided a referral response for this development application?

☐ Yes – referral response(s) received and listed below are attached to this development application ⊠ No

Referral requirement	Referral agency	Date of referral response
	In the second second second second second	and the second state of th

Identify and describe any changes made to the proposed development application that was the subject of the referral response and this development application, or include details in a schedule to this development application *(if applicable)*.

PART 6 – INFORMATION REQUEST

19) Information request under Part 3 of the DA Rules

I agree to receive an information request if determined necessary for this development application

I do not agree to accept an information request for this development application

Note: By not agreeing to accept an information request I, the applicant, acknowledge:

 that this development application will be assessed and decided based on the information provided when making this development application and the assessment manager and any referral agencies relevant to the development application are not obligated under the DA Rules to accept any additional information provided by the applicant for the development application unless agreed to by the relevant parties

• Part 3 of the DA Rules will still apply if the application is an application listed under section 11.3 of the DA Rules.

Further advice about information requests is contained in the <u>DA Forms Guide</u>.

PART 7 – FURTHER DETAILS

20) Are there any associated development applications or current approvals? (e.g. a preliminary approval)			
 Yes – provide details below or include details in a schedule to this development application No 			
List of approval/development application references	Reference number	Date	Assessment manager
Approval	8/37/79(2372305)	2009	Douglas Shire Council
Approval Development application			

21) Has the portable long service leave levy been paid? (only applicable to development applications involving building work or operational work)				
Yes – a copy of the receipted	Yes – a copy of the receipted QLeave form is attached to this development application			
 No – I, the applicant will provide evidence that the portable long service leave levy has been paid before the assessment manager decides the development application. I acknowledge that the assessment manager may give a development approval only if I provide evidence that the portable long service leave levy has been paid Not applicable (e.g. building and construction work is less than \$150,000 excluding GST) 				
Amount paid Date paid (dd/mm/yy) QLeave levy number (A, B or E)		QLeave levy number (A, B or E)		
\$				

22) Is this development application in response to a show cause notice or required as a result of an enforcement notice?

 \Box Yes – show cause or enforcement notice is attached \boxtimes No

23) Further legislative requirements

Environmentally relevant activities

23.1) Is this development application also taken to be an application for an environmental authority for an **Environmentally Relevant Activity (ERA)** under section 115 of the *Environmental Protection Act 1994*?

Yes – the required attachment (form ESR/2015/1791) for an application for an environmental authority accompanies this development application, and details are provided in the table below				
🖾 No				
Note: Application for an environment requires an environmental authority		ing "ESR/2015/1791" as a search tern <u>ov.au</u> for further information.	n at <u>www.qld.gov.au</u> . An ERA	
Proposed ERA number:		Proposed ERA threshold:		
Proposed ERA name:				
Multiple ERAs are applicable to this development application and the details have been attached in a schedule to this development application.				
Hazardous chemical facilities				
23.2) Is this development application for a hazardous chemical facility?				
Yes – Form 69: Notification of a facility exceeding 10% of schedule 15 threshold is attached to this development application				

🛛 No

Note: See <u>www.business.qld.gov.au</u> for further information about hazardous chemical notifications.

Clearing native vegetation
23.3) Does this development application involve clearing native vegetation that requires written confirmation that the chief executive of the <i>Vegetation Management Act 1999</i> is satisfied the clearing is for a relevant purpose under section 22A of the <i>Vegetation Management Act 1999</i> ?
 Yes – this development application includes written confirmation from the chief executive of the Vegetation Management Act 1999 (s22A determination)
 No Note: 1. Where a development application for operational work or material change of use requires a s22A determination and this is not included, the development application is prohibited development. 2. See <u>https://www.qld.gov.au/environment/land/vegetation/applying</u> for further information on how to obtain a s22A determination.
Environmental offsets
23.4) Is this development application taken to be a prescribed activity that may have a significant residual impact on a prescribed environmental matter under the <i>Environmental Offsets Act 2014</i> ?
 Yes – I acknowledge that an environmental offset must be provided for any prescribed activity assessed as having a significant residual impact on a prescribed environmental matter No
Note : The environmental offset section of the Queensland Government's website can be accessed at <u>www.qld.gov.au</u> for further information on environmental offsets.
Koala habitat in SEQ Region
23.5) Does this development application involve a material change of use, reconfiguring a lot or operational work which is assessable development under Schedule 10, Part 10 of the Planning Regulation 2017?
 Yes – the development application involves premises in the koala habitat area in the koala priority area Yes – the development application involves premises in the koala habitat area outside the koala priority area
No Note: If a koala habitat area determination has been obtained for this premises and is current over the land, it should be provided as part of this development application. See koala habitat area guidance materials at <u>www.des.qld.gov.au</u> for further information.
Water resources
23.6) Does this development application involve taking or interfering with underground water through an artesian or subartesian bore, taking or interfering with water in a watercourse, lake or spring, or taking overland flow water under the <i>Water Act 2000</i> ?
Yes – the relevant template is completed and attached to this development application and I acknowledge that a relevant authorisation or licence under the <i>Water Act 2000</i> may be required prior to commencing development
No Note: Contact the Department of Natural Resources, Mines and Energy at www.dnrme.gld.gov.au for further information.
DA templates are available from https://planning.dsdmip.gld.gov.au/. If the development application involves:
Taking or interfering with underground water through an artesian or subartesian bore: complete DA Form 1 Template 1
 Taking or interfering with water in a watercourse, lake or spring: complete DA Form1 Template 2 Taking overland flow water: complete DA Form 1 Template 3.
Waterway barrier works
23.7) Does this application involve waterway barrier works?
 Yes – the relevant template is completed and attached to this development application No
DA templates are available from https://planning.dsdmip.gld.gov.au/ . For a development application involving waterway barrier works, complete DA Form 1 Template 4.
Marine activities
23.8) Does this development application involve aquaculture, works within a declared fish habitat area or removal, disturbance or destruction of marine plants?
Yes – an associated <i>resource</i> allocation authority is attached to this development application, if required under the <i>Fisheries Act 1994</i>
No Note: See guidance materials at <u>www.daf.gld.gov.au</u> for further information.

Quarry materials from a watercourse or lake			
23.9) Does this development application involve the removal of quarry materials from a watercourse or lake under the <i>Water Act 2000?</i>			
No	 Yes – I acknowledge that a quarry material allocation notice must be obtained prior to commencing development No Note: Contact the Department of Natural Resources, Mines and Energy at <u>www.dnrme.gld.gov.au</u> and <u>www.business.gld.gov.au</u> for further 		
information.	and mesources, mines and Energy	ar <u>www.unime.qid.gov.aa</u> ana <u>www.</u>	<u>Jusiness.qid.gov.au</u> ioi iuriner
Quarry materials from land	under tidal waters		
23.10) Does this development under the <i>Coastal Protection</i>		oval of quarry materials from	m land under tidal water
 ☐ Yes – I acknowledge that a ☑ No 			o commencing development
Note: Contact the Department of Env	vironment and Science at <u>www.des.</u>	<u>qld.gov.au</u> for further information.	
Referable dams			
23.11) Does this development section 343 of the <i>Water Sup</i>	ply (Safety and Reliability) A	ct 2008 (the Water Supply Act	t)?
 Yes – the 'Notice Acceptin Supply Act is attached to the No 	g a Failure Impact Assessme his development application	ent' from the chief executive a	idministering the Water
Note: See guidance materials at www	w.dnrme.qld.gov.au for further inforr	nation.	
Tidal work or development	within a coastal manageme	ent district	
23.12) Does this development	t application involve tidal wo	ork or development in a coas	stal management district?
 Yes – the following is included with this development application: Evidence the proposal meets the code for assessable development that is prescribed tidal work (only required if application involves prescribed tidal work) A certificate of title 			
No Note: See guidance materials at www	w.des.ald.gov.au for further information	tion.	
Queensland and local herita	age places		
23.13) Does this development heritage register or on a place			
Yes – details of the heritage No Note: See guidance materials at www			Queensland heritage places
Name of the heritage place:	v.ucs.qu.gov.au for information req	Place ID:	
Brothels			
23.14) Does this development application involve a material change of use for a brothel?			
 Yes – this development application demonstrates how the proposal meets the code for a development application for a brothel under Schedule 3 of the <i>Prostitution Regulation 2014</i> No 			
Decision under section 62 of	of the Transport Infrastruct	ure Act 1994	
23.15) Does this development	t application involve new or c	changed access to a state-cor	ntrolled road?
 Yes – this application will be taken to be an application for a decision under section 62 of the <i>Transport</i> Infrastructure Act 1994 (subject to the conditions in section 75 of the <i>Transport Infrastructure Act 1994</i> being satisfied) No 			

Walkable neighbourhoods assessment benchmarks under Schedule 12A of the Planning Regulation

23.16) Does this development application involve reconfiguring a lot into 2 or more lots in certain residential zones (except rural residential zones), where at least one road is created or extended?

Schedule 12A is applicable to the development application and the assessment benchmarks contained in schedule 12A have been considered

🛛 No

Note: See guidance materials at <u>www.planning.dsdmip.qld.gov.au</u> for further information.

PART 8 – CHECKLIST AND APPLICANT DECLARATION

24) Development application checklist	
I have identified the assessment manager in question 15 and all relevant referral requirement(s) in question 17 Note : See the Planning Regulation 2017 for referral requirements	⊠ Yes
If building work is associated with the proposed development, Parts 4 to 6 of <u>DA Form 2 –</u> <u>Building work details</u> have been completed and attached to this development application	☐ Yes ⊠ Not applicable
Supporting information addressing any applicable assessment benchmarks is with the development application Note: This is a mandatory requirement and includes any relevant templates under question 23, a planning report and any technical reports required by the relevant categorising instruments (e.g. local government planning schemes, State Planning Policy, State Development Assessment Provisions). For further information, see <u>DA</u> Forms Guide: Planning Report Template.	⊠ Yes
Relevant plans of the development are attached to this development application Note : Relevant plans are required to be submitted for all aspects of this development application. For further information, see <u>DA Forms Guide: Relevant plans</u> .	⊠ Yes
The portable long service leave levy for QLeave has been paid, or will be paid before a development permit is issued (see 21)	☐ Yes ⊠ Not applicable

25) Applicant declaration

- By making this development application, I declare that all information in this development application is true and correct
- Where an email address is provided in Part 1 of this form, I consent to receive future electronic communications from the assessment manager and any referral agency for the development application where written information is required or permitted pursuant to sections 11 and 12 of the *Electronic Transactions Act 2001*

Note: It is unlawful to intentionally provide false or misleading information.

Privacy – Personal information collected in this form will be used by the assessment manager and/or chosen assessment manager, any relevant referral agency and/or building certifier (including any professional advisers which may be engaged by those entities) while processing, assessing and deciding the development application. All information relating to this development application may be available for inspection and purchase, and/or published on the assessment manager's and/or referral agency's website.

Personal information will not be disclosed for a purpose unrelated to the *Planning Act 2016*, Planning Regulation 2017 and the DA Rules except where:

- such disclosure is in accordance with the provisions about public access to documents contained in the *Planning Act 2016* and the Planning Regulation 2017, and the access rules made under the *Planning Act 2016* and Planning Regulation 2017; or
- required by other legislation (including the Right to Information Act 2009); or
- otherwise required by law.

This information may be stored in relevant databases. The information collected will be retained as required by the *Public Records Act 2002.*

PART 9 – FOR COMPLETION OF THE ASSESSMENT MANAGER – FOR OFFICE USE ONLY

Date received:	Reference numb	er(s):
Notification of enga	agement of alternative assessment man	ager
Prescribed assessment manager		
Name of chosen assessment manager		
Date chosen assessment manager engaged		
Contact number of	f chosen assessment manager	
Relevant licence n	umber(s) of chosen assessment	

manager

QLeave notification and payment Note: For completion by assessment manager if applicable			
Description of the work			
QLeave project number			
Amount paid (\$)		Date paid (dd/mm/yy)	
Date receipted form sighted by assessment manager			
Name of officer who sighted the form			

	Unit No.	Street No.	Street Name and Type	Suburb
	-	1299B	Mossman Daintree Road	Rocky Point
C)	Postcode	Lot No.	Plan Type and Number (e.g. RP, SP)	Local Government Area(s)
	4874	3	SP238245	Douglas Shire Council



TOWN PLANNING REPORT CODE ASSESSABLE DEVELOPMENT APPLICATION

MATERIAL CHANGE OF USE (DEVELOPMENT PERMIT) FOR A DWELLING HOUSE

UPON LAND AT 1299C MOSSMAN DAINTREE ROAD, ROCKY POINT LOT 4 ON SP238245

ON BEHALF OF DEBELL PROPERTY PTY LTD ATF DEBELL PROPERTY TRUST

OUR REFERENCE: 22-0117



PROJECT DETAILS

CLIENT:	Debell Property Pty Ltd as trustee for Debell Property Trust
PROJECT ADDRESS:	1299C Mossman Daintree Road, Rocky Point
PROJECT NUMBER:	22-0117
TPA PROJECT CONTACT:	Jessica Robson
	Brendan Ferris

TOWN PLANNING ALLIANCE CONTACT DETAILS

Brisbane	Byron Bay	Gold Coast
07 3361 9999	02 6637 9110	07 5613 2499
57 Manilla Street	Office 11, 1 Porter Street	Office 6, 155 Varsity Parade
EAST BRISBANE Q 4169	BYRON BAY NSW 2471	GOLD COAST QLD 4227

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Distribution

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1	2	Town Planning Alliance Pty Ltd
1	2	Douglas Shire Council
1	2	Debell Property Pty Ltd ATF Debell Property Trust



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1 EXECUTIVE SUMMARY

This report is submitted on behalf of *Debell Property Pty Ltd as trustee for Debell Property Trust* (the applicant) in support of a Material Change of Use (Development Permit) for a Dwelling house upon land at 1299C Mossman Daintree Road, Rocky Point legally described as Lot 4 on SP238245. Lot 2 & 3 on SP238245 is also included in the application for access purposes only.

This report addresses the merits of the proposed development with respect to the provisions of the *Douglas Shire Planning Scheme 2018* and the relevant components of the *Planning Act 2016*.

Under the *Douglas Shire Planning Scheme 2018*, the site is identified as being located within the Environmental management zone, which is intended to recognise environmentally sensitive areas and provide for houses on lots and other low impact activities where suitable. The Environmental management zone code prescribes development to be generally restricted to a Dwelling house.

The proposal seeks to establish a Dwelling house upon the subject site, delivering an anticipated use in accordance with the intent of the Environmental Management Zone. The proposal seeks to deliver a two-storey product that adopts a high-quality architectural style, with significant articulation, building recesses, modulation, as well as the use of a variation of materials and finishes to present as a development with high aesthetic value. The proposal dedicates a suitable portion of the site to landscaping to soften the built form, creating a balance between the built and natural form, positively contributing to the surrounding streetscape.

The proposal is identified as being consistent with the overall outcomes of the Environmental management zone, as the development -

- Is solely for a single residential dwelling house on a subdivided lot;
- The Dwelling house is strategically designed and sited to manage any potential environmental impacts on both the site and adjoining properties;
- Remains commensurate to pre-existing residential development in the area, with a compatible scale and design that appropriately utilises the subject allotment;
- A The proposed design results in a high quality urban design which is functional and provide generous private open space for residential living; and
- Appropriately responds to site-specific constraints, and mitigates potential natural hazards.

The proposed development provides a built form and scale that is identified as achieving the relevant assessment benchmarks applicable to the application. Overall, the proposal is identified as meeting the relevant requirements of the *Douglas Shire Planning Scheme 2018*. As such, the application should be approved subject to reasonable and relevant conditions.



2 **PROJECT OVERVIEW**

Applicant Name:	Dehell Property P	ty I thas	trustee f	or Dehe	ll Property Tr	ust	
Site Address:	Debell Property Pty Ltd as trustee for Debell Property Trust 1299C Mossman Daintree Road, Rocky Point						
RPD:		Lot 4 on SP238245 and Lot 2 & 3 on SP238245 for access purposes					
Area of Site:	3042m ²		2000	0.2002			
Local Government Area:	Douglas Shire Cou	ıncil					
Zone	Environmental M		nt				
Precinct	N/A	anageme					
Overlays:	Acid sulfate s	oils overl	av				
overlays.	 Bushfire haza 		•				
	A Hillslopes over		' y				
	 Natural areas 	-					
	A Potential land	•	vard over	lav			
	🙏 Transport ne			,			
Existing Use	Vacant						
Details of Proposal:	Material Change	of Use					
	Gross floor	N/A		Site co	ver	7.0%	
	area	-					
	Building height 8.1m No. of storeys				storeys	2 storeys	
	Number of			Car parks		2 spaces	
	units			•			
	Reconfiguration of	of Lots				•	
	Existing Lots		N/A			New Lots	N/A
Application Type:	Aspects of		Type of	Approv	al Requeste	d	
	Development		Preliminary Approval		Development Permit		
	Material Change	of Use					✓
	Reconfiguration of	of a Lot					
	Building Work						
	Operational Wor	k					
Level of Assessment:	Code Assessable		Pre-Loc	lgement	t Advice	N/A	
Public Notification:	No						
Referral Agencies:	Agency Concurrence/ Advice						
	N/A N/A						
Specialist Plans &	Proposal Plans prepared by Office Deicke;						
Reports Provided:	Geotechnical Rep	•	-				
	Onsite Sewerage	-	-	-		nts	
	Water Pressure E		Report b	oy Acor (Consultants		
	Bushfire Report b						
Applicant Contact:	Brendan Ferris / J	essica Ro	bson				
Applicant Email:	eda@tpalliance.c	om.au					



3 BACKGROUND & SITE CHARACTERISTICS

3.1 Site Details

Site address:	1299C Mossman Daintree Road, Rocky Point
RPD:	Lot 4 on SP238245 and Lot 2 & 3 on SP238245 for access purposes
Site area:	3,042m ²



Figure 1: 1299B Mossman Daintree Road, Rocky Point

3.2 Site History

As shown in Figure 1 (above), the subject site is currently vacant and is not improved by any built structures. A sealed road exists to the end of the battle-axe handle.

The subject site is a result of a residential subdivision in 2009 (Council Ref: 8/37/79(2372305)) which created 3 residential allotments and an access easement. An excerpt of the approved subdivision plan is provided further below.

The applicant seeks to establish a Dwelling house on each of the 3 allotments, however this application is limited to Lot 2 from the resultant subdivision.

It is noted that the subject development application also includes Lot 2 & 3 on SP238245 for access purposes only given the subject site requires access through Lot 2 & 3 to obtain access to Mossman Daintree Road.





Figure 2: Excerpt of approved subdivision plan

3.3 Location

The subject site is located at 1299C Mossman Daintree Road, Rocky Point, formally described as Lot 4 on SP238245. In accordance with *Douglas Shire Planning Scheme 2018*, the subject site is identified within the Environmental management zone. The surrounding area is also primarily identified within the Environmental management zone and Conservation zone.





A Subject Site

B Environmental zone

C Conservation zone

Figure 3: Surrounding Zoning

3.4 Site Characteristics

The subject site represents a rear allotment, 3,042m² in area currently vacant and not encumbered by existing structures. The site will obtain access to Mossman Daintree Road via a sealed road to the end of the battle-axe handle and access easment over Lot 2 & 3 on SP238245

Topography

The subject site possesses a significant change in level from the proposed house pad towards the rear of the site at 118m AHD to 93m at the front.

Flooding

Douglas Shire Council Planning Scheme 2018 indicates the subject site is not susceptible to a flood hazard from any source.

Vegetation

The subject site is devoid of any significant vegetation. It is further noted the Dwelling house will be situated within an existing house pad to minimise the amount of site works required to reduce the impacts on the surrounding vegetation.

3.5 Pre-Lodgement History

A Prelodgement meeting was not undertaken for the proposed development.



4 **PROPOSAL**

The report is submitted in support of the proposed development for a new Dwelling house in the Environmental management zone.

4.1 Proposed Design

The design of the new Dwelling house has extensively considered the character of the surrounding area through architectural form and material selection. The proposal involves the following design features:

Ground Floor								
Double car garage	Double car garage 2 x Bedrooms							
Living room	Swimming pool	Outdoor deck						
Second Floor								
 Living / dining 	Study	Kitchen						
Upper level balcony	 Master bedroom w/ Ensuite 							

The proposed dwelling is accessed via an access handle and crossover onto Mossman Daintree Road. The location of the proposed car accommodation is designed to allow for safe and efficient vehicle manoeuvring within the site and will be consistent with Council standards.

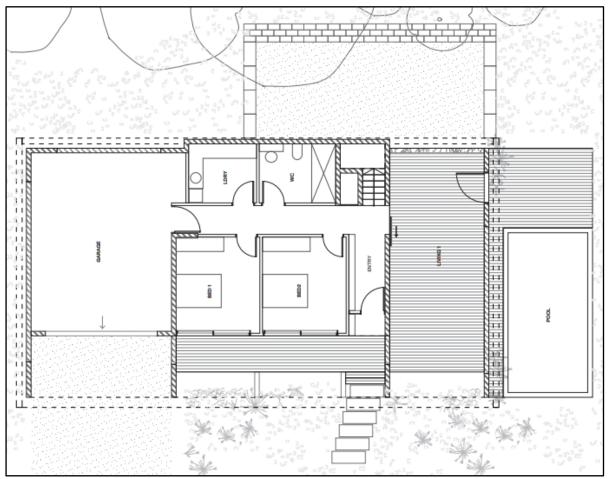


Figure 4: Ground Floor

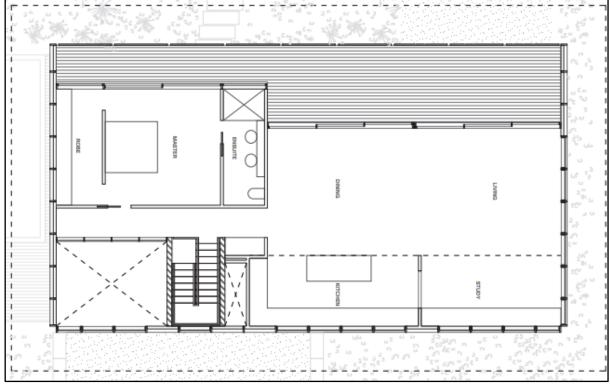


Figure 5: Second Floor

4.2 Building Envelope

The proposed design results in a maximum building height that is maintained under 8.5m and two (2) storeys in accordance with the Environmental management zone code. To this end, the proposed building height complies with the acceptable outcome of the *Douglas Shire Planning Scheme 2018*.

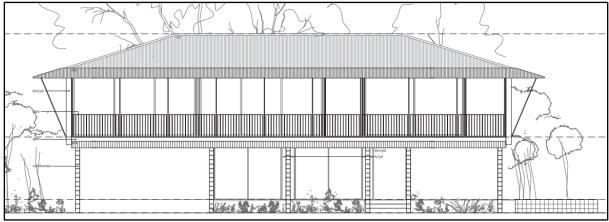


Figure 6: Side Elevation

In addition to the above, the proposed Dwelling house is strategically designed and located within the existing house pad which is centred in the lot and appropriately sited to provide generous setbacks to all boundaries. The proposed building envelope is well-located away from the property boundary and achieves the relevant setback provisions applicable under the assessment benchmarks.

vnPlan



4.3 Car Parking and Access

Access to the development will be provided via the access handle and crossover from Mossman Daintree Road from the previous subdivision. It is noted that the subject development application also includes Lot 2 & 3 on SP238245 for access purposes only given the subject site requires access through Lot 2 and Lot 3 to obtain access to Mossman Daintree Road.

The driveway arrangement is suitable for access and will allow for the necessary spacing for internal manoeuvring for residents of the site. The development seeks to provide two (2) private car parking spaces in the form of a double garage. As such, the proposal provides sufficient car parking in accordance with the provisions identified under the assessment benchmarks.

4.4 Natural Areas Overlay & Bushfire Hazard Overlay

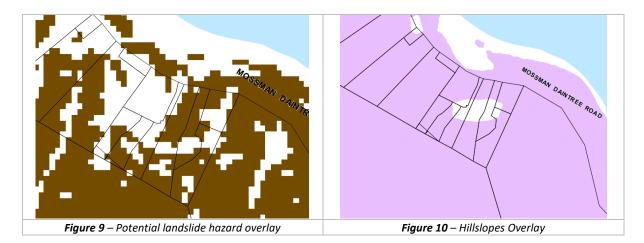
Given the site's significance within the Environmental management zone, the subject site is also encumbered by the Natural areas overlay and Bushfire hazard overlay as illustrated below in Figures 7 & 8. Notwithstanding, the proposed Dwelling house has been designed to ensure the siting and built form results in minimal impacts on the existing vegetation. The proposal will be wholly contained within the existing house pad from the previous subdivision and will not result in the removal of significant vegetation. With regards to the bushfire provisions, the Dwelling house is appropriately separated from the bushfire hazard and will utilize fitting materials to minimise any further potential risk. Please refer to the Bushfire report prepared by *Litoria* for further details.



4.5 Potential Landslide Hazard Overlay & Hillslopes Overlay

In accordance with the *Douglas Shire Planning Scheme 2018*, the subject site is impacted by the Potential landslide hazard overlay and Hillslopes overlay as illustrated below in Figure 9 & 10. As such, the proposed dwelling house has been designed in accordance with consultant supporting documentation to ensure any landslide risk is appropriately mitigated and not result in further adverse impacts on the subject site and adjoining properties. Please refer to the Geotechnical report prepared by *Douglas Partners*.





4.6 Infrastructure Services

The subject site is located within a rural setting and does not have direct access to reticulated sewer. Notwithstanding, the proposed Dwelling house will be supported by an onsite effluent system to service the dwelling house. Please refer to the Onsite sewerage management report prepared by *Acor Consultants*.

Similarly due to the rural setting, the subject site also has limited access to reticulated water. Notwithstanding, a suitable solution is proposed as demonstrated in the Water Pressure Evaluation Report prepared by *Acor Consultants* to ensure the Dwelling house can be appropriately serviced with water supply.

Overall, the proposal is identified as meeting the relevant requirements of the *Douglas Shire Planning Scheme 2018*. As such, the application should be approved subject to reasonable and relevant conditions.



5 STATE PLANNING FRAMEWORK

5.1 Planning Act 2016 – Framework

The *Planning Act 2016* provides the statutory planning framework for the State of Queensland and outlines the statutory process for preparing, assessing and deciding development applications. This proposal will be assessed in accordance with the relevant components of the *Planning Act 2016*.

The proposal will be assessed in accordance with the Development Assessment Rules under section 68(1) of the *Planning Act 2016*.

5.2 State Planning Policies

A new State Planning Policy was introduced on 3 July 2017, expressing the state's interests in land use planning and development. The new State Planning Policy has replaced the existing State Planning Policy 2006.

Part E of the State Planning Policy outlines the State interest policies and assessment benchmarks. These policies and assessment benchmarks are applicable to the assessment of development applications to the extent that the new State Planning Policy has not been identified in a Planning Scheme as being appropriately integrated.

The new State Planning Policy is identified in the *Douglas Shire Planning Scheme* 2018 as being appropriately integrated. As such, an assessment of the State interest policies is not required in this instance.

5.3 Referral Agency

As of 3 July 2017, the Department of State Development, Infrastructure and Planning launched the State Assessment and Referral Agency (SARA). In accordance with the *Planning Act 2016*, a Referral Agency is responsible for the assessment of development applications.

It is acknowledged that the subject site adjoins a State transport corridor, however given the Dwelling house does not require a new or changed access to the State transport corridor as thus is considered to be an excluded material change of use, as defined by Schedule 24 of the *Planning Regulation 2017*.

As such, in accordance with the *Planning Regulation 2017*, the application does not trigger any referrals.

5.4 State Development Assessment Provisions

The State Development Assessment Provisions came into effect on 01 July 2013 and set out matters of interest to the State for development assessment where the State has justification as an assessment manager or referral agency. The State Development Assessment Provisions Versions 2.0 came into effect on 3 July 2017.



The State Assessment and Referral Agency is not triggered as the assessment manager or a referral agency for this application under the *Planning Regulation 2017*. As such, the State Development Assessment Provisions are not applicable to the proposal.

5.5 Far North Queensland Regional Plan 2009-2031

As outlined in part 2.2 of the *Douglas Shire Planning Scheme 2018*, the planning scheme appropriately advances the *Far North Queensland Regional Plan 2009-2031*, as it applies in the planning scheme area.

Given the proposal is generally consistent with the provisions of the planning scheme, no further action is required in addressing the regional plan.



6 LOCAL PLANNING FRAMEWORK

6.1 Definition

The *Douglas Shire Planning Scheme 2018* defines a Dwelling house as follows:

"A residential use of premises for one household that contains a single dwelling. The use includes domestic outbuildings and works normally associated with a dwelling and may include a secondary dwelling."

In view of the above, the proposal is consistent with the definition of a Dwelling house.

6.2 Level of Assessment

Under the *Douglas Shire Planning Scheme 2018*, the site is located within the Environmental management zone. In addition, the site is subject to the following Overlays:

- Acid sulfate soils overlay;
- Bushfire hazard overlay;
- Hillslopes overly;
- Natural areas overlay;
- Potential landslide hazard overlay;
- Transport network overlay.

In accordance with Part 5 of the *Douglas Shire Planning Scheme 2018*, the application is subject to Code Assessment on the following basis:

LEVEL OF ASSESSM	LEVEL OF ASSESSMENT					
ZONE						
ENVIRONMENTAL	MANAGEMENT ZONE					
Code Assessable	In accordance with Table 5.6.D – Levels of Assessment for Environmental management zone, a new Dwelling house is Code Assessable and subject to the assessment of various codes. As such, the application is code assessable and the Environmental management zone code and associated codes are addressed as part of this application.					
OVERLAYS						
No change	The applicable overlays mentioned above do not vary the level of assessment for the proposed Dwelling house.					

In light of the above, the prevailing level of assessment for the application is **Code Assessable**.



6.3 Applicable Codes and Provisions

In accordance with Part 5 of the *Douglas Shire Planning Scheme 2018*, the following codes are identified as being applicable to the proposal:

APPLICABLE CODES					
Zone Codes	🚴 Environmental management zone code				
Use Codes	🙏 Dwelling house code				
Secondary Codes	Access, parking and servicing code				
	Filling and excavation code				
	🙏 Infrastructure code				
	Vegetation management code				
Overlay Codes	🙏 Acid sulfate soils overlay code				
	🙏 Bushfire hazard overlay code				
	🙏 Hillslopes overlay code				
	🙏 Natural areas overlay code				
	Potential landslide hazard overlay code				

The abovementioned applicable Codes have been addressed in **Appendix 1** and the attached consultant reports.



7 CONCLUSION

This report is submitted on behalf of *Debell Property Pty Ltd as trustee for Debell Property Trust* (the applicant) in support of a Material Change of Use (Development Permit) for a Dwelling house upon land at 1299C Mossman Daintree Road, Rocky Point legally described as Lot 4 on SP238245. Lot 2 & 3 on SP238245 is also included in the application for access purposes only.

This report has addressed the merits of the proposed development with respect to the provisions of the *Douglas Shire Planning Scheme 2018* and the relevant components of the *Planning Act 2016*.

The proposal seeks to establish a Dwelling house upon the subject site, delivering an anticipated use in accordance with the intent of the Environmental Management Zone. The proposal seeks to deliver a two-storey product that adopts a high-quality architectural style, with significant articulation, building recesses, modulation, as well as the use of a variation of materials and finishes to present as a development with high aesthetic value. The proposal dedicates a suitable portion of the site to landscaping to soften the built form, creating a balance between the built and natural form, positively contributing to the surrounding streetscape.

Overall, the proposal is identified as meeting the relevant requirements of the *Douglas Shire Planning Scheme 2018*. As such, the application should be approved subject to reasonable and relevant conditions.



8 APPENDIX 1 – CODE ASSESSMENT



APPENDIX 1 – CODE ASSESSMENT

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1 Zone Codes

- 1.1 Environmental Management Zone Code
- 1.1.1 Purpose

PU	RPO	SE & OVERALL OUTCOMES	PROPO	DSAL	COMPLIANCE		
1)	 The purpose of the Environmental management zone code is to recognise environmentally sensitive areas and provide for houses on lots and other low impact activities where suitable. 						
2)	 a) implement the policy direction set in the Strategic Framework, in particular: (i) Theme 2 : Environment and landscape values, Element 3.5.3 – Biodiversity, Element 3.5.5 – Scenic amenity. b) protect and buffer areas of environmental significance from inappropriate development. 						
	a) b) c) d)	Development is generally restricted to a dwelling house; Adverse impacts on natural systems, both on-site and on adjoining land are minimised through the location, design and management of development; Development reflects and responds to the natural features and environmental values of the area; Visual impacts are minimised through the location and design of development;	a) b) c) d)	The proposal is for a new Dwelling house within an approved subdivision; The proposal has been designed to ensure the Dwelling house does not adversely impact upon the site and adjoining land; The proposal appropriately responds to the natural features and environmental values of the area; The proposal will not result in visual impacts;	⊠Compliant □N/A □Performance Solution		
	e) f)	Development does not adversely affect water quality; Development responds to land constraints, including but not limited to topography, vegetation, bushfire, landslide and flooding.	e) f)	The dwelling house will not impact upon water quality; The proposed dwelling house has been designed in accordance with supporting consultant documentation to ensure compliance is achieved against the land constraints.			





1.1.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
For assessable development			
PO1	A01.1	A01.1	
The height of all buildings and structures is in keeping with the natural characteristics of the site. Buildings and	Buildings and structures are not more than 8.5 metres and two storeys in height.	The proposal achieves a building height no greater than 8.5m and 2 storeys.	⊠Compliant □N/A
structures are low-rise and not unduly			□Performance Solution
visible from external sites.	A01.2	A01.2	
	Buildings have a roof height not less than 2 metres.	The proposal does not have a roof height less than 2m.	⊠Compliant □N/A □Performance Solution
PO2	AO2	A02	
 Buildings and structures are set back to: a) maintain the natural character of the area; b) achieve separation from neighbouring buildings and from 	 Buildings and structures are set back not less than: a) 40 metres from the frontage of a state controlled road; b) 25 metres from the frontage to Cape Tribulation Road; 	The subject site is appropriately setback from the street frontage meeting the acceptable outcome.	⊠Compliant □N/A □Performance Solution
road frontages.	c) 6 metres from any other road;d) 6 metres from the side and rear boundaries of the site.		
PO3	AO3	A03	
Development is consistent with the purpose of the Environmental management zone and protects the zone from the intrusion of inconsistent uses.	Inconsistent uses as identified in Table 6.2.4.3.b are not established in the Environmental management zone.	The proposal is for a dwelling house which is identified as a consistent use within the Environmental management zone.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
PO4 The site coverage of all buildings and structures and associated services do not have an adverse effect on the environmental or scenic values of the site.	PO4 No acceptable outcomes are prescribed.	AO4 The proposal generates a site cover of approximately 7% which will not have an adverse impact on the scenic values of the site.	⊠Compliant □N/A □Performance Solution
PO5 Development is located, designed, operated and managed to respond to the characteristics, features and constraints of the site and its surrounds.	 AO5.1 Buildings, structures and associated access, infrastructure and private open space are sited: a) within areas of the site which are already cleared; or b) within areas of the site which are environmentally degraded; c) to minimise additional vegetation clearing. 	AO5.1 The proposed siting of the dwelling house is located within generally clear areas of the site along the house pad.	⊠Compliant □N/A □Performance Solution
	AO5.2 Buildings and structures and associated infrastructure are not located on slopes greater than 1 in 6 (16.6%) or on a ridgeline.	AO5.2 The proposed dwelling is not located on a slope that is greater than 1 in 6.	⊠Compliant □N/A □Performance Solution
 PO6 Buildings and structures are responsive to steep slope through innovative construction techniques so as to: a) maintain the geotechnical stability of slopes; 	AO6.1 Where development on land steeper than 1 in 6 (16.6%) cannot be avoided, development follows the natural contours of the land and single plane concrete slab on-ground methods of construction are not utilised.	AO6.1 The proposed dwelling house is located on a house pad that is not steeper than 1 in 6.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
 b) minimise cut and/or fill; c) minimise the overall height of development. 	 AO6.2 Access and vehicle manoeuvring and parking areas are constructed and maintained to: a) minimise erosion; b) minimise cut and fill; follow the natural contours of the site. 	AO6.2 Access to the site will be granted in accordance with the approved subdivision via the access handle.	⊠Compliant □N/A □Performance Solution
PO7 The exterior finishes of buildings and structures are consistent with the surrounding natural environment.	PO7 The exterior finishes and colours of buildings and structures are non-reflective and are moderately dark to darker shades of grey, green, blue and brown or the development is not visible external to the site.	AO7 The exterior finishes of the building will be consistent with the surrounding natural environment.	⊠Compliant □N/A □Performance Solution
PO8 Development does not adversely affect the amenity of the zone and adjoining land uses in terms of traffic, noise, dust, odour, lighting or other physical or environmental impacts.	AO8 No acceptable outcomes are prescribed.	AO8 The proposal does not adversely impact the amenity of the zone and adjoining land uses.	⊠Compliant □N/A □Performance Solution
PO9 The density of development ensures that the environmental and scenic amenity values of the site and surrounding area are not adversely affected.	AO9 The maximum residential density is one dwelling house per lot.	AO9 The proposal is limited to one dwelling house per lot.	⊠Compliant □N/A □Performance Solution
PO10 Lot reconfiguration results in no additional lots.	AO10 No acceptable outcomes are prescribed.	AO10 The proposal is not for a reconfiguring a lot.	□Compliant ⊠N/A





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
			□Performance Solution

2 Use / Development Codes

2.1 Dwelling House Code

2.1.1 Purpose

PURPOSE & OVERALL OUTCOMES	PROPOSAL	COMPLIANCE
1) The purpose of the Dwelling house code is to assess the suitability		
2) The purpose of the code will be achieved through the following o		
 a) The dwelling house, including all habitable buildings on site, is occupied by a single household; b) A dwelling house, including a secondary dwelling or domestic out-buildings; ensures that the secondary dwelling is sub-ordinate to the primary dwelling house; c) Development of a dwelling house provides sufficient and safe vehicle access and parking for residents; d) The built form, siting, design and use of each dwelling is consistent with the desired neighbourhood character and streetscape elements of the area 	 a) The proposed dwelling house will be occupied by a single household. b) The proposal is limited to a single dwelling house and does not propose a secondary dwelling. c) The proposal provides safe and sufficient vehicle access for the parking of residents. d) The dwelling house is designed and sited to be consistent with the character and outcomes sought in the Environmental management zone. 	⊠Compliant □N/A □Performance Solution





2.1.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE			
For self-assessable and assessable development						
PO1	A01	A01				
 Secondary dwellings: a) are subordinate, small-scaled dwellings; b) contribute to a safe and pleasant living environment; c) are established on appropriate sized lots; d) do not cause adverse impacts on 	 The secondary dwelling: a) has a total gross floor area of not more than 80m2, excluding a single carport or garage; b) is occupied by 1 or more members of the same household as the dwelling house. 	The proposal does not include a secondary dwelling.	□Compliant ⊠N/A □Performance Solution			
adjoining properties.						
PO2 Resident's vehicles are accommodated on- site.	 AO2 Development provides a minimum number of on- site car parking spaces comprising: a) 2 car parking spaces which may be in tandem for the dwelling house; b) 1 car parking space for any secondary dwelling on the same site. 	AO2 The dwelling house includes double car accommodate in the form of a garage.	⊠Compliant □N/A □Performance Solution			
 PO3 Development is of a bulk and scale that: a) is consistent with and complements the built form and front boundary setbacks prevailing in the street and local area; b) does not create an overbearing development for adjoining 	AO3 Development meets the acceptable outcome for building height in the applicable Zone code associated with the site.	AO3 The proposal achieves a compliant building height in accordance with the Environmental management zone.	⊠Compliant □N/A □Performance Solution			





PE	RFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	dwelling houses and their private open space;			
c)	does not impact on the amenity and privacy of residents in adjoining dwelling houses;			
d)	ensures that garages do not dominate the appearance of the street.			





3 Secondary Codes

3.1 Access, Parking and servicing Code

3.1.1 Purpose

PURPC	DSE & OVERALL OUTCOMES	PROPO	DSAL	COMPLIANCE		
-	 The purpose of the Access, parking and servicing code is to assess the suitability of access, parking and associated servicing aspects of a development. The purpose of the code will be achieved through the following overall outcomes: 					
 a) b) c) d) e) f) 	sufficient vehicle parking is provided on-site to cater for all types of vehicular traffic accessing and parking on- site, including staff, guests, patrons, residents and short term delivery vehicles; sufficient bicycle parking and end of trip facilities are provided on-site to cater for customer and service staff; on-site parking is provided so as to be accessible and convenient, particularly for any short term uses; development provides walking and cycle routes through the site which link the development to the external walking and cycling network; the provision of on-site parking, loading / unloading facilities and the provision of access to the site do not impact on the efficient function of street network or on the area in which the development is located; new vehicular access points are safely located and are not in conflict with the preferred ultimate streetscape character and local character and do not unduly disrupt any current or future on-street parking arrangements.	a) b) c) d) e) f)		⊠Compliant □N/A □Performance Solution		





3.1.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE			
For self-assessable and assessable dev	For self-assessable and assessable development					
PO1	A01.1	A01.1				
Sufficient on-site car parking is provided to cater for the amount and type of vehicle traffic expected to be generated by the use or uses of the site,	The minimum number of on-site vehicle parking spaces is not less than the number prescribed in Table 9.4.1.3.b for that particular use or uses.	The proposal provides on-site car parking in accordance with Table 9.4.1.3.B.	⊠Compliant □N/A □Performance Solution			
having particular regard to:	A01.2	A01.2				
 a) the desired character of the area; b) the nature of the particular use and its specific characteristics and scale; 	Car parking spaces are freely available for the parking of vehicles at all times and are not used for external storage purposes, the display of products or rented/sub-leased.	The proposed garage will be freely available for the parking of vehicles.	⊠Compliant □N/A □Performance Solution			
c) the number of employees and the	A01.3	A01.3				
likely number of visitors to the site;	Parking for motorcycles is substituted for ordinary vehicle parking to a maximum level of 2% of total ordinary vehicle parking.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A			
d) the level of local accessibility;			□Performance Solution			
 e) the nature and frequency of any public transport serving the area; f) whether or not the use involves the retention of an existing building and the previous requirements for car parking for the building 	AO1.4 For parking areas exceeding 50 spaces parking, is provided for recreational vehicles as a substitute for ordinary vehicle parking to a maximum of 5% of total ordinary vehicle parking rate.	AO1.4 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution			
 g) whether or not the use involves a heritage building or place of local significance; 						





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
 h) whether or not the proposed use involves the retention of significant vegetation. 			
PO2	AO2	AO2	
Vehicle parking areas are designed and constructed in accordance with relevant standards.	 Vehicle parking areas are designed and constructed in accordance with Australian Standard: a) AS2890.1; b) AS2890.3; c) AS2890.6. 	Vehicle parking areas will be in accordance with the Australian standards.	⊠Compliant □N/A □Performance Solution
PO3	AO3.1	AO3.1	
 Access points are designed and constructed: a) to operate safely and efficiently; b) to accommodate the anticipated type and volume of vehicles c) to provide for shared vehicle 	 Access is limited to one access cross over per site and is an access point located, designed and constructed in accordance with: a) Australian Standard AS2890.1; b) Planning scheme policy SC6.5 – FNQROC Regional Development Manual - access crossovers. 	Access to the site will be in accordance with the existing subdivision approval granted over the site.	⊠Compliant □N/A □Performance Solution
(including cyclists) and pedestrian	A03.2	A03.2	
 use, where appropriate; d) so that they do not impede traffic or pedestrian movement on the adjacent road area; e) so that they do not adversely 	Access, including driveways or access crossovers: a) are not placed over an existing: (i) telecommunications pit;	Access to the site will be in accordance with the existing subdivision approval granted over the site.	⊠Compliant □N/A □Performance Solution
impact upon existing intersections or future road or intersection improvements;	 (ii) stormwater kerb inlet; (iii) sewer utility hole; (iv) water valve or hydrant. b) are designed to accommodate any 		





PER	FORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
f) g)	so that they do not adversely impact current and future on- street parking arrangements; so that they do not adversely impact on existing services within	adjacent footpath; c) adhere to minimum sight distance requirements in accordance with AS2980.1.		
	the road reserve adjacent to the site;	AO3.3 Driveways are:	AO3.3 Access to the site will be in accordance with	⊠Compliant
h)	so that they do not involve ramping, cutting of the adjoining road reserve or any built structures (other than what may	 a) designed to follow as closely as possible to the existing contours, but are no steeper than the gradients outlined in Planning scheme policy SC6.5 – FNQROC Regional Development Manual; 	the existing subdivision approval granted over the site.	□N/A □Performance Solution
	be necessary to cross over a stormwater channel).	 b) constructed such that where there is a grade shift to 1 in 4 (25%), there is an area with a grade of no more than 1 in in 6 (16.6%) prior to this area, for a distance of at least 5 metres; 		
		 c) on gradients greater than 1 in 6 (16.6%) driveways are constructed to ensure the cross-fall of the driveway is one way and directed into the hill, for vehicle safety and drainage purposes; 		
		 constructed such that the transitional change in grade from the road to the lot is fully contained within the lot and not within the road reserve; 		
		 e) designed to include all necessary associated drainage that intercepts and directs storm water runoff to the storm water drainage system. 		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	AO3.4 Surface construction materials are consistent with the current or intended future streetscape or character of the area and contrast with the surface construction materials of any adjacent footpath.	AO3.4 Access to the site will be in accordance with the existing subdivision approval granted over the site.	⊠Compliant □N/A □Performance Solution
P04	AO4	AO4	
Sufficient on-site wheel chair accessible car parking spaces are provided and are identified and reserved for such purposes.	The number of on-site wheel chair accessible car parking spaces complies with the rates specified in AS2890 Parking Facilities.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
PO5	A05	A05	
Access for people with disabilities is provided to the building from the parking area and from the street.	Access for people with disabilities is provided in accordance with the relevant Australian Standard	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
PO6	AO6	AO6	
Sufficient on-site bicycle parking is provided to cater for the anticipated demand generated by the development.	The number of on-site bicycle parking spaces complies with the rates specified in Table 9.4.1.3.b.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
P07	A07.1	A07.1	
Development provides secure and convenient bicycle parking which:	Development provides bicycle parking spaces for employees which are co-located with end-of-trip facilities (shower cubicles and lockers);	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
 a) for visitors is obvious and located close to the building's main entrance; b) for employees is conveniently located to provide secure and 	AO7.2 Development ensures that the location of visitor bicycle parking is discernible either by direct view or using signs from the street.	AO7.2 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
convenient access between the bicycle storage area, end-of-trip facilities and the main area of the building;c) is easily and safely accessible from outside the site.	AO7.3 Development provides visitor bicycle parking which does not impede pedestrian movement.	AO7.2 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
 PO8 Development provides walking and cycle routes through the site which: a) link to the external network and pedestrian and cyclist destinations such as schools, shopping centres, open space, public transport stations, shops and local activity centres along the safest, most direct and convenient routes; b) encourage walking and cycling; c) ensure pedestrian and cyclist safety. 	 AO8 Development provides walking and cycle routes which are constructed on the carriageway or through the site to: a) create a walking or cycle route along the full frontage of the site; b) connect to public transport and existing cycle and walking routes at the frontage or boundary of the site. 	AO8 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
PO9 Access, internal circulation and on-site parking for service vehicles are designed and constructed:	AO9.1 Access driveways, vehicle manoeuvring and on- site parking for service vehicles are designed and constructed in accordance	AO9.1 Access to the site will be in accordance with the existing subdivision approval granted over the site.	⊠Compliant □N/A





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
a) in accordance with relevant standards;	with AS2890.1 and AS2890.2.		□Performance Solution
b) so that they do not interfere with	A09.2	A09.2	
the amenity of the surrounding area;	Service and loading areas are contained fully within the site.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant
c) so that they allow for the safe and			⊠N/A
convenient movement of			□Performance Solution
pedestrians, cyclists and other	AO9.3	AO9.3	
vehicles.	The movement of service vehicles and service operations are designed so they:	The proposal is for a dwelling house. As such, this criterion is not applicable.	
	a) do not impede access to parking spaces;		⊠N/A
	 b) do not impede vehicle or pedestrian traffic movement. 		□Performance Solution
PO10	AO10.1	A010.1	
Sufficient queuing and set down areas	Development provides adequate area on-	The proposal is for a dwelling house. As such,	□Compliant
are provided to accommodate the demand generated by the	site for vehicle queuing to accommodate the demand generated by the development	this criterion is not applicable.	⊠N/A
development.	where drive through facilities or drop-		□Performance Solution
	off/pick-up services are proposed as part of		
	the use, including, but not limited to, the		
	following land uses:		
	a) car wash;		
	b) child care centre;c) educational establishment where for a		
	school;		
	 food and drink outlet, where including a drive- through facility; 		
	 e) hardware and trade supplies, where including a drive-through facility; 		
	f) hotel, where including a drive-through		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	facility;		
	g) service station.		
	AO10.2	A010.2	
	Queuing and set-down areas are designed	The proposal is for a dwelling house. As such,	□Compliant
	and constructed in accordance with AS2890.1.	this criterion is not applicable.	⊠N/A
			□Performance Solution

3.2 Infrastructure Works Code

3.2.1 Purpose

PURPOSE & OVERALL OUTCOMES	PROPOSAL	COMPLIANCE	
	 The purpose of the Infrastructure works code is to ensure that development is safely and efficiently serviced by, and connected to, infrastructure. The purpose of the Infrastructure works code is to ensure that development is safely and efficiently serviced by, and connected to, infrastructure. 		
 a) the standards of water supply, waste water disposal, stormwater drainage, local electelecommunications, footpaths and road meet the needs of development and efficient; b) development maintains high environments c) development is located, designed, commanaged to avoid or minimise impacts altered stormwater quality or flow, discharge, and the creation of non-waterways; d) the integrity of existing infrastructure is material storms and storms and storms and storms and the sto	 ctricity supply, d construction are safe and al standards; nstructed and s arising from , wastewater tidal artificial effluent treatment as well as sufficient of the supporting construction of the support of th	ent water supply. ultant reports for gh environmental result in significant low; g infrastructure; etract from any	





PURPOSE & OVERALL OUTCOMES	PROPOSAL	COMPLIANCE
e) development does not detract from environmental values or the desired character and amenity of an area.		

3.2.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
For self-assessable and assessable deve	lopment		
Works on a local government road			
PO1 Works on a local government road do not adversely impact on footpaths or existing infrastructure within the road verge and maintain the flow, safety and efficiency of pedestrians, cyclists and vehicles.	AO1.1 Footpaths/pathways are located in the road verge and are provided for the hierarchy of the road and located and designed and constructed in accordance with Planning scheme policy SC5 – FNQROC Regional Development Manual.	AO1.1 The proposal is for a new Dwelling house on a residential lot. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	AO1.2 Kerb ramp crossovers are constructed in accordance with Planning scheme policy SC 5 – FNQROC Regional Development Manual.	AO1.2 The proposal is for a new Dwelling house on a residential lot. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	AO1.3 New pipes, cables, conduits or other similar infrastructure required to cross existing footpaths: a) are installed via trenchless	AO1.3 The proposal is for a new Dwelling house on a residential lot. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 methods; or b) where footpath infrastructure is removed to install infrastructure, the new section of footpath is installed to the standard detailed in the Planning scheme policy SC5 – FNQROC Regional Development Manual, and is not less than a 1.2 metre section. AO1.4 Where existing footpaths are damaged as a result of development, footpaths are reinstated ensuring: a) similar surface finishes are used; b) there is no change in level at joins of new and existing sections; c) new sections are matched to existing in terms of dimension and reinforcement. 	A01.4 The proposal is for a new Dwelling house on a residential lot. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	AO1.5 Decks, verandahs, stairs, posts and other structures located in the road reserve do not restrict or impede pedestrian movement on footpaths or change the level of the road verges.	AO1.5 The proposal is for a new Dwelling house on a residential lot. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
Accessibility structures			T
PO2 Development is designed to ensure it is accessible for people of all abilities and accessibility features do not	AO2.1 Accessibility structures are not located within the road reserve.	AO2.1 The proposal is for a new Dwelling house on a residential lot. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
impact on the efficient and safe use of footpaths.	AO2.2 Accessibility structures are designed in	AO2.2 The proposal is for a new Dwelling house on	□Compliant
	accordance with AS1428.3.	a residential lot. As such, this criterion is not applicable.	 ☑ N/A □ Performance Solution
	A02.3	A02.3	
	When retrofitting accessibility features in existing buildings, all structures and changes in grade are contained within the boundaries of the lot and not within the road reserve.	The proposal is for a new Dwelling house on a residential lot. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
Water supply			
PO3 An adequate, safe and reliable supply of potable, fire fighting and general use water is provided.	AO3.1 The premises is connected to Council's reticulated water supply system in accordance with the Design Guidelines set out in Section D6 of the Planning scheme policy SC5 – FNQROC Regional Development Manual;	AO3.1 Please refer to the Water pressure evaluation report submitted as part of the application.	⊠Compliant □N/A □Performance Solution
	AO3.2 Where a reticulated water supply system is not available to the premises, on site water storage tank/s with a minimum capacity of 10,000 litres of stored water, with a minimum 7,500 litre tank, with the balance from other sources (e.g. accessible swimming pool, dam etc.) and access to the tank/s for fire trucks is provided for each	AO3.2 Please refer to the Water pressure evaluation report submitted as part of the application.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	new house or other development. Tank/s are to be fitted with a 50mm ball valve with a camlock fitting and installed and connected prior to occupation of the house and sited to be visually unobtrusive.		
Treatment and disposal of effluent			
PO4 Provision is made for the treatment and disposal of effluent to ensure that there are no adverse impacts on water quality and no adverse ecological impacts as a result of the system or as a result of increasing the cumulative effect of systems in the locality.	AO4.1 The site is connected to Council's sewerage system and the extension of or connection to the sewerage system is designed and constructed in accordance with the Design Guidelines set out in Section D7 of the Planning scheme policy SC5 – FNQROC Regional Development Manual;	AO4.1 Please refer to the onsite sewerage evaluation report submitted as part of the application.	⊠Compliant □N/A □Performance Solution
	AO4.2 Where not in a sewerage scheme area, the proposed disposal system meets the requirements of Section 33 of the <i>Environmental Protection Policy (Water)</i> 1997 and the proposed on site effluent disposal system is designed in accordance with the <i>Plumbing and Drainage Act (2002)</i> .	AO4.2 Please refer to the onsite sewerage evaluation report submitted as part of the application.	⊠Compliant □N/A □Performance Solution
Stormwater quality			
PO5 Development is planned, designed, constructed and operated to avoid or minimise adverse impacts on stormwater quality in natural and	AO5.1 A connection is provided from the premises to Council's drainage system;	AO5.1 The proposed dwelling house will be connected to Council's drainage system.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
 developed catchments by: a) achieving stormwater quality objectives; b) protecting water environmental values; c) maintaining waterway hydrology. 	AO5.2 An underground drainage system is constructed to convey stormwater from the premises to Council's drainage system in accordance with the Design Guidelines set out in Sections D4 and D5 of the Planning scheme policy SC5 – FNQROC Regional Development Manual.	AO5.2 The proposed dwelling house will be connected to Council's drainage system.	⊠Compliant □N/A □Performance Solution
	 AO5.3 A stormwater quality management plan is prepared, and provides for achievable stormwater quality treatment measures meeting design objectives listed in Table 9.4.5.3.b and Table 9.4.5.3.c, reflecting land use constraints, such as: a) erosive, dispersive and/or saline soil types; b) landscape features (including landform); c) acid sulfate soil and management of nutrients of concern; d) rainfall erosivity. 	AO5.3 The proposed dwelling house will be connected to Council's drainage system.	⊠Compliant □N/A □Performance Solution
	AO5.4 Erosion and sediment control practices are designed, installed, constructed, monitored, maintained, and carried out in accordance with an erosion and sediment control plan.	AO5.4 Erosion and sediment control practices will be carried out in accordance with an erosion and sediment control plan.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	AO5.5 Development incorporates stormwater flow control measures to achieve the design objectives set out in Table 9.4.5.3.b and Table 9.4.5.3.c, including management of frequent flows, peak flows, and construction phase hydrological impacts.	AO5.5 The proposed dwelling house will be connected to Council's drainage system.	□Compliant □N/A □Performance Solution
Non-tidal artificial waterways			
 PO6 Development involving non-tidal artificial waterways is planned, designed, constructed and operated to: a) protect water environmental values; b) be compatible with the land use constraints for the site for protecting water environmental values; c) be compatible with existing tidal and non-tidal waterways; 	 AO6.1 Development involving non-tidal artificial waterways ensures: a) environmental values in downstream waterways are protected; b) any ground water recharge areas are not affected; c) the location of the waterway incorporates low lying areas of the catchment connected to an existing waterway; d) existing areas of ponded water are included. 	AO6.1 The proposal does not involve non-tidal artificial waterways. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
 d) perform a function in addition to stormwater management; e) achieve water quality objectives. 	 AO6.2 Non-tidal artificial waterways are located: a) outside natural wetlands and any associated buffer areas; b) to minimise disturbing soils or sediments; c) to avoid altering the natural hydrologic 	AO6.2 The proposal does not involve non-tidal artificial waterways. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	regime in acid sulfate soil and nutrient hazardous areas.		
	AO6.3	A06.3	
	Non-tidal artificial waterways located adjacent to, or connected to a tidal waterway by means of a weir, lock, pumping system or similar ensures: a) there is sufficient flushing or a tidal	The proposal does not involve non-tidal artificial waterways. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	range of		
	 b) >0.3 m; or c) any tidal flow alteration does not adversely impact on the tidal waterway; or 		
	d) there is no introduction of salt water into freshwater environments.		
	AO6.4	AO6.4	
	Non-tidal artificial waterways are designed and managed for any of the following end- use purposes:	The proposal does not involve non-tidal artificial waterways. As such, this criterion is not applicable.	□Compliant ⊠N/A
	a) amenity (including aesthetics), landscaping or recreation; or		□Performance Solution
	 b) flood management, in accordance with a drainage catchment management plan; or 		
	c) stormwater harvesting plan as part of an integrated water cycle management plan; or		
	d) aquatic habitat.		
	AO6.5	AO6.5	
	The end-use purpose of the non-tidal artificial waterway is designed and operated	The proposal does not involve non-tidal artificial waterways. As such, this criterion is	□Compliant





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	in a way that protects water environmental values.	not applicable.	⊠N/A □Performance Solution
	AO6.6 Monitoring and maintenance programs adaptively manage water quality to achieve relevant water quality objectives downstream of the waterway.	AO6.6 The proposal does not involve non-tidal artificial waterways. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	AO6.7 Aquatic weeds are managed to achieve a low percentage of coverage of the water surface area, and pests and vectors are managed through design and maintenance.	AO6.7 The proposal does not involve non-tidal artificial waterways. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
Wastewater discharge			
 PO7 Discharge of wastewater to waterways, or off site: a) meets best practice environmental management; b) is treated to: (i) meet water quality objectives for its receiving waters; (ii) avoid adverse impact on 	 AO7.1 A wastewater management plan is prepared and addresses: a) wastewater type; b) climatic conditions; c) water quality objectives; d) best practice environmental management. 	AO7.1 Please refer to the onsite sewerage evaluation report for further details.	⊠Compliant □N/A □Performance Solution
 (ii) avoid adverse impact on ecosystem health or waterway health; (iii) maintain ecological processes, riparian vegetation and 	AO7.2 The waste water management plan is managed in accordance with a waste management hierarchy that: a) avoids wastewater discharge to	AO7.2 Please refer to the onsite sewerage evaluation report for further details.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
waterway integrity; (iv) offset impacts on high ecological value waters.	 waterways; or b) if wastewater discharge cannot practicably be avoided, minimises wastewater discharge to waterways by re-use, recycling, recovery and treatment for disposal to sewer, surface water and ground water. 		
	AO7.3 Wastewater discharge is managed to avoid or minimise the release of nutrients of concern so as to minimise the occurrence, frequency and intensity of algal blooms.	AO7.3 Please refer to the onsite sewerage evaluation report for further details.	⊠Compliant □N/A □Performance Solution
	 AO7.4 Development in coastal catchments avoids or minimises and appropriately manages soil disturbance or altering natural hydrology and: a) avoids lowering ground water levels where potential or actual acid sulfate soils are present; b) manages wastewater so that: (i) the pH of any wastewater discharges is maintained between 6.5 and 8.5 to avoid mobilisation of acid, iron, aluminium and other metals; (ii) holding times of neutralised wastewater ensures the flocculation and removal of any dissolved iron prior to release; (iii) visible iron floc is not present in any 	AO7.4 Please refer to the onsite sewerage evaluation report for further details.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 discharge; (iv) precipitated iron floc is contained and disposed of; (v) wastewater and precipitates that cannot be contained and treated for discharge on site are removed and disposed of through trade waste or another lawful method. 		
Electricity supply			
PO8 Development is provided with a source of power that will meet its energy needs.	AO8.1 A connection is provided from the premises to the electricity distribution network; AO8.2 The premises is connected to the electricity distribution network in accordance with the Design Guidelines set out in Section D8 of the Planning scheme policy SC5 – FNQROC Regional Development Manual.	 AO8.1 The dwelling house will be connected with a power source from the electricity distribution network. AO8.2 The dwelling house will be connected with a power source from the electricity distribution network.	 Compliant N/A Performance Solution Compliant N/A Performance Solution
PO9 Development incorporating pad- mount electricity infrastructure does not cause an adverse impact on amenity.	 AO9.1 Pad-mount electricity infrastructure is: a) not located in land for open space or sport and recreation purposes; b) screened from view by landscaping or fencing; c) accessible for maintenance. 	AO9.1 A pad-mount electricity infrastructure is not proposed as part of the application.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	AO9.2 Pad-mount electricity infrastructure within a building, in a Town Centre is designed and located to enable an active street frontage.	AO9.2 A pad-mount electricity infrastructure is not proposed as part of the application.	□Compliant ⊠N/A □Performance Solution
Telecommunications			
PO10Development is connected to a telecommunications service approved by the relevant telecommunication regulatory authority.PO11Provision is made for future telecommunications services (e.g. fibre optic cable).	 AO10 The development is connected to telecommunications infrastructure in accordance with the standards of the relevant regulatory authority. AO11 Conduits are provided in accordance with Planning scheme policy SC5 – FNQROC Regional Development Manual. 	AO10 The dwelling house will be connected to telecommunication infrastructure. AO11 The dwelling house will be connected to telecommunication infrastructure.	 ☑ Compliant ☑ N/A ☑ Performance Solution ☑ Compliant ☑ N/A
Road construction			□Performance Solution
 PO12 The road to the frontage of the premises is constructed to provide for the safe and efficient movement of: a) pedestrians and cyclists to and from the site; b) pedestrians and cyclists adjacent to the site; 	AO12.1 The road to the frontage of the site is constructed in accordance with the Design Guidelines set out in Sections D1 and D3 of the Planning scheme policy SC5 – FNQROC Regional Development Manual, for the particular class of road, as identified in the road hierarchy.	AO12.1 Road construction is not proposed or required as part of the application.	□Compliant ⊠N/A □Performance Solution
c) vehicles on the road adjacent to the site;	AO12.2 There is existing road, kerb and channel for the full road frontage of the site.	AO12.2 Road construction is not proposed or required as part of the application.	□Compliant ⊠N/A





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
d) vehicles to and from the site;			□Performance Solution
e) emergency vehicles.	A012.3	A012.3	
	Road access minimum clearances of 3.5 metres wide and 4.8 metres high are provided for the safe passage of emergency vehicles.	Road construction is not proposed or required as part of the application.	□Compliant ⊠N/A □Performance Solution
Alterations and repairs to public utility s	ervices		
P013	A013	A013	
Infrastructure is integrated with, and efficiently extends, existing networks.	Development is designed to allow for efficient connection to existing infrastructure networks.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
PO14	A014.1	A014.1	
Development and works do not affect the efficient functioning of public utility mains, services or installations.	Public utility mains, services and installations are not required to be altered or repaired as a result of the development;	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	A014.2	A014.2	
	Public utility mains, services and installations are altered or repaired in association with the works so that they continue to function and satisfy the relevant Design Guidelines set out in Section D8 of the Planning scheme policy SC5 – FNQROC Regional Development Manual.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
Construction management			
PO15	A015	A015	
Work is undertaken in a manner which	Works include, at a minimum:		⊠Compliant





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
minimises adverse impacts on vegetation that is to be retained.	 a) installation of protective fencing around retained vegetation during construction; b) erection of advisory signage; c) no disturbance, due to earthworks or storage of plant, materials and equipment, of ground level and soils below the canopy of any retained vegetation; d) removal from the site of all declared noxious weeds. 	The proposed dwelling house will be undertaken to minimise adverse impacts on existing vegetation over the site.	□N/A □Performance Solution
PO16 Existing infrastructure is not damaged by construction activities.	AO16 Construction, alterations and any repairs to infrastructure is undertaken in accordance with the Planning scheme policy SC5 – FNQROC Regional Development Manual.	AO16 The proposed dwelling house will be undertaken in accordance with the planning scheme policy.	⊠Compliant □N/A □Performance Solution
For assessable development			
High speed telecommunication infrastr	ucture		
PO17 Development provides infrastructure to facilitate the roll out of high speed telecommunications infrastructure.	AO17 No acceptable outcomes are prescribed.	AO17 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
Trade waste			
PO18 Where relevant, the development is capable of providing for the storage, collection treatment and disposal of trade waste such that:	AO18 No acceptable outcomes are prescribed.	AO18 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
a) off-site releases of contaminants do not occur;			
 b) the health and safety of people and the environment are protected; 			
 c) the performance of the wastewater system is not put at risk. 			
Fire services in developments accessed I	by common private title		
PO19 Hydrants are located in positions that will enable fire services to access water safely, effectively and efficiently.	AO19.1 Residential streets and common access ways within a common private title places hydrants at intervals of no more than 120 metres and at each intersection. Hydrants may have a single outlet and be situated above or below ground.	AO19.1 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	AO19.2 Commercial and industrial streets and access ways within a common private title serving commercial properties such as factories and warehouses and offices are provided with above or below ground fire hydrants located at not more than 90 metre intervals and at each intersection. Above ground fire hydrants have dual-valved outlets.	AO19.2 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
PO20	AO20	AO20	
Hydrants are suitable identified so that fire services can locate them at all hours.	No acceptable outcomes are prescribed.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution

3.3 Vegetation management Code

3.3.1 Purpose

PURPOSE & OVERALL OUTCOMES	P	PROPO	SAL	COMPLIANCE
1) The purpose of the Vegetation n	nanagement code is achieved thro	ough tl	ne overall outcomes.	
2) The purpose of the code will be	achieved through the following or	verall o	outcomes:	
 a sustainable manner; c) significant trees are maintain d) biodiversity and ecological maintained; e) habitats for rare, threatene flora and fauna are protected 	does occur it is undertaken in ined and protected; values are protected and d and endemic species of ed and maintained; enic amenity is protected and	a) b) c) d) e) f)	The proposed dwelling house ensures vegetation is protected from inappropriate damages; Where vegetation damage is required, it will be undertaken in a sustainable manner; The proposal does not impact upon significant trees; The proposed dwelling house is of a built form and scale that will protect and maintain biodiversity and ecological values; The proposed dwelling house does not impact upon rare, threatened and endemic species of flora and fauna; The proposed dwelling house protects the landscape character and scenic amenity; The subject site is not identified to contain heritage values.	⊠Compliant □N/A □Performance Solution





3.3.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
For self-assessable and assessable deve	lopment		
 PO1 Vegetation is protected to ensure that: a) the character and amenity of the local area is maintained; b) vegetation damage does not result in fragmentation of habitats; 	AO1.1 Vegetation damage is undertaken by a statutory authority on land other than freehold land that the statutory authority has control over;	AO1.1 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
 c) vegetation damage is undertaken in a sustainable manner; d) the Shire's biodiversity and ecological values are maintained and protected; e) vegetation of historical, cultural 	AO1.2 Vegetation damage is undertaken by or on behalf of the local government on land controlled, owned or operated by the local government;	AO1.2 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
 and / or visual significance is retained; f) vegetation is retained for erosion prevention and slope stabilisation. 	 AO1.3 Vegetation damage, other than referenced in AO1.1 or AO1.2 is the damage of: a) vegetation declared as a pest pursuant to the Land Protection (Pest and Stock Route Management) Act 2002; or b) vegetation identified within the local govern- ment's register of declared plants pursuant to the local government's local laws; or 	AO1.3 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 c) vegetation is located within a Rural zone and the trunk is located within ten metres of an existing building; or 		
	 vegetation is located within the Conservation zone or Environmental management zone and the trunk is located within three metres of an existing or approved structure, not including a boundary fence;. 		
	A01.4	A01.4	
	 Vegetation damage that is reasonably necessary for carrying out work that is: a) authorised or required under legislation or a local law; b) specified in a notice served by the local government or another regulatory authority; 	The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
	A01.5	A01.5	
	Vegetation damage for development where the damage is on land the subject of a valid development approval and is necessary to give effect to the development approval;	The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
	A01.6	A01.6	
	Vegetation damage is in accordance with an approved Property Map of Assessable Vegetation issued under the Vegetation Management Act 1999;	The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	AO1.7 Vegetation damage is essential to the maintenance of an existing fire break;	AO1.7 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
	AO1.8 Vegetation damage is essential to prevent interference to overhead service cabling;	AO1.8 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
	AO1.9 Vegetation damage is for an approved Forest practice, where the lot is subject to a scheme approved under the Vegetation Management Act 1999;	AO1.9 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
	AO1.10 Vegetation damage is undertaken in accordance with section 584 of the <i>Sustainable Planning Act 2009.</i>	AO1.10 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
	A01.11 Vegetation damage where it is necessary to remove one tree in order to protect an adjacent more significant tree (where they are growing close to one another).	AO1.11 The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
		dwelling house.	
	A01.12	A01.12	
	Private property owners may only remove dead, dying, structurally unsound vegetation following receipt of written advice from, at minimum, a fully qualified Certificate V Arborist. A copy of the written advice is to be submitted to Council for its records, a minimum of seven business days prior to the vegetation damage work commencing.	The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
PO2	A02.1	A02.1	
Vegetation damaged on a lot does not result in a nuisance	Damaged vegetation is removed and disposed of at an approved site;	The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
	AO2.2	A02.1	
	Damaged vegetation is mulched or chipped if used onsite.	The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution
For assessable development			
PO3	AO3	A03	
Vegetation damage identified on the Places of significance overlay lot does not result in a negative impact on the site's heritage values.	No acceptable outcomes are prescribed.	The proposed dwelling house requires minimal site works as it will be constructed over the existing house pad. Vegetation clearing will not be required to support the dwelling house.	□Compliant ⊠N/A □Performance Solution





4 Overlay Codes

- 4.1 Acid Sulfate Soils Overlay Code
- 4.1.1 Purpose

PURPOSE & OVERALL OUTCOMES	PROPOSAL	COMPLIANCE			
 The purpose of the acid sulfate soils overlay code is to: implement the policy direction in the Strategic Framework, in particular: Theme 2: Environment and landscape values, Element 3.5.4 Coastal zones. Theme 3: Natural resource management, Element 3.6.2 land and catchment management, Element 3.6.3 Primary production, forestry and fisheries. 					
2) enable an assessment of whether development is suitable on la	and within the Acid sulfate soils overlay sub-categories.				
3) The purpose of the code will be achieved through the following	g overall outcomes:				
 a) Development ensures that the release of any acid and associated metal contaminant is avoided by not disturbing acid sulfate soils when excavating, removing soil or extracting ground water or filling land; b) Development ensures that disturbed acid sulfate soils, or drainage waters, are treated and, if required, on-going management practices are adopted that minimise the potential for environmental harm from acid sulfate soil. 	 A) The proposal will ensure any release of acid and associated metal contaminants is avoided; B) The proposal ensures any found disturbed acid sulfate soils, or drainage waters are treated to minimise environmental harm. 	⊠Compliant □N/A □Performance Solution			





4.1.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
For Assessable Development			
PO1	A01.1	A01.1	
The extent and location of potential or actual acid sulfate soils is accurately identified	No excavation or filling occurs on the site.	The proposed dwelling house requires minimal site works as the house will be located on the existing house pad.	⊠Compliant □N/A □Performance Solution
	AO1.2 An acid sulfate soils investigation is	AO1.2 The proposed dwelling house requires	
	undertaken.	minimal site works as the house will be located on the existing house pad.	 ☑Compliant ☑N/A ☑Performance Solution
PO2	AO2.1	A02.1	
Development avoids disturbing potential acid sulfate soils or actual acid sulfate soils, or is managed to avoid or minimise the release of acid and metal contaminants.	 The disturbance of potential acid sulfate soils or actual acid sulfate soils is avoided by: a) not excavating, or otherwise removing, soil or sediment identified as containing potential or actual acid sulfate soils; b) not permanently or temporarily extracting groundwater that results in the aeration of previously saturated acid sulfate soils; c) not undertaking filling that results in: d) actual acid sulfate soils being moved below the water table; e) previously saturated acid sulfate soils being aerated. 	The proposed dwelling house requires minimal site works as the house will be located on the existing house pad.	 ☑ Compliant ☑ N/A ☑ Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 AO2.2 The disturbance of potential acid sulfate soils or actual acid sulfate soils is undertaken in accordance with an acid sulfate soils management plan and avoids the release of metal contaminants by: a) neutralising existing acidity and preventing the generation of acid and metal contaminants; b) preventing the release of surface or groundwater flows containing acid and metal contaminants into the environment; c) preventing the in situ oxidisation of potential acid sulfate soils and actual acid sulfate soils through ground water level management; d) appropriately treating acid sulfate soils before disposal occurs on or off site; e) documenting strategies and reporting requirements in an acid sulfate soils environment plan. 	AO2.2 The proposed dwelling house requires minimal site works as the house will be located on the existing house pad.	⊠Compliant □N/A □Performance Solution
PO3 No environmental harm is caused as a result of exposure to potential acid sulfate soils or actual acid sulfate soils.	AO3 No acceptable outcomes are prescribed.	AO3 The proposed dwelling house requires minimal site works as the house will be located on the existing house pad.	⊠Compliant □N/A □Performance Solution





4.2 Bushfire Hazard Overlay Code

4.2.1 Purpose

PURPO	SE & OVERALL OUTCOMES	PROPOSAL	COMPLIANCE
1) a) b) 2)	The purpose of the Bushfire overlay code is to: implement the policy direction in the Strategic Framewor (i) Theme 1 Settlement pattern: Element 3.4.7 Mit (ii) Theme 6 Infrastructure and transport: Element enable an assessment of whether development is suitable The purpose of the code will be achieved through the fol	igation of hazards; 3.9.2 Energy. e on land within the Bushfire risk overlay sub-categories.	
2) a) b) c) d) e)	 development avoids the establishment or intensification of vulnerable activities within or near areas that are subject to bushfire hazard; development is designed and located to minimise risks to people and property from bushfires; bushfire risk mitigation treatments are accommodated in a manner that avoids or minimises impacts on the natural environment and ecological processes; development involving the manufacture or storage of hazardous materials does not increase the risk to public safety or the environment in a bushfire event; development contributes to effective and efficient disaster management response and recovery capabilities. 	 a) The proposed dwelling house has been designed to avoid areas that are subject to bushfire hazard; b) The proposal strategically located to minimise the threat of bushfire; c) The development will incorporate bushfire risk mitigation treatments; d) The development does not involve the manufacture or storage of hazardous materials; e) The proposal will ensure the dwelling house has an effective and efficient disaster management response capability. 	⊠Compliant □N/A □Performance Solution





4.2.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE			
For self-assessable and assessable deve	lopment					
Compatible development						
PO1 A vulnerable use is not established or materially intensified within a bushfire hazard area (bushfire prone area) unless there is an overriding need or other exceptional circumstances.	AO1 Vulnerable uses are not established or expanded	AO1 The proposed dwelling house is appropriately located to manage the bushfire risk applicable over the site. Please refer to the bushfire report submitted as part of the application.	⊠Compliant □N/A □Performance Solution			
PO2 Emergency services and uses providing community support services are able to function effectively during and immediately after a bushfire hazard event.	AO2 Emergency Services and uses providing community support services are not located in a bushfire hazard sub-category and have direct access to low hazard evacuation routes.	AO2 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution			
PO3 Development involving hazardous materials manufactured or stored in bulk is not located in bushfire hazard sub-category.	AO3 The manufacture or storage of hazardous material in bulk does not occur within bushfire hazard sub-category.	AO3 The proposal does not involve the manufacture or storage of hazardous materials.	□Compliant ⊠N/A □Performance Solution			
Development design and separation from bushfire hazard – reconfiguration of lots						
PO4.1 Where reconfiguration is undertaken in an urban area or is for urban purposes or smaller scale rural residential purposes, a separation distance from hazardous vegetation is provided to	AO4.1 No new lots are created within a bushfire hazard sub-category.	AO4.1 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution			





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
achieve a radiant heat flux level of 29 kW/m ² at the edge of the proposed lot(s).			
PO4.2	AO4.2	AO4.2	
Where reconfiguration is undertaken for other purposes, a building envelope of reasonable dimensions is provided on each lot which achieves radiant heat flux level of 29kW/m2 at any point.	 Lots are separated from hazardous vegetation by a distance that: a) achieves radiant heat flux level of 29kW/m² at all boundaries; and b) is contained wholly within the development site. 	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
PO5	A05.1	A05.1	
Where reconfiguration is undertaken in an urban area or is for urban purposes, a constructed perimeter road with reticulated water supply is established between the lots and the hazardous vegetation and is readily accessible at all times for urban fire fighting vehicles. The access is available for both fire fighting and maintenance/defensive works.	 Lot boundaries are separated from hazardous vegetation by a public road which: a) has a two lane sealed carriageway; b) contains a reticulated water supply; c) is connected to other public roads at both ends and at intervals of no more than 500m; d) accommodates geometry and turning radii in accordance with Queensland Fire and Emergency Services' Fire Hydrant and Vehicle Access Guidelines; e) has a minimum of 4.8m vertical clearance above the road; f) is designed to ensure hydrants and water access points are not located within parking bay allocations; and g) incorporates roll-over kerbing. 	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	A05.2	A05.2	
	Fire hydrants are designed and installed in accordance with AS2419.1 2005, unless otherwise specified by the relevant water entity.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
PO6	A06	A06	
Where reconfiguration is undertaken for smaller scale rural residential purposes, either a constructed	Lot boundaries are separated from hazardous vegetation by a public road or fire trail which has:	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A
perimeter road or a formed, all weather fire trail is established	a) a reserve or easement width of at least 20m;		□Performance Solution
between the lots and the hazardous vegetation and is readily accessible at all times for the type of fire fighting vehicles servicing the area.	b) a minimum trafficable (cleared and formed) width of 4m capable of accommodating a 15 tonne vehicle and which is at least 6m clear of vegetation;		
The access is available for both fire fighting and maintenance/hazard	 c) no cut or fill embankments or retaining walls adjacent to the 4m wide trafficable path; 		
reduction works.	d) a minimum of 4.8m vertical clearance;		
	e) turning areas for fire-fighting appliances in accordance with Queensland Fire and Emergency Services' Fire Hydrant and Vehicle Access Guidelines;		
	f) a maximum gradient of 12.5%;		
	g) a cross fall of no greater than 10 degrees;		
	 h) drainage and erosion control devices in accordance with the standards prescribed in a planning scheme policy; 		
	i) vehicular access at each end which is		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 connected to the public road network at intervals of no more than 500m; j) designated fire trail signage; k) if used, has gates locked with a system authorised by Queensland Fire and Emergency Services; and l) if a fire trail, has an access easement that is granted in favour of Council and Queensland Fire and Emergency Services. 		
P07	A07	A07	
Where reconfiguration is undertaken for other purposes, a formed, all weather fire trail is provided between the hazardous vegetation and either the lot boundary or building envelope, and is readily accessible at all times for the type of fire fighting vehicles servicing the area. However, a fire trail will not be required where it would not serve a practical fire management purpose.	 Lot boundaries are separated from hazardous vegetation by a public road or fire trail which has: a) a reserve or easement width of at least 20m; b) a minimum trafficable (cleared and formed) width of 4m capable of accommodating a 15 tonne vehicle and which is at least 6m clear of vegetation; c) no cut or fill embankments or retaining walls adjacent to the 4m wide trafficable path; 	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	 d) a minimum of 4.8m vertical clearance; e) turning areas for fire-fighting appliances in accordance with Queensland Fire and Emergency Services' Fire Hydrant and Vehicle Access Guidelines; f) a maximum gradient of 12.5%; g) a cross fall of no greater than 10 		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
PO8 The development design responds to the potential threat of bushfire and establishes clear evacuation routes which demonstrate an acceptable or tolerable risk to people.	 degrees; h) drainage and erosion control devices in accordance with the standards prescribed in a planning scheme policy; i) vehicular access at each end which is connected to the public road network; j) designated fire trail signage; k) if used, has gates locked with a system authorised by Queensland Fire and Emergency Services; and l) if a fire trail, has an access easement that is granted in favour of Council and Queensland Fire and Emergency Services. AO8 The lot layout: a) minimises the length of the development perimeter exposed to, or adjoining hazardous vegetation; b) avoids the creation of potential bottleneck points in the movement network; c) establishes direct access to a safe assembly d) /evacuation area in the event of an approaching bushfire; and 	AO8 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	e) ensures roads likely to be used in the event of a fire are designed to minimise traffic congestion.		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
PO9 Critical infrastructure does not increase the potential bushfire hazard.	AO9 Critical or potentially hazardous infrastructure such as water supply, electricity, gas and telecommunications are	AO9 The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A
	placed underground.		□Performance Solution
Development design and separation from	m bushfire hazard – material change of use		
PO10	AO10	AO10	
Development is located and designed to ensure proposed buildings or building envelopes achieve a radiant heat flux level at any point on the building or envelope respectively, of: a) 10kW/m ² where involving a vulnerable use; or b) 29kW/m ² otherwise. The radiant heat flux level is achieved	 Buildings or building envelopes are separated from hazardous vegetation by a distance that: a) achieves a radiant heat flux level of at any point on the building or envelope respectively, of 10kW/m² for a vulnerable use or 29kW/m² b) otherwise; and c) is contained wholly within the 	The proposed dwelling house is appropriately located and separated from the bushfire risk. Please refer to the bushfire report submitted as part of the application.	⊠Compliant □N/A □Performance Solution
by separation unless this is not practically achievable.	development site.		
PO11	A011	A011	
A formed, all weather fire trail is provided between the hazardous vegetation and the site boundary or building envelope, and is readily accessible at all times for the type of fire fighting vehicles servicing the area.	 Development sites are separated from hazardous vegetation by a public road or fire trail which has: a) a reserve or easement width of at least 20m; 	The proposed dwelling house is appropriately located and separated from the bushfire risk. Please refer to the bushfire report submitted as part of the application.	⊠Compliant □N/A □Performance Solution
However, a fire trail will not be required where it would not serve a	 b) a minimum trafficable (cleared and formed) width of 4m capable of accommodating a 15 tonne vehicle and which is at least 6m clear of vegetation; 		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
practical fire management purpose.	 c) no cut or fill embankments or retaining walls adjacent to the 4m wide trafficable path; 		
	d) a minimum of 4.8m vertical clearance;		
	 e) turning areas for fire-fighting appliances in accordance with Queensland Fire and Emergency Services' Fire Hydrant and Vehicle Access Guidelines; 		
	f) a maximum gradient of 12.5%;		
	g) a cross fall of no greater than 10 degrees;		
	 h) drainage and erosion control devices in accordance with the standards prescribed in a planning scheme policy; 		
	 vehicular access at each end which is connected to the public road network which is connected to the public road network at intervals of no more than 500m; 		
	j) designated fire trail signage;		
	 k) if used, has gates locked with a system authorised by Queensland Fire and Emergency Services; and 		
	 if a fire trail, has an access easement that is granted in favour of Council and Queensland Fire and Emergency Services. 		
All Development			





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
PO12	A012	A012	
All premises are provided with vehicular access that enables safe evacuation for occupants and easy access by fire fighting appliances.	 Private driveways: a) do not exceed a length of 60m from the street to the building; b) do not exceed a gradient of 12.5%; c) have a minimum width of 3.5m; d) have a minimum of 4.8m vertical clearance; e) accommodate turning areas for fire-fighting appliances in accordance with Queensland Fire and Emergency Services' Fire Hydrant and Vehicle Access Guidelines; and f) serve no more than 3 dwellings or buildings. 	The proposed dwelling house is appropriately located and separated from the bushfire risk. Please refer to the bushfire report submitted as part of the application.	 □ Compliant □ N/A □ Performance Solution
P013	A013	A013	
Development outside reticulated water supply areas includes a dedicated static supply that is available solely for fire fighting purposes and can be accessed by fire fighting appliances.	 A water tank is provided within 10m of each building (other than a class 10 building) which: a) is either below ground level or of non-flammable construction; b) has a take off connection at a level that 	The proposed dwelling house is appropriately located and separated from the bushfire risk. Please refer to the bushfire report submitted as part of the application.	⊠Compliant □N/A □Performance Solution
	 allows the following dedicated, static water supply to be left available for access by fire fighters: i. 10,000l for residential buildings ii. 45,000l for industrial buildings; and iii. 20,000l for other buildings; c) includes shielding of tanks and pumps in 		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 accordance with the relevant standards; d) includes a hardstand area allowing medium rigid vehicle (15 tonne fire appliance) access within 6m of the tank; e) is provided with fire brigade tank fittings 50mm ball valve and male camlock coupling and, if underground, an access hole of 200mm (minimum) to accommodate suction lines; and f) is clearly identified by directional 		
2014	signage provided at the street frontage.	1014	
PO14 Landscaping does not increase the potential bushfire risk.	AO14 Landscaping uses species that are less likely to exacerbate a bushfire event, and does not increase fuel loads within separation areas.	AO14 The proposed dwelling house is appropriately located and separated from the bushfire risk. Please refer to the bushfire report submitted as part of the application.	⊠Compliant □N/A □Performance Solution
PO15 The risk of bushfire and the need to	AO15 Bushfire risk mitigation treatments do not	AO15 The proposed dwelling house is appropriately	⊠Compliant
mitigate that risk is balanced against other factors (such as but not limited to, biodiversity or scenic amenity).	have a significant impact on the natural environment or landscape character of the locality where this has value.	located and separated from the bushfire risk. Please refer to the bushfire report submitted as part of the application.	□N/A □Performance Solution





4.3 Hillslopes Overlay Code

4.3.1 Purpose

PURPOSE & OVERALL OUTCOMES P	ROPOSAL	COMPLIANCE
 The purpose of the Hillslopes overlay code is to: a) implement the policy direction in the Strategic Framewore (i) Theme 1 - Settlement pattern: Element 3.4.7 Mitig (ii) Theme 2 – Environment and landscape values: Element b) enable an assessment of whether development is suitable The purpose of the code will be achieved through the following over the pattern of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following over the purpose of the code will be achieved through the following the purpose of the code will be achieved through the following the purpose of the code will be achieved through the following the purpose of the code will be achieved through the following the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of the code will be achieved through the purpose of t	gation of hazards; ement 3.5.5 Scenic amenity. le on land within the Hillslopes sub-categories.	
 a) development on hillslopes is safe, serviceable and accessible; b) the ecological values, landscape character and visual quality of the hillslopes are protected from development so as to retain the scenic backdrop to the region; c) Development on hillslopes is appropriate, having regard to the topographic constraints and environmental characteristics of the land; d) Development responds to the constraints of the site including gradient and slope stability; e) Works do not involve complex engineering solutions. 	 f) The proposal is for a new dwelling house on an approved subdivision over the site. g) The proposal does not adversely impact upon the ecological values of the site. h) The proposed dwelling house is located on a house pad which is appropriate with regards to topographic constraints. i) The proposed dwelling house is located on a house pad which is appropriate with regards to topographic constraints. j) The proposal does not involve complex engineering solutions. 	⊠Compliant □N/A □Performance Solution





4.3.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE		
For self-assessable development	For self-assessable development				
PO1 The landscape character and visual amenity quality of hillslopes areas is	AO1 Development is located on parts of the site that are not within the Hillslopes constraint	AO1 The proposal is not located on parts of the site that are not within the Hillslopes	⊠Compliant		
retained to protect the scenic backdrop to the region.	sub- category as shown on the Hillslopes overlay Maps contained in schedule 2.	constraints sub-category.	□N/A □Performance Solution		
For assessable development					
PO2 The landscape character and visual amenity quality of hillslopes areas is retained to protect the scenic backdrop to the region.	AO2.1 Development does not occur on land with a gradient in excess of 1 in 6 (16.6%)	AO2.1 The proposed dwelling house is on a house pad which does not occur in excess of 1 in 6.	⊠Compliant □N/A □Performance Solution		
	AO2.2 Where development on land steeper than 1 in 6 (16.6%) cannot be avoided, development follows the natural contours of the site.	AO2.2 The proposed dwelling house is on a house pad which does not occur in excess of 1 in 6.	⊠Compliant □N/A □Performance Solution		
	 AO2.3 Access ways and driveways are: a) constructed with surface materials that blend with the surrounding environment; b) landscaped with dense planting to minimise the visual impact of the construction; c) provided with erosion control measures immediately after construction. 	AO2.3 The proposed access ways and driveway will be constructed with surface materials that blend with the surrounding environment.	⊠Compliant □N/A □Performance Solution		





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 AO2.4 The clearing or disturbance of vegetation is limited to clearing and disturbance that: a) is necessary for the construction of driveways; b) is necessary to contain the proposed development; c) minimises canopy clearing or disturbance; d) minimises riparian clearing or disturbance. 	AO2.4 The proposal is for a new dwelling house upon an approved and carried out subdivision and results in minimal vegetation clearing.	⊠Compliant □N/A □Performance Solution
	AO2.5 On land with slopes greater than 1 in 6 (16.6%) or greater, alternative construction methods to concrete slab on ground are utilised (i.e. split level or post and beam constructed buildings that minimise modification to the natural terrain of the land).	AO2.5 The proposed dwelling house is on a house pad which does not occur in excess of 1 in 6.	⊠Compliant □N/A □Performance Solution
	AO2.6 Development does not alter the sky line.	AO2.6 The proposal does not alter the sky line.	⊠Compliant □N/A □Performance Solution
	AO2.7 Buildings and structures: a) are finished predominantly in the	A02.7	⊠Compliant





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 following exterior colours or surfaces: i. moderately dark to darker shades of olive green, brown, green, blue, or charcoal; or (i) moderately dark to darker wood stains that blend with the colour and hues of the surrounding vegetation and landscape; b) are not finished in the following exterior colours or surfaces: i. pastel or terracotta colours, reds, yellows, shades of white or beige, or other bright colours that do not blend with the surrounding vegetation and landscape; ii. reflective surfaces. 	The proposed dwelling house will be finished with exterior colours in accordance with the acceptable outcome.	□N/A □Performance Solution
	AO2.8 Exterior colour schemes limit the use of white or other light colours to exterior trim and highlighting of architectural features	AO2.8 The proposed dwelling house will be finished with exterior colours in accordance with the acceptable outcome.	⊠Compliant □N/A □Performance Solution
	AO2.9 Areas between the first floor (including outdoor deck areas) and ground level are screened from view.	AO2.9 Any screening requirements can be conditioned by Council accordingly.	⊠Compliant □N/A □Performance Solution
	AO2.10 Recreational or ornamental features (including tennis courts, ponds or swimming	AO2.10	□Compliant ⊠N/A





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 pools) do not occur on land: a) with a gradient of 1 in 6 (16.6%) or more; b) are designed to be sited and respond to the natural constraints of the land and require minimal earthworks. 	The proposed dwelling house including the pool is on a house pad which does not occur in excess of 1 in 6.	□Performance Solution
 PO3 Excavation or filling does not have an adverse impact on the amenity, safety, stability or function of the site or adjoining premises through: a) loss of privacy; b) loss of access to sunlight; c) intrusion of visual or overbearing impacts; d) complex engineering solutions. 	 AO3 Excavation or fill: a) is not more than 1.2 metres in height for each batter or retaining wall; b) is setback a minimum of 2 metres from property boundaries; c) is stepped with a minimum 2 metre wide berm to incorporate landscaping in accordance with Planning scheme policy SC6.7 – Landscaping; d) does not exceed a maximum of 3 batters and 3 berms (i.e. not greater than 3.6 metres in height) on any one lot. 	AO3 The proposal does not require excavation or fill nominated in the acceptable outcome.	□Compliant ⊠N/A □Performance Solution
Lot reconfiguration			
PO4	AO4.1	A04.1	
For development that involves reconfiguring a lot, lot layout and design is responsive to the natural constraints of the land and each lot is capable of being used for its intended purpose.	 The frontage and depth of all lots is of sufficient width to: a) allow driveways to follow the natural contours of the site and not exceed a gradient of 1 in 6 (16.6%); b) accommodate any changes in gradient between the road and lot within the lot 	The proposal is for a new dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	boundary and not within the road reserve.		
	A04.2	AO4.2	
	Development does not create new lots containing land of greater than 1 in 6 (16.6%), except where a rectangular area of land of lesser grade is contained within the new lots to accommodate the intended land use, with the balance left in its natural state to the greatest extent possible.	The proposal is for a new dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	AO4.3	A04.3	
	Development does not alter ridgelines.	The proposal is for a new dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution
	AO4.4	A04.4	
	Lots are designed to ensure rooflines of future buildings and structures do not protrude above a ridgeline.	The proposal is for a new dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





4.4 Natural Areas Overlay Code

4.4.1 Purpose

PU	RPO	SE &	OVER	ALL OUTCOMES	PROPOSAL	COMPLIANCE
1)	The	e purp	oose o	f the Natural areas overlay code is to:		
		a)	imple	ment the policy direction in the Strategic Framew	vork, in particular:	
			(i)	Theme 2: Environment and landscape values, E	lement 3.5.3 Biodiversity, Element	
				3.5.4 Coastal zones;		
			(ii)	Theme 3: Natural resource management Element fisheries.	ent 3.6.2 Land and catchment management, Element 3.6.3 Primary	production, forestry and
		b)	enabl	e an assessment of whether development is suita	able on land within the Biodiversity area overlay sub-categories.	
2)	The	e purp	oose o	f the code will be achieved through the following	g overall outcomes:	
	a)	deve	elopm	ent is avoided within:	a) The proposed dwelling house is located outside	
		i.		reas containing matters of state environmental gnificance (MSES);	of state environmental significance areas, natural areas, wetlands and wetland buffers and waterways;	⊠Compliant
		ii.	0	ther natural areas;	b) The proposed development results in minimal	□N/A
		iii.	W	vetlands and wetland buffers;	site works and has been designed to protect the	□Performance Solution
		iv.	W	vaterways and waterway corridors.	environmental significance characteristics of the site.	
	b)	whe	re dev	velopment cannot be avoided, development:		
		i.		rotects and enhances areas containing matters f state environmental significance;		
		ii.	р	rovides appropriate buffers;		
		iii.	h: sp	rotects the known populations and supporting abitat of rare and threatened flora and fauna pecies, as listed in the relevant State and ommonwealth legislation;		
		iv.	o m	nsures that adverse direct or indirect impacts n areas of environmental significance are ninimised through design, siting, operation, nanagement and mitigation measures;		





PURPO	SE & OV	/ERALL OUTCOMES	PROPOSAL	COMPLIANCE
	v.	does not cause adverse impacts on the integrity and quality of water in upstream or downstream catchments, including the Great Barrier Reef World Heritage Area;		
	vi.	protects and maintains ecological and hydrological functions of wetlands, waterways and waterway corridors;		
	vii.	enhances connectivity across barriers for aquatic species and habitats; rehabilitates degraded areas to provide improved habitat condition, connectivity, function and extent;		
	viii.	protects areas of environmental significance from weeds, pests and invasive species.		
c)	where	gic rehabilitation is directed to areas on or off site, it is possible to achieve expanded habitats and sed connectivity.		

4.4.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE						
For self-assessable and assessable dev	For self-assessable and assessable development								
Protection of matters of environmenta	al significance								
P01	A01.1	A01.1							
Development protects matters of environmental significance.	Development avoids significant impact on the relevant environmental values.	The proposal avoids significant impact on the relevant environmental values.	⊠Compliant □N/A □Performance Solution						





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	AO1.2 A report is prepared by an appropriately qualified person demonstrating to the satisfaction of the assessment manager, that the development site does not contain any matters of state and local environmental significance.	AO1.2 The proposal avoids significant impact on the relevant environmental values.	⊠Compliant □N/A □Performance Solution
	AO1.3 Development is located, designed and operated to mitigate significant impacts on environmental values. For example, a report certified by an appropriately qualified person demonstrating to the satisfaction of the assessment manager, how the proposed development mitigates impacts, including on water quality, hydrology and biological processes.	AO1.2 The proposal avoids significant impact on the relevant environmental values.	⊠Compliant □N/A □Performance Solution
Management of impacts on matters of e	environmental significance		
PO2 Development is located, designed and constructed to avoid significant impacts on matters of environmental significance.	 AO2 The design and layout of development minimises adverse impacts on ecologically important areas by: a) focusing development in cleared areas to protect existing habitat; b) utilising design to consolidate density and preserve existing habitat and native vegetation; c) aligning new property boundaries to maintain ecologically important areas; d) ensuring that alterations to natural landforms, hydrology and drainage patterns on the development site do 	AO2 The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
PO3	 not negatively affect ecologically important areas; e) ensuring that significant fauna habitats are protected in their environmental context; and f) incorporating measures that allow for the safe movement of fauna through the site. AO3.1 	AO3.1	
An adequate buffer to areas of state environmental significance is provided and maintained.	 A buffer for an area of state environmental significance (Wetland protection area) has a minimum width of: a) 100 metres where the area is located outside Urban areas; or b) 50 metres where the area is located within a Urban areas. 	The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution
	AO3.2 A buffer for an area of state environmental significance is applied and maintained, the width of which is supported by an evaluation of environmental values, including the function and threats to matters of environmental significance.	AO3.2 The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution
PO4 Wetland and wetland buffer areas are maintained, protected and restored.	AO4.1 Native vegetation within wetlands and wetland buffer areas is retained.	AO4.1 The subject site is not located within a wetland or wetland buffer area.	□Compliant ⊠N/A
			□Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	AO4.2 Degraded sections of wetlands and wetland buffer areas are revegetated with endemic native plants in patterns and densities which emulate the relevant regional ecosystem.	AO4.2 The subject site is not located within a wetland or wetland buffer area.	□Compliant ⊠N/A □Performance Solution
PO5 Development avoids the introduction of non- native pest species (plant or animal), that pose a risk to ecological integrity.	AO5.1 Development avoids the introduction of non-native pest species.	AO5.1 The proposal is for a dwelling house and does not introduce non-native species.	□Compliant ⊠N/A □Performance Solution
	AO5.2 The threat of existing pest species is controlled by adopting pest management practices for long-term ecological integrity.	AO5.1 The proposal is for a dwelling house and does not introduce non-native species.	□Compliant ⊠N/A □Performance Solution
Ecological connectivity			
PO6 Development protects and enhances ecological connectivity and/or habitat extent.	AO6.1 Development retains native vegetation in areas large enough to maintain ecological values, functions and processes.	AO6.1 The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution
	AO6.2 Development within an ecological corridor rehabilitates native vegetation.	AO6.2 The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	AO6.3 Development within a conservation corridor mitigates adverse impacts on native fauna, feeding, nesting, breeding and roosting sites and native fauna movements.	AO6.3 The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution
PO7 Development minimises disturbance to matters of state environmental significance (including existing ecological corridors).	AO7.1 Development avoids shading of vegetation by setting back buildings by a distance equivalent to the height of the native vegetation.	AO7.1 The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution
	AO7.2 Development does not encroach within 10 metres of existing riparian vegetation and watercourses.	AO7.2 The proposal is located upon an existing house pad and requires minimal site works to facilitate the dwelling house. The proposal will avoid significant impacts on matters of environmental significance.	⊠Compliant □N/A □Performance Solution
Waterways in an urban area			
PO8 Development is set back from waterways to protect and maintain: a) water quality; b) hydrological functions;	AO8.1 Where a waterway is contained within an easement or a reserve required for that purpose, development does not occur within the easement or reserve;	AO8.1 The subject site is not located upon a waterway.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
 c) ecological processes; d) biodiversity values; e) riparian and in-stream habitat values and connectivity; f) in-stream migration. 	AO8.2 Development does not occur on the part of the site affected by the waterway corridor.	AO8.2 The subject site is not located upon a waterway.	□Compliant ⊠N/A □Performance Solution
Waterways in a non-urban area			
 PO9 Development is set back from waterways to protect and maintain: a) water quality; b) hydrological functions; c) ecological processes; d) biodiversity values; e) riparian and in-stream habitat values and connectivity; f) in-stream migration. 	AO9 Development does not occur on that part of the site affected by a waterway corridor.	A9 The subject site is not located upon a waterway.	□Compliant ⊠N/A □Performance Solution





4.5 Potential Landslide overlay Code

4.5.1 Purpose

PURP	OSE & OVERALL OUTCOMES	PROPO	SAL	COMPLIANCE				
	 The purpose of the Potential landslide hazard overlay code is: a) implement the policy direction of the Strategic Framework, in particular: (i) Theme 1: Settlement pattern Element 3.4.7 Mitigation of hazards. 							
	enable an assessment of whether development is suitable o		· · · · · ·					
2) T	he purpose of the code will be achieved through the following							
a)	development is located, designed and constructed to not put at risk the safety of people, property and the environment;	A)	The proposed dwelling house is located, designed and constructed to reduce the risk of people, property and the environment;	⊠Compliant □N/A				
b)	development is not at risk from and does not pose a risk to adjacent and nearby sites from landslides;	B)	The proposal does not pose a risk to adjacent and nearby sites;	□Performance Solution				
c)	ensures that community infrastructure is protected from the effects of potential landslides;	C) D)	The proposal is for a dwelling house; The development involves minimal site work as the					
d)	ensures that vegetation clearing, stormwater management and filling and/or excavation does not create a landslide hazard and/or rectifies potential pre-existing landslide risks;	E)	dwelling house is to be positioned on the existing house pad. Please refer to the geotechnical report submitted as part					
e)	development does not occur where works to provide a solution for safety of people, property or the environment involves complex engineering solutions to overcome the risk, or would result in a built form or outcome that causes an adverse visual impact on the Hillslopes or Landscape values of Douglas Shire.		of the application.					





4.5.2 Performance Outcomes & Acceptable Outcomes

PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE				
For self-assessable and assessable development							
 PO1 The siting and design of development does not involve complex engineering solutions and does not create or increase the potential landslide hazard risk to the site or adjoining premises through: a) building design; b) increased slope; c) removal of vegetation; 	AO1.1 Development is located on that part of the site not affected by the Potential landslide hazard overlay. AO1.2 Development is on an existing stable, benched site and requires no further earthworks	AO1 The proposed dwelling house is strategically positioned to minimise any potential landslide hazard impacts. Please refer to the geotechnical report submitted as part of the application. AO1.2 The proposed dwelling house is strategically positioned to minimise any potential landslide hazard impacts. Please refer to the geotechnical report submitted as part of the	 Compliant N/A Performance Solution Compliant N/A Performance Solution 				
 d) stability of soil; e) earthworks; f) alteration of existing ground water or surface water paths; 	AO1.3 A competent person certifies that:	application. AO1.3					
g) waste disposal areas.	 a) the stability of the site, including associated buildings and infrastructure, will be maintained during the course of the development and will remain stable for the life of the development; 	The proposed dwelling house is strategically positioned to minimise any potential landslide hazard impacts. Please refer to the geotechnical report submitted as part of the application.	⊠Compliant □N/A □Performance Solution				
	 b) development of the site will not increase the risk of landslide hazard activity on other land, including land above the site; c) the site is not subject to the risk of landslide activity on other land; d) any measures identified in a site-specific geotechnical report for stabilising the 						





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
	 site or development have been fully implemented; e) development does not concentrate existing ground water and surface water paths; f) development does not incorporate on- site waste water disposal. 		
PO2	AO2	AO2	
The siting and design of necessary retaining structures does not cause an adverse visual impact on landscape character or scenic amenity quality of the area.	 Excavation or fill: a) is not more than 1.2 metres in height for each batter or retaining wall; b) is setback a minimum of 2 metres from property boundaries; c) is stepped with a minimum 2 metre wide berm to incorporate landscaping in accordance with Planning scheme policy SC6.7 – Landscaping; d) does not exceed a maximum of 3 batters and 3 berms (i.e. Not greater than 3.6 metres in height) on any one lot. 	The proposal does not require excavation or fill identified within the acceptable outcome.	 ☑ Compliant □ N/A □ Performance Solution
Additional requirements for Communit	y infrastructure		
PO3	AO3	AO3	
 Development for community infrastructure: a) is not at risk from the potential landslide hazard areas; b) will function without impediment from a landslide; 	Development is designed in accordance with the recommendations of a site-specific geotechnical assessment which makes reference to the community infrastructure and its needs and function.	The proposal is for a dwelling house. As such, this criterion is not applicable.	□Compliant ⊠N/A □Performance Solution





PERFORMANCE OUTCOMES	ACCEPTABLE OUTCOMES	PROPOSAL	COMPLIANCE
 c) provides access to the infrastructure without impediment from the effects of a landslide; 			
 does not contribute to an elevated risk of a landslide to adjoining properties. 			



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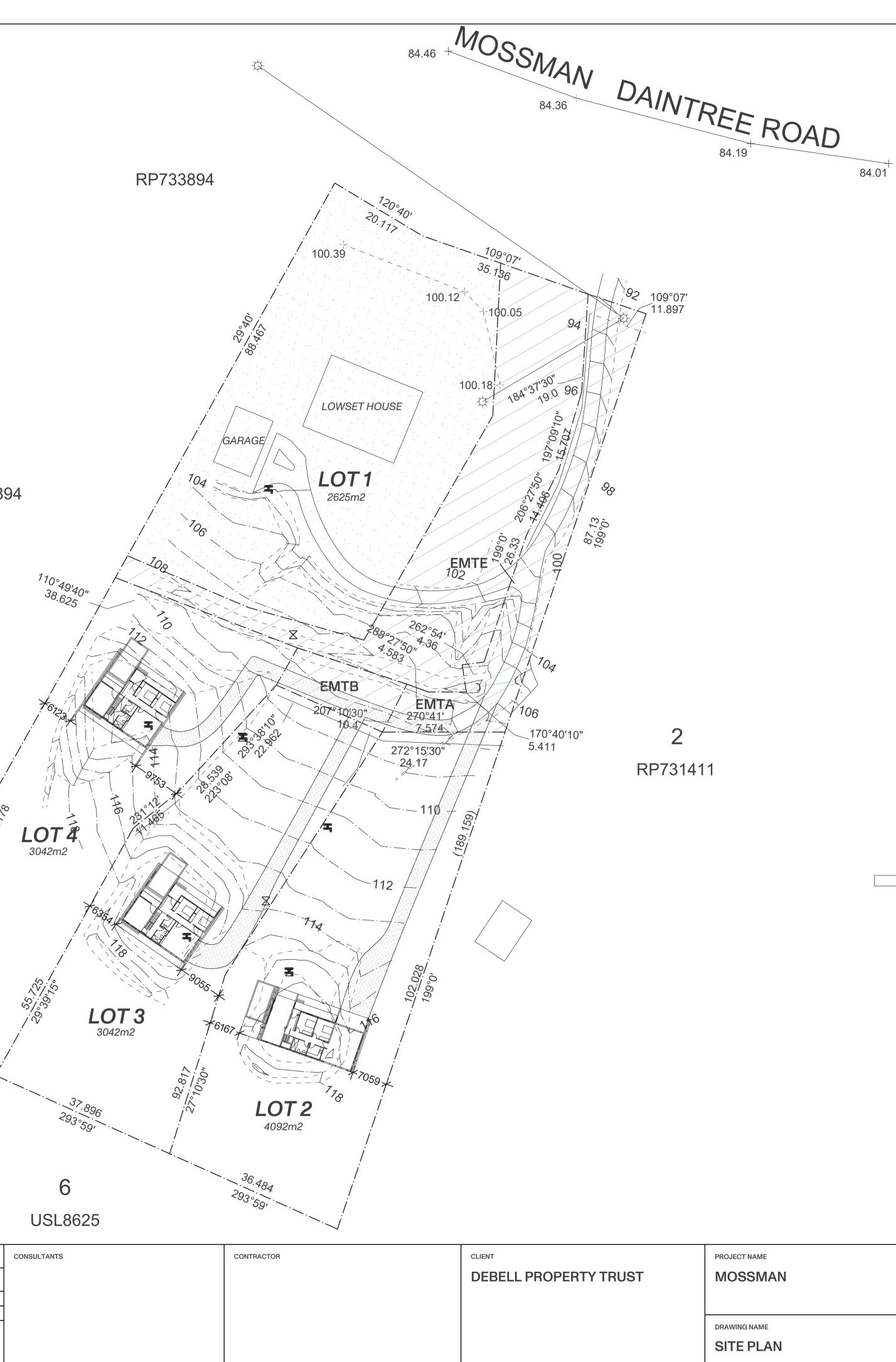
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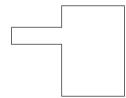
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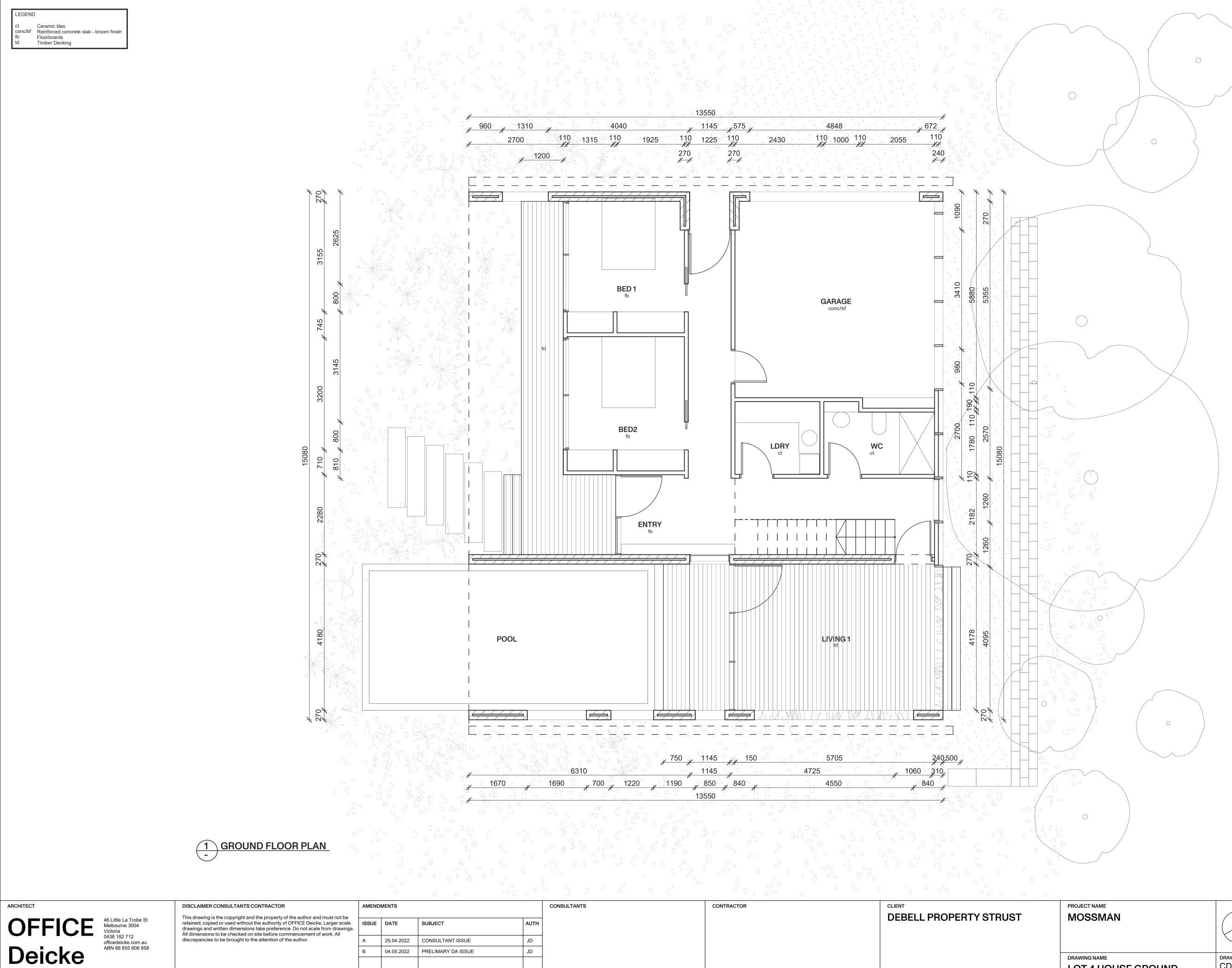
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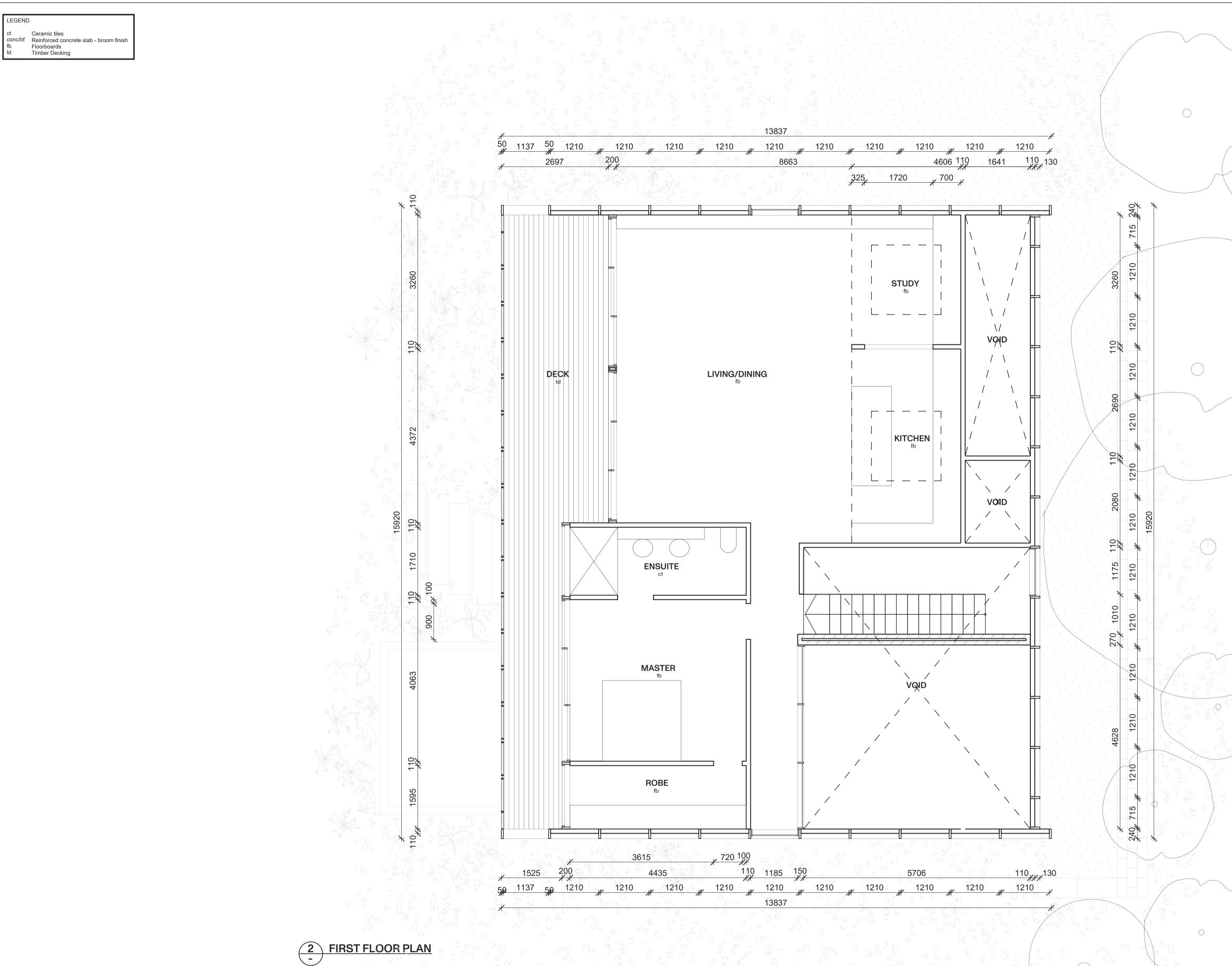
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NOTE: EXACT LOCATION AND ORIENTATION OF DWELLING TO BE CONFIRMED



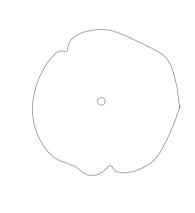
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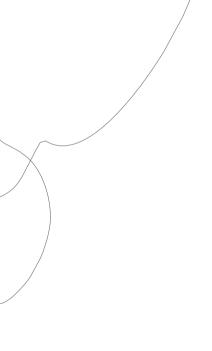
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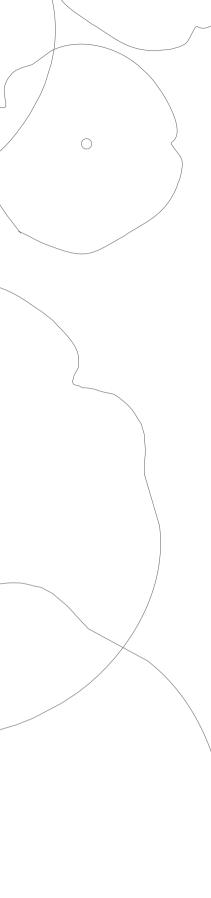
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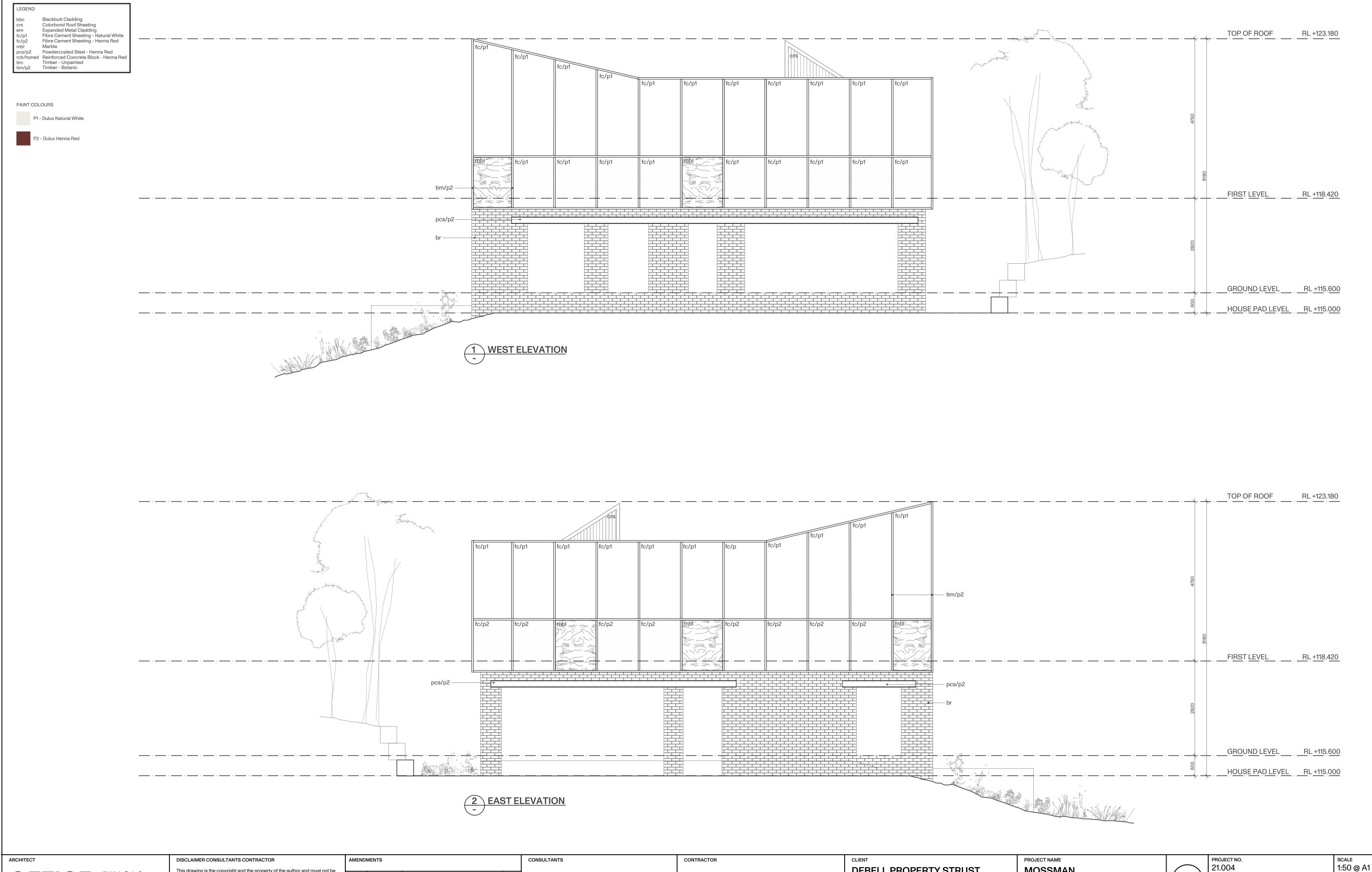


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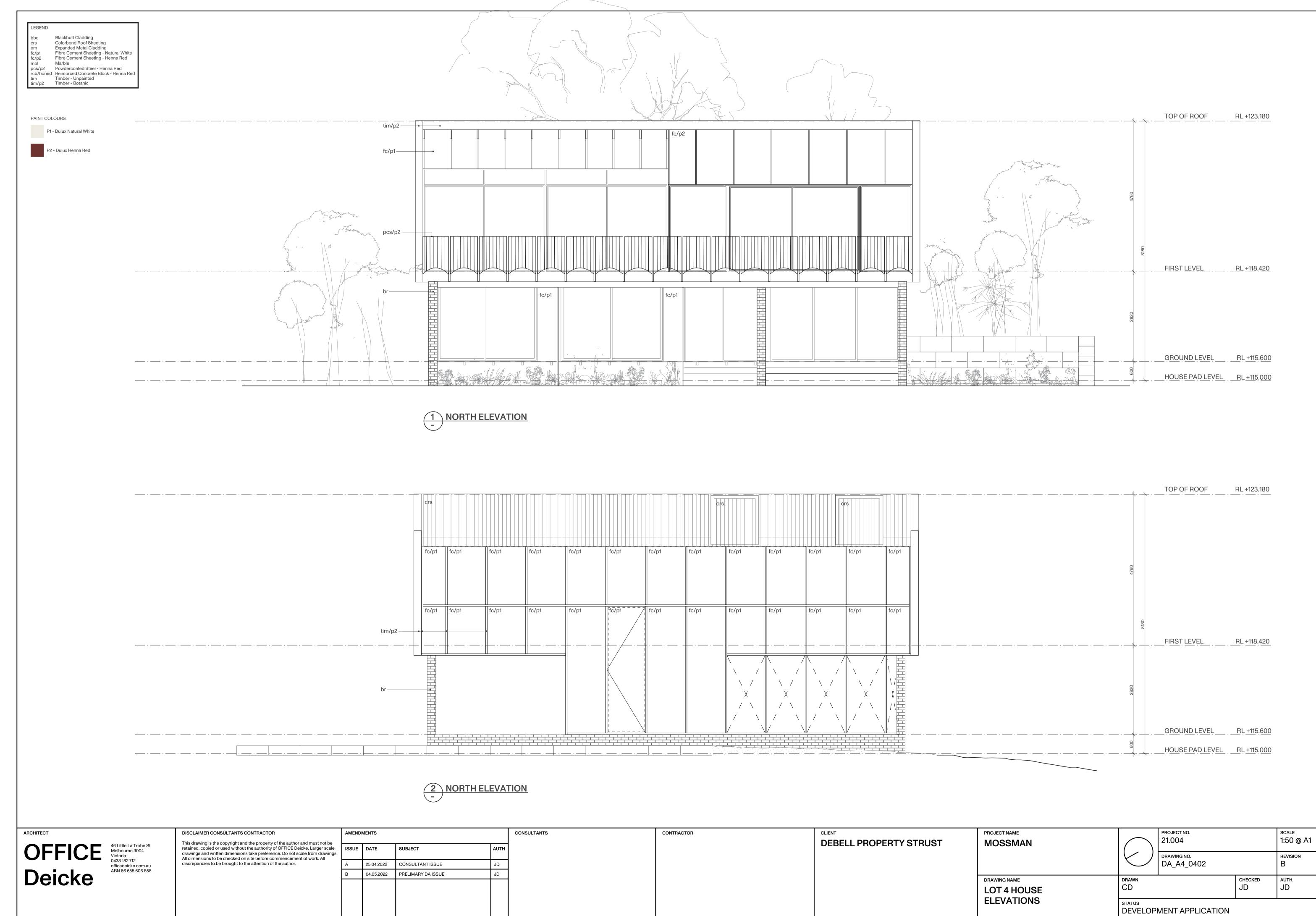
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ISSUE DATE SUBJECT AUTH 25.04.2022 CONSULTANT ISSUE JD 04.05.2022 PRELIMARY DA ISSUE JD

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LOT 2 HOUSE



Roof Sheeting COLORBOND® steel Monument

RGB 19 32 43



Concrete Block

Austral Masonry GB Honed Porcelain



Duker Int

Dulux® Natural White PN1E1 RGB 238 236 229



Dulux Kelburn RGB 168 92 96



LOT 3 HOUSE



COLORBOND® steel Surfmist

RGB 215 214 203

Concrete Block
Austral Masonry GB Honed Porcelain



Paint Colour 1
Dulux® Natural White PN1E1

ulux® Natural White PN1E1 RGB 238 236 229



Paint Colour 2

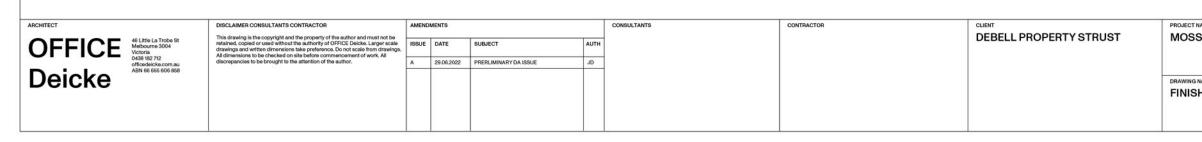
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Timber Blackbutt

LOT 4 HOUSE











Marble Cairns Marble 'Dreamtime'



Blackbutt



Marble Cairns Marble 'Dreamtime'

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29 June 2022

RE: MATERIAL CHANGE OF USE FOR 3 X DWELLING HOUSES UPON LAND AT 1299A, 1299B & 1299C MOSSMAN DAINTREE RD, ROCKY POINT

We, DEBELL PROPERTY PTY LTD A.C.N 655 606 625 TRUSTEE UNDER INSTRUMENT 721498908

the registered owners of 1299a, 1299b & 1299c Mossman Daintree rd, rocky point (Lot 2- 4 on SP238245) hereby grant consent for a development application to be lodged with Council under the Planning Act 2016 by Town Planning Alliance Pty Ltd.

(Name of Sole Director)

Signature of Sole Director

SOLE DIRECTOR TITLE

1

ABN/ACN

1. JOHN DEICKE

(Name of Director/Secretary)

DIRECTOR (TITLE)

FONA (AMPR

(Name of Director/Secretary)

(TITLE: (Director or Secretary)

OR V. Slick

Signature of Director/Secretary 606 625

655 ABN/ACN

Kampbell,

Signature of Director/Secretary 606 625

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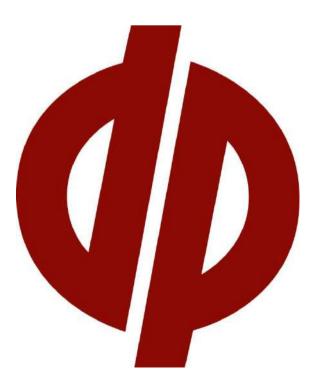


Report on Geotechnical Investigation

Proposed Residences Mossman Daintree Road, Rocky Point

> Prepared for Office Deicke

Project 214803.00 June 2022



Douglas Partners Geotechnics | Environment | Groundwater

Document History

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The undersigned, on behalf of Douglas Partners Pty Ltd, confirm that this document and all attached drawings, logs and test results have been checked and reviewed for errors, omissions and inaccuracies.

	Signature	Date
Author	APPOLL	10 June 2022
Reviewer		10 June 2022



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Report on Geotechnical Investigation Proposed Residences Mossman Daintree Road, Rocky Point

1. Introduction

This report presents the results of geotechnical investigation undertaken by Douglas Partners Pty Ltd (DP) for three proposed residential dwellings at Mossman Daintree Road, Rocky Point. The investigation was commissioned in an email dated 28 April 2022 by Mr John Deicke of Office Deicke and was undertaken in accordance with DP proposal 214803.00.P.001.Rev0 dated 23 April 2022.

It is understood that the project includes the construction of three residential dwellings on separate but adjoining allotments of the subdivided 1299 Mossman Daintree Road.

The aim of the investigation was to assess the subsurface soil and groundwater conditions at the field test locations to provide:

- site classification to AS 2870 (2011);
- excavation conditions and suitable temporary and permanent batter slopes;
- site preparation earthworks and suitability of site won materials for re-use as engineered fill;
- geotechnical retaining wall design parameters;
- footing options and footing design parameters; and
- allocation of soil categories and determination of design soil permeability and design effluent loading rates based on AS 1547 (2012).

Slope risk assessment was excluded from the requested scope of work.

The investigation included the excavation of ten test pits and laboratory testing of selected samples. The details of the field and laboratory work are presented in this report, together with comments and recommendations on the items listed above.

This report must be read in conjunction with the notes entitled 'About This Report' in Appendix A and other explanatory notes, and should be kept in its entirety without separation of individual pages or sections.

2. Proposed Development

DP was supplied with a preliminary site plan (Drawing SD_A01_0101 dated 4 February 2022) for the purposes of investigation planning which is included in Appendix B for ease of reference. This plan indicated the proposed development to comprise the construction of three, two-level residential dwellings on the existing building platforms. It is assumed the buildings will be constructed using lightweight materials, elevated above ground and supported by steel posts.



3. Site Description

The site (designated as 1299A to 1299C Mossman Daintree Road) comprises three adjoining allotments within a battle-axe area above an existing residence (refer Drawing 1 in Appendix B and Figure 1 below). Each lot is approximately rectangular in shape, and comprising an area of 3042 m^2 (Lots 3 and 4) or 4092 m^2 (Lot 2). For the purposes of this report, 'site' refers to the combined area of the three allotments.

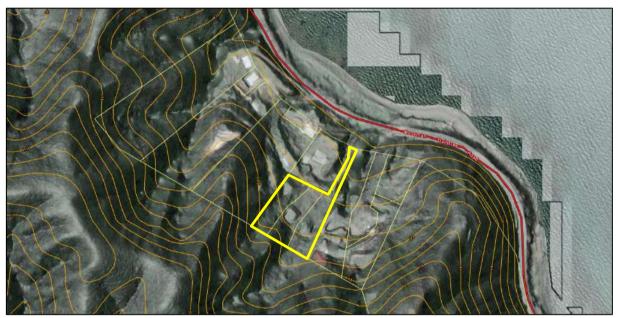


Figure 1: Qld Globe image showing site location in yellow outline.

The site is situated on the north-eastern facing foot slopes of the Dagmar Range and site elevations approximately range from 35 AHD to 55 AHD. Much of the southwestern portion of the site (approximately 25%) comprises steeply sloping hillside vegetated by dense rainforest.

Previous earthworks are evident on site including the preparation of building platforms using 'cut to fill' methodology, and an open unlined drain has been excavated along much of the site's eastern boundary, presumably to divert overland flows down to the road reserve. No structures were observed within site boundaries and other than that for the above earthworks, it appears that no previous site development has occurred.

The main portion of the site, below the south-western steep slopes, has mostly been cleared of native vegetation and is generally vegetated by short grass with several large trees and shrubs present mostly around the building platforms (refer Figures 2 to 5). The terrain in the cleared portions is generally uneven and hummocky, and the building platforms appear to have been created by localised cutting and filling. The natural slope gradients downhill of the of the building platforms generally range between 10° to 15°(ie 'moderately' sloping).

The site is bordered by residential properties downhill to the north, east and west, and steeply sloping, densely vegetated slopes uphill to the south.





Figure 2: Showing general site conditions. Photo is from the end of the shared driveway with a view towards the southwest.



Figure 3: View looking northeast showing the building platform at 1299A.



Figure 4: View looking northeast showing the building platform at 1299B.



Figure 5: View looking west showing the building platform at 1299C.



4. Geology

Reference to the Mossman 1:100,000 Geological Series Sheet (Department of Mines and Energy, 1999) indicates that the site is underlain by Hodgkinson Formation bedrock typically comprising siltstone and fine to coarse grained arenite, and residual soils thereof.

Investigation findings are generally consistent with the published findings however colluvial materials were commonly encountered, generally overlying residual soils in the majority of the test locations.

5. Field Work

5.1 Methods

The field work was completed on 4 May 2022 and comprised the excavation of ten test pits, designated as Pits 1 to 10, to depths ranging from 1.7 m to 3.9 m. Approximate test locations are shown on Drawing 1 in Appendix B.

The pits were excavated using a 5.5 tonne excavator. Strata identification was through observation of excavated spoil, and soil samples were taken at regular depth intervals for laboratory testing. Pocket penetrometer readings were taken in the walls of the pits and dynamic cone penetrometer (DCP) tests were undertaken alongside the pits to provide additional information for assessment of soil strength consistency. On completion of excavation, the pits were backfilled with excavated spoil in layers which were nominally compacted using the back of the excavator bucket. Any excess spoil was mounded and track rolled at the surface.

The field work was carried out in the presence of a DP engineering geologist.

A differential GPS unit was used to record UTM co-ordinates to GDA94. Surface levels were extrapolated from a supplied contour drawing. The co-ordinates and surface levels are presented on the test pit logs in Appendix D.

5.2 Results

The subsurface conditions encountered in the pits are detailed on the logs in Appendix D. Notes defining the sampling methods, soil descriptions, and symbols and abbreviations used in their preparation are given in Appendix A.

Subsurface conditions may be grouped into two areas for discussion, the building platform (ie modified) areas and downslope (ie natural) areas.

Fill, comprising medium dense clayey sandy gravel (similar to the naturally occurring colluvium encountered elsewhere on site) was encountered in Pits 2, 4 and 6 to depths of between 1.2 m and 2.2 m.

Natural materials encountered within the majority of the building platform test locations, either below the fill or from the surface (refer Pits 1, 2 and 4 to 6) are broadly characterised by colluvial clayey or silty



sandy gravel with cobbles (generally assessed to be medium dense or denser), underlain in Pits 5 and 6 by stiff or stronger residual clayey silt, transitioning to weathered siltstone within which virtual excavator refusal occurred on probable very low strength or stronger rock. Colluvium was not encountered in Pit 3, which encountered hard, residual clayey silt to 1.5 m depth, overlying siltstone to virtual excavator refusal at 1.7 m depth.

Within the downslope hillside test locations (refer Pits 7 to 10), ground conditions generally comprised a shallow (0.15 m or 0.2 m thick) layer of firm or stronger clayey silt topsoil overlying stiff or stronger clayey silt. The exception to this generalised profile description was the presence of very stiff silty clay encountered below 1.5 m depth in Pit 7, to the limit of investigation.

Free groundwater was not observed in any of the test pits, however it should be noted that groundwater depths and ground moisture conditions are affected by climatic conditions, soil permeability and human influences, and will therefore vary with time. Rocky Point is in the wet tropics and subject to wet and dry seasons, during which the groundwater levels may be prone to vary considerably.

Deeply eroded ephemeral water courses / surface drains were observed to the east and north of site indicating the likelihood of significant overland flows during and following significant rainfall events.

6. Laboratory Testing

Geotechnical Laboratory Testing 6.1

Geotechnical laboratory testing comprised:

- Atterberg limits, linear shrinkage, and field moisture content (three tests); and
- Particle size distribution, greater than 0.075 mm (two tests).

Geotechnical laboratory results are summarised in Tables 1 and 2, with laboratory certificates presented in Appendix E.

Pit	Depth (m)	Primary Description	FMC (%)	LL (%)	PL (%)	PI (%)	LS (%)
4	0.5	Clayey Sandy GRAVEL	18.1	28	23	5	3.5
5	1.0	Clayey SILT	18.0	34	23	11	4.5
8	0.3	Clayey SILT	14.7	24	19	5	2.5
Notes to				_ '	.0	•	2.0

Table 1: Results of Laboratory Testing – Field Moisture, Atterberg Limits and Linear Shrinkage

FMC - Field Moisture Content LL - Liquid Limit

PL - Plastic Limit

PI - Plasticity Index LS - Linear Shrinkage



Pit	Depth (m)	Primary Description	Gravel (%)	Sand (%)	Silt and clay (%)
1	0.4	Clayey Sandy GRAVEL	38	30	32
4	0.5	Clayey GRAVEL	52	19	29

Table 2: Results of Laboratory Testing - Gradings

6.2 Effluent Disposal Laboratory Testing

Effluent disposal laboratory testing comprised:

- Emerson class number, soil pH, electrical conductivity (EC), cation exchange capacity (three tests); and
- total nitrogen, sodium adsorption ratio, phosphorous absorption capacity (three tests).

Effluent disposal laboratory results are summarised in Tables 3 and 4, with laboratory certificates presented in Appendix E.

Pit	Depth (m)	Primary Description	Soil pH 1:5	EC (µS/cm)	Phosphorous Sorption Capacity (mgP/kg)	Total Nitrogen in Soil (mg/kg)	Sodium Adsorption Ratio	Emerson Class Number
7	1.7	Silty CLAY	5.9	48	670	170	1.2	5
9	1.5	Clayey SILT	5.2	51	560	180	1.8	6
10	1.0	Clayey SILT	5.4	45	530	270	0.56	5

Table 3: Results of pH, EC, Phosphorous Ssorption and Emerson Class Number Testing

Table 4: Results of Cation Exchange Capacity Testing

Pit	Depth (m)	Description	Calcium (meq%)	Potassium (meq%)	Magnesium (meq%)	Sodium (meq%)	CEC (meq%)	ESP (%)
7	1.7	Silty CLAY	0.2	<0.1	0.1	<0.1	<1	<1
9	1.5	Clayey SILT	0.1	<0.1	<0.1	<0.1	<1	<1
10	1.0	Clayey SILT	0.5	<0.1	0.2	<0.1	<1	<1

Legend:

CEC – cation exchange capacity

ESP - exchangeable sodium percentage



7. Comments

7.1 Appreciation of Site Geotechnical Conditions

The natural ground conditions at the field test locations are generally characterised as comprising medium dense or denser colluvial soils consisting of varying proportions of clayey silt, sand, gravel and cobbles, generally overlying residual stiff or stronger clayey silt transitioning with depth to weathered siltstone bedrock. Fill of similar properties to the colluvial soils was encountered to depths of between 1.2 m and 2.2 m within the north-western portions of each of the building platforms. It is assumed that the fill has been site-won from the uphill (south-western) sides of the building platform areas.

In isolation of upslope instability risk, the assessment of which was not requested by the client but is assumed to have been completed by others, the primary geohazard for this project is anticipated to be that of the existing uncontrolled fill, which should not be relied upon for structural support. The design of on-site effluent disposal systems should only be undertaken by appropriately experienced personnel familiar with the site and climatic conditions, giving particular consideration to hydraulic balance during the wet season.

Further comments on design and construction are given in the following sections of the report.

7.2 Site Classification

AS 2870 (2011) states that site classification is based on expected ground surface movement. Surface movement is usually due to soil reactivity under normal moisture conditions, however in some cases the level of ground movement may be controlled by other factors. Sites where ground movements may be significantly affected by factors other than reactive soil movements under normal moisture conditions are classified as 'Class P' under the standard and footing design must by based on engineering principles.

AS2870 (2011) provides a list of scenarios that would require a 'Class P' classification, including the presence of 'uncontrolled' fill, which applies to the existing building platforms in their current conditions.

7.3 Excavation Conditions

Excavations for high level footings and trenches for effluent disposal are anticipated to encounter either colluvial clayey/silty sandy gravel or residual clayey silt transitioning to very low strength weathered siltstone. It is considered that the colluvial and residual soils should be readily excavatable by standard small sized (ie 12 - 20 tonne) earthmoving equipment. Ripping would likely be required for excavations within low strength or stronger siltstone, depending on fracturing and strength variability.



7.4 Batter Slopes

Short-term temporary batter slopes of 1.5H:1V are suggested for unsurcharged, dry, temporary excavations in natural in-situ soils or existing fill up to 3 m deep. Permanent batter slope angles of no steeper than 3H:1V are suggested, with steeper slopes requiring additional geotechnical assessment, possibly requiring the construction of engineer designed retaining walls (refer Section 7.6).

Stockpiles or heavy plant should not be placed near the batter crests, as this may instigate slope failure. Further advice should be sought from DP if such surcharge is located within the batter vertical height laterally behind the crest.

Surface runoff should be diverted away from the crests and toes of the batters to reduce the potential for scour erosion. It is also recommended that all batters incorporate crest and toe drains and be appropriately vegetated with respect to erosion control.

7.5 Site Preparation

It is suggested that site preparation and fill for the support of ground slabs, pavements and upper level footings be carried out in accordance with the following guidelines::

- Remove all existing uncontrolled fill, surface vegetation, organic topsoil, and any deleterious soft, wet or highly compressible material;.
- Tine the subgrade and adjust the moisture content to within 2% of optimum moisture content for standard compaction (SOMC);
- Compact the subgrade with at least six passes of a minimum 12 t static weight smooth drum roller. Test roll the compacted subgrade under careful inspection by a geotechnical engineer to detect any remaining relatively soft or loose zones, which should be excavated out and replaced with approved engineered fill under 'Level 1' inspection and testing;
- If additional fill is required, place approved engineered fill in layers not exceeding 200 mm loose thickness, and compact to at least 98% standard dry density ratio or density index of at least 75%. Moisture contents within cohesive fill should be maintained within 2% of SOMC, during and after compaction; and
- Undertake 'Level 1' inspection and testing for all additional fill placement works, in accordance with AS 3798 (2007).

It is important to note that dry, over-compacted cohesive fill or cohesive subgrade (ie compacted to move than 102% SMDD and dry of OMC) is generally more prone to swelling and softening. For this reason, fill compaction and moisture should be carefully controlled on site, with both compaction and moisture control criteria included in the bulk earthworks specification.

Fill placement and compaction under 'Level 1' inspection and testing in accordance with AS 3798 (2007), is required where structural loads are to be supported by fill. A 'Level 1' inspection and test report must also be prepared at the completion of the works stating that the fill has been completed as recommended above and as required by AS 3798 (2007).



7.6 Retaining Walls

Table 5 presents lateral earth pressure, bulk density and sliding coefficient for the various soils encountered.

Material	Strength Consistency /Relative Density	Ka	Ко	Кр	Bulk Unit Weight (kN/m³)	tanδ⁵	
Clayey Silt / Silty Clay	Stiff	0.4	0.55	2.5	18	0.3	
	Very stiff or hard	0.4	0.55	2.5	20		
Granular	Medium dense (or denser)	0.28	0.45	3.5	20	0.4	

Notes: * for concrete cast directly onto the clean soil surface

For design of retaining walls, due allowance should be made for the following:

- Surcharge loadings (over and above the lateral earth pressure coefficients presented above) where the finished ground level above retaining walls is above horizontal and where additional loading is likely to be applied from existing or future upslope structures, or from traffic.
- Drainage material installed for the full height behind the wall, to a width of at least 0.3 m. The material must be free draining and granular and have a perforated or slotted drainage pipe at the heel of the wall to rapidly remove the water into the stormwater system. If drainage is not provided, the wall loading caused by flooding or inundation, must be considered. Such flooding may penetrate up to 0.9 m depth into cracks behind the wall and result in a hydrostatic load.

Care should be taken when placing soils behind retaining walls, to reduce the risk of damage associated with the use of heavy compaction plant and swelling. Compaction should not exceed 95% to 98% standard maximum dry density ratio and all filling should be placed within 2% of OMC.

It is recommended that factors of safety of 2 against overturning and sliding stability and 1.5 for global stability, be adopted in the design of all retaining walls.

For limit state design methods, the ultimate parameters provided above in Table 5 will need to be factored in accordance with (AS 4678, 2002). Guidance on the selection of material strength partial factors is provided in Section 5.2 of (AS 4678, 2002) and is dependent upon the nature and state of the natural in-situ soil.

7.7 Footings

High level pad and strip footings up to 1 m and 0.6 m wide, respectively, founding in 'controlled' fill, medium dense or denser colluvium or stiff or stronger clayey silt may be sized for a maximum allowable bearing pressure of 100 kPa.

Settlements of approximately 10 mm to 15 mm are estimated for properly designed and constructed pad or strip footings dimensioned and supported as above. Wider footings are possible but would be subject to specific settlement assessment.

7.8 Soil and Site Assessment

The suitability of the site to accept effluent is dependent on a number of factors, including:

- the subsurface profile (soil and groundwater) within the proposed application area;
- the type and output nutrient quality of the treatment system utilised;
- the physical characteristics such as topography, landform, distances to water bodies, etc; and
- prevailing climatic conditions.

The controlling soil for on-site effluent disposal by traditional systems is likely to be the residual clayey silt due to its low permeability and poor soil structure. Based on Table 5.1 of (AS 1547, 2012), for a weakly structured Category 4 soil an indicative permeability of between 0.12 and 0.5 m/day is suggested.

Site and soil characteristics observed during the field work are assigned either a minor, moderate or major limitation depending on the restrictions to the disposal area in accordance with (AS 1547, 2012). The moderate/major limitations for effluent disposal within the site are shown below. Soil and site assessment summaries are provided in Appendix C.

Cation Exchange Capacity (CEC)

The CEC results of <1 meq% fall into the major limitation category due to the inability of these soils to hold plant nutrients.

Soil pH

Soil pH test results of 5.2 and 5.4 fall into the moderate limitation category due to non-optimal plant growth conditions.

Slope Angle

Slope angle of between 10° and 20° (approximately 20% to 40% grade) is a major limiting factor for all disposal systems due to the risk of runoff.

Landform and Site Drainage

The site topography presents a moderate risk of groundwater pollution and transport of waste offsite.

Upslope seepage

The run on and upslope seepage potential is a moderate risk due to transport of wastewater off site.



7.9 Design Effluent Loading Rates

For Category 4 soil it is recommended that the effluent design loading rates and design irrigation rates (DLR/DIR) as presented below in Table 6 be adopted, based on (AS 1547, 2012).

Table 6: Effluent Design Loading / Irrigation Rates

Dispessel System	Effluent Design Loading / Irrigation Rates			
Disposal System	Primary Treated Effluent	Secondary Treated Effluent		
Conventional Trenches or Beds	6 to 10 mm/day	20 mm/day		
Evapotranspiration/absorption (ETA/ETS) system	8 mm/day			
Irrigation System – Drip or spray	3.5 mm/day ^A			
Irrigation System – Low pressure effluent distribution (LPED)	3 mm/day			
Mound System	8 mm/day			

Notes: A requires a minimum of 150 mm of high-quality topsoil

8. Limitations

Douglas Partners Pty Ltd (DP) has prepared this report for this project at 1299 Mossman Daintree Road in accordance with DP's proposal dated 23 April 2022 and acceptance received from John Deicke dated 28 April 2022. The work was carried out under DP's Conditions of Engagement. This report is provided for the exclusive use of Office Deicke for this project only and for the purposes as described in the report. It should not be used by or relied upon for other projects or purposes on the same or other site or by a third party. Any party so relying upon this report beyond its exclusive use and purpose as stated above, and without the express written consent of DP, does so entirely at its own risk and without recourse to DP for any loss or damage. In preparing this report DP has necessarily relied upon information provided by the client and/or their agents.

The results provided in the report are indicative of the sub-surface conditions on the site only at the specific sampling and/or testing locations, and then only to the depths investigated and at the time the work was carried out. Sub-surface conditions can change abruptly due to variable geological processes and also as a result of human influences. Such changes may occur after DP's field testing has been completed.

DP's advice is based upon the conditions encountered during this investigation. The accuracy of the advice provided by DP in this report may be affected by undetected variations in ground conditions across the site between and beyond the sampling and/or testing locations.



The assessment of atypical safety hazards arising from this advice is restricted to the geotechnical components set out in this report and based on known project conditions and stated design advice and assumptions. While some recommendations for safe controls may be provided, detailed 'safety in design' assessment is outside the current scope of this report and requires additional project data and assessment.

This report must be read in conjunction with all of the attached and should be kept in its entirety without separation of individual pages or sections. DP cannot be held responsible for interpretations or conclusions made by others unless they are supported by an expressed statement, interpretation, outcome or conclusion stated in this report.

This report, or sections from this report, should not be used as part of a specification for a project, without review and agreement by DP. This is because this report has been written as advice and opinion rather than instructions for construction.

The scope of work for this investigation/report did not include the assessment of surface or sub-surface materials or groundwater for contaminants, within or adjacent to the site. Should evidence of fill of unknown origin be noted in the report, and in particular the presence of building demolition materials, it should be recognised that there may be some risk that such fill may contain contaminants and hazardous building materials.

9. References

AS 1547. (2012). On-site domestic wastewater management. Standards Australia.

- AS 2870. (2011). Residential Slabs and Footings. Standards Australia.
- AS 3798. (2007). *Guidelines on Earthworks for Commercial and Residential Developments.* Standards Australia.

AS 4678. (2002). *Earth-retaining structures*. Standards Australia.

Department of Mines and Energy. (1999). Mossman. Brisbane, QLD: Queensland Government.

Douglas Partners Pty Ltd

Appendix A

About This Report



Introduction

These notes have been provided to amplify DP's report in regard to classification methods, field procedures and the comments section. Not all are necessarily relevant to all reports.

DP's reports are based on information gained from limited subsurface excavations and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

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This report is the property of Douglas Partners Pty Ltd. The report may only be used for the purpose for which it was commissioned and in accordance with the Conditions of Engagement for the commission supplied at the time of proposal. Unauthorised use of this report in any form whatsoever is prohibited.

Borehole and Test Pit Logs

The borehole and test pit logs presented in this report are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling or excavation. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable or possible to justify on economic grounds. In any case the boreholes and test pits represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes or pits, the frequency of sampling, and the possibility of other than 'straight line' variations between the test locations.

Groundwater

Where groundwater levels are measured in boreholes there are several potential problems, namely:

 In low permeability soils groundwater may enter the hole very slowly or perhaps not at all during the time the hole is left open;

- A localised, perched water table may lead to an erroneous indication of the true water table;
- Water table levels will vary from time to time with seasons or recent weather changes. They may not be the same at the time of construction as are indicated in the report; and
- The use of water or mud as a drilling fluid will mask any groundwater inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water measurements are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

Reports

The report has been prepared by qualified personnel, is based on the information obtained from field and laboratory testing, and has been undertaken to current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal, the information and interpretation may not be relevant if the design proposal is changed. If this happens, DP will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface conditions, discussion of geotechnical and environmental aspects, and recommendations or suggestions for design and construction. However, DP cannot always anticipate or assume responsibility for:

- Unexpected variations in ground conditions. The potential for this will depend partly on borehole or pit spacing and sampling frequency;
- Changes in policy or interpretations of policy by statutory authorities; or
- The actions of contractors responding to commercial pressures.

If these occur, DP will be pleased to assist with investigations or advice to resolve the matter.

About this Report

Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, DP requests that it be immediately notified. Most problems are much more readily resolved when conditions are exposed rather than at some later stage, well after the event.

Information for Contractual Purposes

Where information obtained from this report is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. DP would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

Site Inspection

The company will always be pleased to provide engineering inspection services for geotechnical and environmental aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

Sampling

Sampling is carried out during drilling or test pitting to allow engineering examination (and laboratory testing where required) of the soil or rock.

Disturbed samples taken during drilling provide information on colour, type, inclusions and, depending upon the degree of disturbance, some information on strength and structure.

Undisturbed samples are taken by pushing a thinwalled sample tube into the soil and withdrawing it to obtain a sample of the soil in a relatively undisturbed state. Such samples yield information on structure and strength, and are necessary for laboratory determination of shear strength and compressibility. Undisturbed sampling is generally effective only in cohesive soils.

Test Pits

Test pits are usually excavated with a backhoe or an excavator, allowing close examination of the insitu soil if it is safe to enter into the pit. The depth of excavation is limited to about 3 m for a backhoe and up to 6 m for a large excavator. A potential disadvantage of this investigation method is the larger area of disturbance to the site.

Large Diameter Augers

Boreholes can be drilled using a rotating plate or short spiral auger, generally 300 mm or larger in diameter commonly mounted on a standard piling rig. The cuttings are returned to the surface at intervals (generally not more than 0.5 m) and are disturbed but usually unchanged in moisture content. Identification of soil strata is generally much more reliable than with continuous spiral flight augers, and is usually supplemented by occasional undisturbed tube samples.

Continuous Spiral Flight Augers

The borehole is advanced using 90-115 mm diameter continuous spiral flight augers which are withdrawn at intervals to allow sampling or in-situ testing. This is a relatively economical means of drilling in clays and sands above the water table. Samples are returned to the surface, or may be collected after withdrawal of the auger flights, but they are disturbed and may be mixed with soils from the sides of the hole. Information from the drilling (as distinct from specific sampling by SPTs or undisturbed samples) is of relatively low reliability, due to the remoulding, possible mixing or softening of samples by groundwater.

Non-core Rotary Drilling

The borehole is advanced using a rotary bit, with water or drilling mud being pumped down the drill rods and returned up the annulus, carrying the drill cuttings. Only major changes in stratification can be determined from the cuttings, together with some information from the rate of penetration. Where drilling mud is used this can mask the cuttings and reliable identification is only possible from separate sampling such as SPTs.

Continuous Core Drilling

A continuous core sample can be obtained using a diamond tipped core barrel, usually with a 50 mm internal diameter. Provided full core recovery is achieved (which is not always possible in weak rocks and granular soils), this technique provides a very reliable method of investigation.

Standard Penetration Tests

Standard penetration tests (SPT) are used as a means of estimating the density or strength of soils and also of obtaining a relatively undisturbed sample. The test procedure is described in Australian Standard 1289, Methods of Testing Soils for Engineering Purposes - Test 6.3.1.

The test is carried out in a borehole by driving a 50 mm diameter split sample tube under the impact of a 63 kg hammer with a free fall of 760 mm. It is normal for the tube to be driven in three successive 150 mm increments and the 'N' value is taken as the number of blows for the last 300 mm. In dense sands, very hard clays or weak rock, the full 450 mm penetration may not be practicable and the test is discontinued.

The test results are reported in the following form.

 In the case where full penetration is obtained with successive blow counts for each 150 mm of, say, 4, 6 and 7 as:

4,6,7 N=13

In the case where the test is discontinued before the full penetration depth, say after 15 blows for the first 150 mm and 30 blows for the next 40 mm as:

15, 30/40 mm

Sampling Methods

The results of the SPT tests can be related empirically to the engineering properties of the soils.

Dynamic Cone Penetrometer Tests / Perth Sand Penetrometer Tests

Dynamic penetrometer tests (DCP or PSP) are carried out by driving a steel rod into the ground using a standard weight of hammer falling a specified distance. As the rod penetrates the soil the number of blows required to penetrate each successive 150 mm depth are recorded. Normally there is a depth limitation of 1.2 m, but this may be extended in certain conditions by the use of extension rods. Two types of penetrometer are commonly used.

- Perth sand penetrometer a 16 mm diameter flat ended rod is driven using a 9 kg hammer dropping 600 mm (AS 1289, Test 6.3.3). This test was developed for testing the density of sands and is mainly used in granular soils and filling.
- Cone penetrometer a 16 mm diameter rod with a 20 mm diameter cone end is driven using a 9 kg hammer dropping 510 mm (AS 1289, Test 6.3.2). This test was developed initially for pavement subgrade investigations, and correlations of the test results with California Bearing Ratio have been published by various road authorities.

Soil Descriptions

Description and Classification Methods

The methods of description and classification of soils and rocks used in this report are generally based on Australian Standard AS1726:2017, Geotechnical Site Investigations. In general, the descriptions include strength or density, colour, structure, soil or rock type and inclusions.

5

Soil Types

Soil types are described according to the predominant particle size, qualified by the grading of other particles present:

Туре	Particle size (mm)
Boulder	>200
Cobble	63 - 200
Gravel	2.36 - 63
Sand	0.075 - 2.36
Silt	0.002 - 0.075
Clay	<0.002

The sand and gravel sizes can be further subdivided as follows:

Туре	Particle size (mm)
Coarse gravel	19 - 63
Medium gravel	6.7 - 19
Fine gravel	2.36 - 6.7
Coarse sand	0.6 - 2.36
Medium sand	0.21 - 0.6
Fine sand	0.075 - 0.21

Definitions of grading terms used are:

- Well graded a good representation of all particle sizes
- Poorly graded an excess or deficiency of particular sizes within the specified range
- Uniformly graded an excess of a particular particle size
- Gap graded a deficiency of a particular particle size with the range

The proportions of secondary constituents of soils are described as follows:

In fine grained soils (>35% fines))
-------------------------	-------------	---

Term	Proportion	Example			
	of sand or				
	gravel				
And	Specify	Clay (60%) and			
		Sand (40%)			
Adjective	>30%	Sandy Clay			
With	15 – 30%	Clay with sand			
Trace	0 - 15%	Clay with trace			
		sand			

In coarse grained soils (>65% coarse)

 with clays or silts 	6	
Term	Proportion of fines	Example
And	Specify	Sand (70%) and Clay (30%)
Adjective	>12%	Clayey Sand
With	5 - 12%	Sand with clay

0 - 5%

Sand with trace clay

In coarse grained soils (>65% coarse) - with coarser fraction

Trace

Term	Proportion	Example	
	of coarser		
	fraction		
And	Specify	Sand (60%) and	
		Gravel (40%)	
Adjective	>30%	Gravelly Sand	
With	15 - 30%	Sand with gravel	
Trace	0 - 15%	Sand with trace	
		gravel	

The presence of cobbles and boulders shall be specifically noted by beginning the description with 'Mix of Soil and Cobbles/Boulders' with the word order indicating the dominant first and the proportion of cobbles and boulders described together.

Soil Descriptions

Cohesive Soils

Cohesive soils, such as clays, are classified on the basis of undrained shear strength. The strength may be measured by laboratory testing, or estimated by field tests or engineering examination. The strength terms are defined as follows:

Description	Abbreviation	Undrained shear strength (kPa)
Very soft	VS	<12
Soft	S	12 - 25
Firm	F	25 - 50
Stiff	St	50 - 100
Very stiff	VSt	100 - 200
Hard	Н	>200
Friable	Fr	-

Cohesionless Soils

Cohesionless soils, such as clean sands, are classified on the basis of relative density, generally from the results of standard penetration tests (SPT), cone penetration tests (CPT) or dynamic penetrometers (PSP). The relative density terms are given below:

Relative Density	Abbreviation	Density Index (%)
Very loose	VL	<15
Loose	L	15-35
Medium dense	ense MD	35-65
Dense	D	65-85
Very dense	VD	>85

Soil Origin

It is often difficult to accurately determine the origin of a soil. Soils can generally be classified as:

- Residual soil derived from in-situ weathering of the underlying rock;
- Extremely weathered material formed from in-situ weathering of geological formations. Has soil strength but retains the structure or fabric of the parent rock;
- Alluvial soil deposited by streams and rivers;

- Estuarine soil deposited in coastal estuaries;
- Marine soil deposited in a marine environment;
- Lacustrine soil deposited in freshwater lakes;
- Aeolian soil carried and deposited by wind;
- Colluvial soil soil and rock debris transported down slopes by gravity;
- Topsoil mantle of surface soil, often with high levels of organic material.
- Fill any material which has been moved by man.

Moisture Condition – Coarse Grained Soils For coarse grained soils the moisture condition

should be described by appearance and feel using the following terms:

- Dry (D) Non-cohesive and free-running.
- Moist (M) Soil feels cool, darkened in colour.

Soil tends to stick together. Sand forms weak ball but breaks easily.

Wet (W) Soil feels cool, darkened in colour.

Soil tends to stick together, free water forms when handling.

Moisture Condition – Fine Grained Soils

For fine grained soils the assessment of moisture content is relative to their plastic limit or liquid limit, as follows:

- 'Moist, dry of plastic limit' or 'w <PL' (i.e. hard and friable or powdery).
- 'Moist, near plastic limit' or 'w ≈ PL (i.e. soil can be moulded at moisture content approximately equal to the plastic limit).
- 'Moist, wet of plastic limit' or 'w >PL' (i.e. soils usually weakened and free water forms on the hands when handling).
- 'Wet' or 'w ≈LL' (i.e. near the liquid limit).
- 'Wet' or 'w >LL' (i.e. wet of the liquid limit).

Symbols & Abbreviations

Introduction

These notes summarise abbreviations commonly used on borehole logs and test pit reports.

Drilling or Excavation Methods

С	Core drilling
R	Rotary drilling
SFA	Spiral flight augers
NMLC	Diamond core - 52 mm dia
NQ	Diamond core - 47 mm dia
HQ	Diamond core - 63 mm dia
PQ	Diamond core - 81 mm dia

Water

\triangleright	Water seep
\bigtriangledown	Water level

Sampling and Testing

- A Auger sample
- B Bulk sample
- D Disturbed sample
- E Environmental sample
- U₅₀ Undisturbed tube sample (50mm)
- W Water sample
- pp Pocket penetrometer (kPa)
- PID Photo ionisation detector
- PL Point load strength Is(50) MPa
- S Standard Penetration Test
- V Shear vane (kPa)

Description of Defects in Rock

The abbreviated descriptions of the defects should be in the following order: Depth, Type, Orientation, Coating, Shape, Roughness and Other. Drilling and handling breaks are not usually included on the logs.

Defect Type

Bedding plane
Clay seam
Cleavage
Crushed zone
Decomposed seam
Fault
Joint
Lamination
Parting
Sheared Zone
Vein

Orientation

The inclination of defects is always measured from the perpendicular to the core axis.

- h horizontal
- v vertical
- sh sub-horizontal

ar

sv sub-vertical

Coating or Infilling Term

cln	clean
со	coating
he	healed
inf	infilled
stn	stained
ti	tight
vn	veneer

Coating Descriptor

ca	calcite
cbs	carbonaceous
cly	clay
fe	iron oxide
mn	manganese
slt	silty

Shape

cu	curved
ir	irregular
pl	planar
st	stepped
un	undulating

Roughness

ро	polished
ro	rough
sl	slickensided
sm	smooth
vr	very rough

Other

fg	fragmented
bnd	band
qtz	quartz

Symbols & Abbreviations

Graphic Symbols for Soil and Rock

General

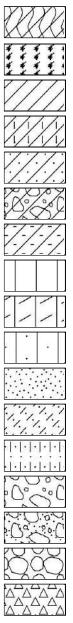
D. D. D. Z	
1. A. A. A. XXXX	
\times	

Asphalt Road base

Concrete

Filling

Soils



Topsoil

Peat Clay

Silty clay

Sandy clay

Gravelly clay

Shaly clay

Silt

Clayey silt

Sandy silt

Sand

Clayey sand

Silty sand

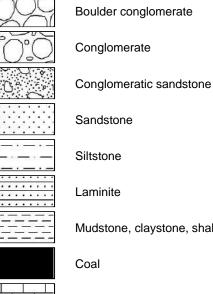
Gravel

Sandy gravel

Cobbles, boulders

Talus

Sedimentary Rocks



Mudstone, claystone, shale

Limestone

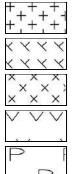
Metamorphic Rocks

Slate, phyllite, schist

Quartzite

Gneiss

Igneous Rocks



Granite

Dolerite, basalt, andesite

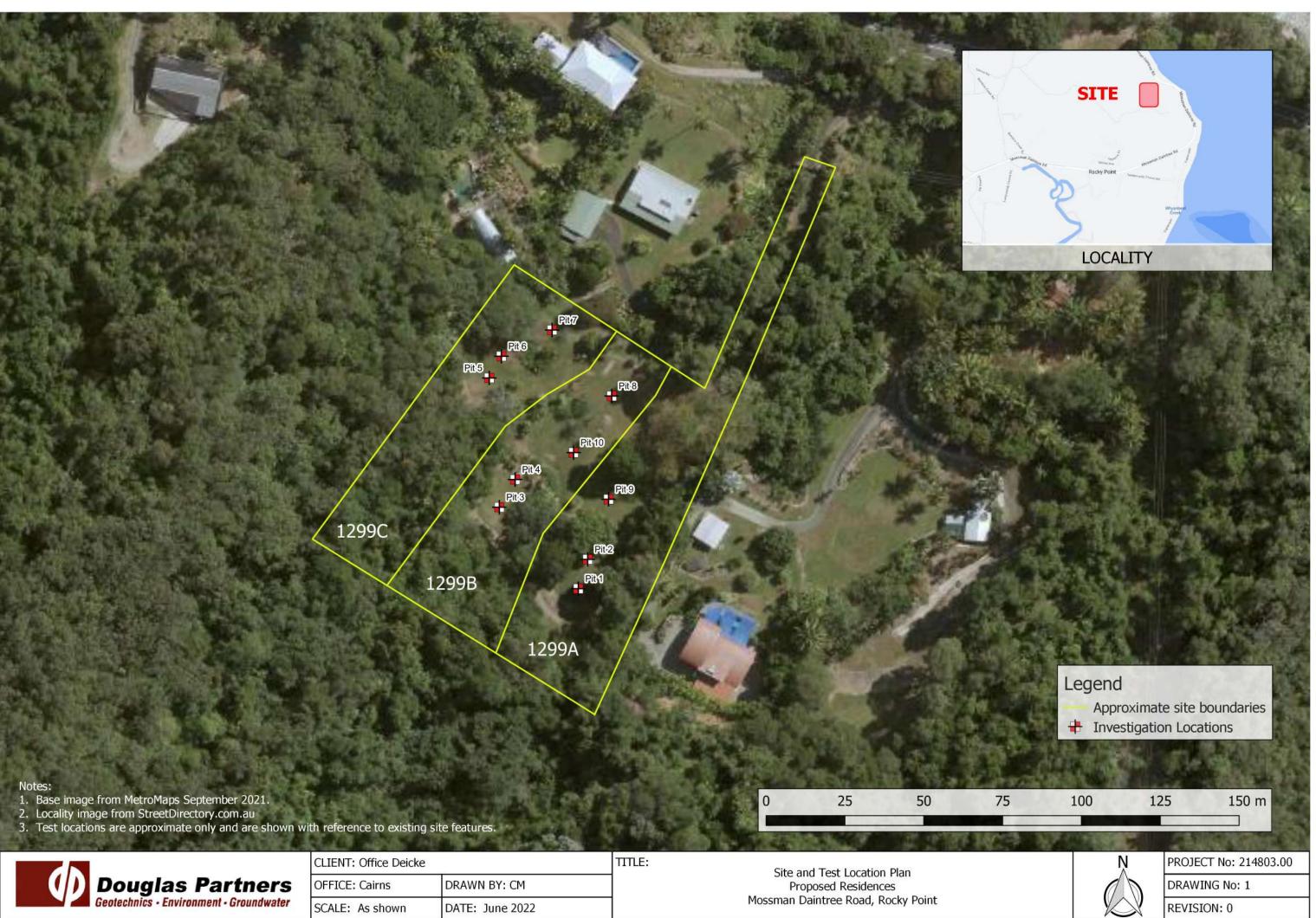
Dacite, epidote

Tuff, breccia

Porphyry

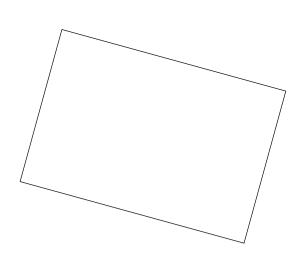
Appendix B

Drawings





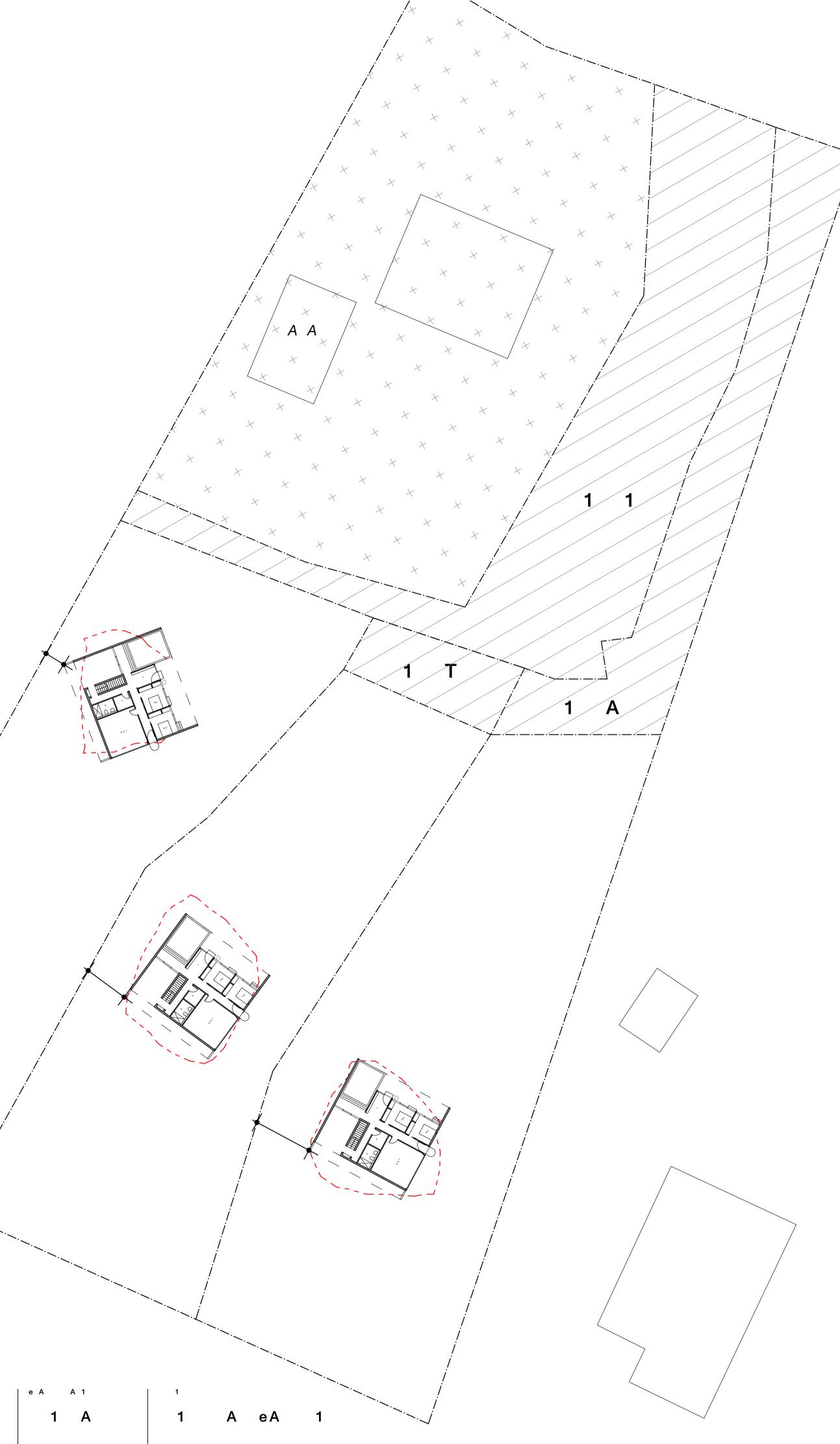
	CLIENT: Office Deicke		TITLE: Site and Test Location Plan
;	OFFICE: Cairns	DRAWN BY: CM	Proposed Residences
	SCALE: As shown	DATE: June 2022	Mossman Daintree Road, Rocky Point

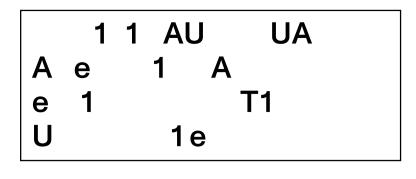


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e







o Ak c

Appendix C

Table C1 - Effluent Disposal Soil Assessment Summary Table C2 - Effluent Disposal Site Assessment Summary

Table C1 - Soil Assessment Summary

Soil Feature	Relevant System(s)	Minor Limitation	Moderate Limitation	Major Limitation	Restrictive Feature		
Depth to bedrock or hardpan (m)	Surface and sub-surface irrigation	>1.0	0.5 – 1.0	<0.5	Restricts plant growth (trees), excessive runoff, waterlogging		
	Absorption system	>1.5	1.0 – 1.5	<1.0	Groundwater pollution hazard. Resurfacing hazard		
Depth to high episodic or seasonal water table (m)*	Surface and sub-surface irrigation	>1.0	0 0.5 – 1.0		Groundwater pollution hazard. Resurfacing hazard		
	Absorption system	>1.5	1.0 – 1.5	<1.0	Potential for groundwater pollution		
Soil permeability Category	Surface and sub-surface irrigation	2b, 3 and 4	2a, 5	1 and 6	Excessive run-off, waterlogging,		
	Absorption system	3 and 4		1,2,5 and 6	percolation		
Coarse fragments (%)	All land application systems	0-20	20-40	>40	May restrict plant growth, affect trench installation		
рН	All land application systems	>6.0	4.5 - 6.0	<4.5	Reduces optimum plant growth		
Electrical conductivity (dS/m)	All land application systems	<4	4-8	>8	Excessive salt may restrict plant growth		
Sodicity (exchangeable sodium percentage) [#]	Surface and sub-surface irrigation (0-0.4m)	0-5	5-10	>10	Potential for structural degradation		
	Absorption system (0-1.2m)		0.10				
Cation exchange capacity (cmol+/kg) (0-40cm)	Surface and sub-surface irrigation	>15	5-15	<5	Unable to hold plant nutrients		
Phosphorous sorption (kg P/ha) (0-1m for irrigation) (1 m below intended base of trench)	All land application systems	>6000	2000-6000	<2000	Unable to immobilise any excess P		

Bold text indicates applicable limitation

* water table taken as 2.0 m below ground level

Adapted from NSW EPA, "Environmental and Health Protection Guidelines, on-site Sewage Management for Single Households", January 1998

Site Feature	Relevant System(s)	Minor Limitation	Moderate Limitation	Major Limitation	Restrictive Feature
	All land application systems	Rare, above 1 in 20 year flood contour		Frequent, below 1 in 20 year flood contour	Transport of wastewater off-site
Flood potential	All treatment systems	Vents, openings, and electrical components above 1 in 100 year flood contour		Vents, openings, and electrical components below 1 in 100 year flood contour	Transport of wastewater off-site. System failure and electrocution hazard
Exposure	All land application systems	High sun and wind exposure		Low sun and wind exposure	Poor evapotranspiration
	Surface irrigation	0-6	6-12	>12	
Slope %	Sub-surface irrigation	0-10	10-20	>20	Run-off, erosion
	Absorption system	0-10	10-20	>20	
Landform	All systems	Hill crests, convex side slopes and plains	Concave side slopes and foot slopes	Drainage plains and incised channels	Groundwater pollution and resurfacing hazard
Run-on and upslope seepage	All land application systems	None – Iow	Moderate	High – diversion not practical	Transport of wastewater off-site
Erosion potential	All land application systems	No signs of erosion potential present		Signs of erosion, eg rills, mass movement and slope failure present	Soil degradation and transport, system failure
Site drainage	All land application systems	No visible signs of surface dampness		Visible signs of surface dampness, such as moisture-tolerant vegetation, and seepages, soaks and springs	Groundwater pollution hazard. Resurfacing hazard
Fill	All systems	No fill	Fill present		Subsidence. Variable permeability
Land area	All systems	Area is available	Area is limited	Area is not available	Health and pollution risks
Rocks and rock outcrops (% of land surface containing boulders)	All land application systems	<10%	10-20%	>20%	Limits system performance
Geology/ Regolith	All land application systems			Major geological discontinuities, fractured or highly porous regolith	Groundwater pollution hazard

Table C2 - Site Assessment Summary

Bold text indicates applicable limitations

Adapted from NSW EPA, "Environmental and Health Protection Guidelines, on-site Sewage Management for Single Households", January 1998

Appendix D

Field Work Results

CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 38.8 m AHD
 PIT No:
 1

 EASTING:
 330539
 PROJECT

 NORTHING:
 8187979
 DATE:
 4/5

PIT No: 1 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

_	Description	jc		Sam		& In Situ Testing	<u> </u>	
고 Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
98 - 1 - 1 - 2 - 3 - 3	Silty Sandy GRAVEL GM: pale brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, loose, colluvial - medium dense below 0.4 m depth		D	0.4 0.6 0.8 1.0		pp = 170 pp = 200 pp = 180 pp = 170		
32	- boulder at 3.6 m depth							
- 3.9	Pit discontinued at 3.9m depth - limit of excavator reach	υ _α υ						

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Pho

 P
 Piston sample
 PL(A) Poir

 U
 Tube sample (x mm dia.)
 PL(D) Poir

 W
 Water sample
 PL
 Pho

 W
 Water sample
 Star
 Star

 mple
 ¥
 Water level
 V
 She

PID	Photo ionisation detector (ppm)
PL(A)	Point load axial test Is(50) (MPa)
PL(D)	Point load diametral test Is(50) (MPa)
pp	Pocket penetrometer (kPa)
pp S	Standard penetration test
V	Shear vane (kPa)

LOGGED: McDonald

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 39.6 m AHD
 PIT No:
 2

 EASTING:
 330542
 PROJECT

 NORTHING:
 8187989
 DATE:
 4/5

PIT No: 2 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

			Description	ic		Sam		& In Situ Testing	-	Dumon	Demetre		
RL	Dep (m		of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Dynamic Penetrometer Tes (blows per 100mm) 5 10 15 20				
	- - - - - - - - -		FILL / Silty Sandy GRAVEL: dark brown and pale brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, trace rootlet, moist, medium dense		D	0.3 0.5 0.6 0.9		pp = 150 pp = 140 pp = 100					
	- - - - - - - - -	1.2-	Silty Sandy GRAVEL: pale brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense, colluvial										
	-		- very dense below 2.2 m		D	2.5							
	-3	3.0 -	Pit discontinued at 3.0m depth	<u> </u>						-3			





LOGGED: McDonald

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Photo ionisation detector (ppm)

 P
 Piston sample
 PL(A) Point load axial test Is(50) (MPa)

 U,
 Tube sample (x mm dia.)
 PL(D) Point load diametral test Is(50) (MPa)

 W
 Water sample
 pp
 Pocket penetrometer (kPa)

 W
 Water seep
 S
 Standard penetration test

 mple
 ¥
 Water level
 V
 Shear vane (kPa)

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 38.8 m AHD
 PIT No:
 3

 EASTING:
 330514
 PROJECT

 NORTHING:
 8188005
 DATE:
 4/5

PIT No: 3 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

	Description	ic		Sam		& In Situ Testing		
Uepth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
	Clayey SILT ML / SILTSTONE HW: Approximately 50% pale brown, low plasticity, w <pl, (extremely<br="" hard="">weathered Argillite) and 50% pale brown, very low strength, highly weathered siltstone</pl,>		D	0.2 0.3 0.4 0.6 0.8		рр >600 рр >600 рр >600 рр >600		
- 1.	Hodgkinsons Formation Pit discontinued at 1.7m depth - refusal on very low strength siltstone	<u>. </u>	_					





LOGGED: McDonald

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Augers B Bulksa BLK Blocks

Cor

SAN	IPLING	& IN SITUTESTIN	G LEGE	:ND	
sample	G	Gas sample	PID	Photo ionisation detector (ppm)	
ample	Р	Piston sample		Point load axial test Is(50) (MPa)	I
sample	U,	Tube sample (x mm dia.)	PL(D)	Point load diametral test Is(50) (MPa)	I
Irilling	Ŵ	Water sample	pp	Pocket penetrometer (kPa)	I
ped sample	⊳	Water seep	S	Standard penetration test	I
nmental sample	Ŧ	Water level	V	Shear vane (kPa)	

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT: Office Deicke **PROJECT:** Proposed Residences LOCATION: Mossman Daintree Road, Rocky Point SURFACE LEVEL: 38.6 m AHD PIT No: 4 EASTING: 330519 **NORTHING:** 8188014

PROJECT No: 214803.00 **DATE:** 4/5/2022 SHEET 1 OF 1

			Description	<u>.</u>		Sam	pling &	& In Situ Testing	_	
RL	Dej (n		of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
37	1		FILL / Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense		D	0.2 0.4 0.5 0.6 0.8		pp = 180 pp = 150 pp = 100 pp = 90		
35 36		2.2 -	Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense, colluvial		D	3.0				





RIG: Kubota 6 tonne with 450 mm bladed bucket

₽

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

SAMPLING & IN SITU TESTING LEGEND Gas sample Piston sample Tube sample (x mm dia.) Water sample Water seep Water level G P U, W

LEGENU PID Photo ionisation detector (ppm) PL(A) Point load axial test Is(50) (MPa) PL(D) Point load diametral test Is(50) (MPa) pp Pocket penetrometer (kPa) S Standard penetration test V Shear vane (kPa)

LOGGED: McDonald

□ Sand Penetrometer AS1289.6.3.3

SURVEY DATUM: GDA94 Zone 55K

☑ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 36.6 m AHD
 PIT No:
 5

 EASTING:
 330511
 PROJECT

 NORTHING:
 8188046
 DATE:
 4/5

PIT No: 5 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

\prod	_	Description	jc		Sam		& In Situ Testing	-	Durania Danatan tan Tart
RL	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
35	- 0.5	Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense, colluvial Clayey SILT ML: brown, low plasticity, trace sand and gravel, w <pl, extremely="" hard,="" hodgkinsons<br="" weathered="">Formation SILTSTONE HW: brown and grey, very low strength</pl,>		D	0.4 0.5 0.6 0.9 1.0		рр >600 pp >600 pp >600		
	-2 2.0-	Pit discontinued at 2.0m depth - refusal on very low strength siltstone							





RIG: Kubota 6 tonne with 450 mm bladed bucket

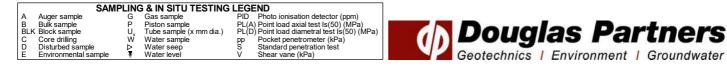
LOGGED: McDonald

SURVEY DATUM: GDA94 Zone 55K

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Dynamic penetrometer test completed at ground surface, and reattempted at 1.1 m depth

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 36.4 m AHD
 PIT No:
 6

 EASTING:
 330515
 PROJECT

 NORTHING:
 8188054
 DATE:
 4/5

PIT No: 6 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

Γ	-		Description	jc		Sam		& In Situ Testing	-			Densit		
RL	D	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water		Dynamic Penetrome (blows per 100m 5 10 15		100mr	n) 20
4	- 1 - 1 1 	2.0 -	FILL / Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense Clayey SILT ML: brown, low plasticity, trace sand and gravel, trace cobble, w <pl, residual<="" stiff,="" td="" very=""><td></td><td>D</td><td>0.5</td><td></td><td>pp range from 100-200 kPa in top 1.0 m depth</td><td></td><td></td><td></td><td></td><td></td><td></td></pl,>		D	0.5		pp range from 100-200 kPa in top 1.0 m depth						
34		3.2 -	Pit discontinued at 3.2m depth - refusal on very low strength siltstone		D	3.0				-3	۰۲			





LOGGED: McDonald

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

SAM	PLINC	3 & IN SITU TESTING	LEGE	IND
A Auger sample	G	Gas sample	PID	Photo ionisation detector (ppm)
B Bulk sample	Р	Piston sample	PL(A) Point load axial test Is(50) (MPa)
BLK Block sample	U,	Tube sample (x mm dia.)	PL(D) Point load diametral test Is(50) (MPa)
C Core drilling	Ŵ	Water sample	pp	Pocket penetrometer (kPa)
D Disturbed sample	⊳	Water seep	S	Standard penetration test
E Environmental sample	Ŧ	Water level	V	Shear vane (kPa)

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



Office Deicke CLIENT: **PROJECT: Proposed Residences** LOCATION: Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 31 m AHD EASTING: 3305314 **NORTHING:** 8188062

PIT No: 7 PROJECT No: 214803.00 **DATE:** 4/5/2022 SHEET 1 OF 1

		Description	ic.		Sam	pling &	& In Situ Testing	_				
1 RL	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	(blow	Peneti /s per ¹⁰	romete 100mm 15	1 est 1) 20
	- - - - - - - - - - - - - - - - - - -	Topsoil / Clayey SILT ML: dark brown, low plasticity, with organics (rootlets), trace sand and gravel, w~PL, firm Clayey SILT ML: brown, low plasticity, with subrounded and subangular sand and gravel, w <pl, colluvial<br="" stiff,="">Silty CLAY CI: red brown, medium plasticity, trace sand and gravel, very stiff, possibly residual</pl,>		D	0.1 0.3 0.4 0.5 0.7 0.9		pp = 100 pp = 180 pp = 220 pp = 190 pp = 200					
29-	-2 2.0	Pit discontinued at 2.0m depth										





RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Pho

 P
 Piston sample
 PIL(A) Poir
 PL(A) Poir

 U
 Tube sample (x mm dia.)
 PL(A) Poir
 PL(A) Poir

 W
 Water sample (x mm dia.)
 PL(D) Poir
 Pu poir

 W
 Water sample
 X mm dia.)
 Star

 mple
 ¥
 Water level
 V

LEGE	
	Photo ionisation detector (ppm)
PL(A)	Point load axial test Is(50) (MPa)
PL(D)	Point load axial test Is(50) (MPa) Point load diametral test Is(50) (MPa)
	Pocket penetrometer (kPa)
pp S	Standard penetration test
V	Shear vane (kPa)

LOGGED: McDonald

□ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 32 m AHD EASTING: 330550 NORTHING: 8188041 PIT No: 8 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

		Description	Li		Sam	pling &	& In Situ Testing		
2 RL	Depti (m)	th st	Graphic Log	Type	Depth	to be the second secon	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20		
	- - (Topsoil / Clayey SILT ML: dark brown, low plasticity, with 0.2 organics (rootlets), trace sand and gravel, w~PL, stiff	X	D	0.3		pp = 180		
		Clayey SILT ML: brown, low plasticity, with subangular and subrounded sand and gravel, trace cobble, w~PL, stiff, colluvial		D	0.5		pp = 200		
		- becoming w>PL below 0.4 m depth	,,,,,,						
31	- 1 -		 	D	0.9 1.0		pp = 190		
-									
		- very stiff grading to hard below 1.4 m depth	////						
30 .	- - 2		///// /////	D	1.8				
3	- 2		 						
-	-			D	2.5				
	- -		1111						
29	- -33	3.0 Pit discontinued at 3.0m depth						-	3





RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Pho

 P
 Piston sample
 PL(A) Poir
 U,

 U
 Tube sample (x mm dia.)
 PL(D) Poir

 W
 Water sample
 PL
 Pho

 W
 Water sample
 Star

 mple
 ¥
 Water level
 V

G.	LEGE	
	PID	Photo ionisation detector (ppm)
	PL(A)	Point load axial test Is(50) (MPa)
	PL(D)	Point load diametral test Is(50) (MPa)
	pp	Pocket penetrometer (kPa)
	pp S	Standard penetration test
	V	Shear vane (kPa)

LOGGED: McDonald

SURVEY DATUM: GDA94 Zone 55K

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 35 m AHD EASTING: 330549 NORTHING: 8188008 PIT No: 9 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

		Description		Sampling &			& In Situ Testing	Ļ				
	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20			
, - -	0.2-	Topsoil / Clayey SILT ML: dark brown, low plasticity, with organics (rootlets), trace sand and gravel, w>PL, firm to stiff			0.3		pp = 220					
F		Clayey SILT ML: brown, low plasticity, trace subrounded and subangular sand and gravel, trace cobble, w>PL, stiff, colluvial		D	0.5 0.6		pp = 190					
	1	- with subrounded to subangular sand and gravel and subangular to subrounded cobble, below 1.1 m depth		D	0.9 1.0		pp = 240					
	2	- hard below 1.9 m depth							-2			
	3								-3			
-	3.9 -	Pit discontinued at 3.9m depth - limit of excavator reach										





RIG: Kubota 6 tonne with 450 mm bladed bucket

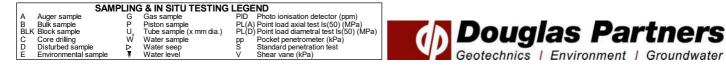
LOGGED: McDonald

SURVEY DATUM: GDA94 Zone 55K

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Dynamic penetrometer test completed at ground surface, and reattempted at 1.0 m depth

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



TEST PIT LOG

CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 34 m AHD EASTING: 330538 NORTHING: 8188023 PIT No: 10 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

pth n)	of				·p	& In Situ Testing	L .		_			
	Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water			nic Pene lows pe		
0.15	Topsoil Clayey / SILT ML: dark brown, low plasticity, with $\startin and$ gravel, with organics (rootlets) w>PL, firm to stiff /			0.2		nn - 150		- L	5			
	Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial	 	D	0.5 0.6		pp = 150		ן ר	- L		-	• • • •
	- w>PL below 0.8 m depth	 	D	0.9 1.0		pp = 150						
		/ / / / / / / / / / / /	D	1.5								
	- hard below 1.7 m depth							-2				
0.7			D	2.5				-				•
2.1	Pit discontinued at 2.7m depth											•
												•
	2.7 -	 ^{0.15} sand and gravel, with organics (rootlets) w>PL, firm to stiff/ Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial w>PL below 0.8 m depth hard below 1.7 m depth 	 ^{0.15} sand and gravel, with organics (rootlets) w>PL, firm to stiff Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial w>PL below 0.8 m depth - hard below 1.7 m depth 	 ^{0.15} sand and gravel, with organics (rootlets) w>PL, firm to stiff Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial w>PL below 0.8 m depth hard below 1.7 m depth 	0.15 sand and gravel, with organics (rootlets) w>PL, firm to stiff/ 0.3 Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial 0.3 - w>PL below 0.8 m depth 0.9 - hard below 1.7 m depth 0 2.5 2.5	0.15 sand and gravel, with organics (rootlets) w>PL, firm to stiff 0.3 Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial 0.3 - w>PL below 0.8 m depth 0.9 - hard below 1.7 m depth 0 2.7 0	0.15 saind and gravel, with organics (rootlets) w>PL, firm to stiff 0.3 pp = 150 Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial 0 0.5 0.6 pp = 200 - w>PL below 0.8 m depth 0 0 0 0.9 0.9 pp = 150 - hard below 1.7 m depth 0 1.5 0 0 1.5 0	0.15 sand and gravel, with organics (rootlets) w>PL, firm to stiff, 0.3 pp = 150 Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial 0 0.5 0.6 pp = 200 - w>PL below 0.8 m depth 0.9 0 1.0 0 1.0 pp = 150 - hard below 1.7 m depth 0 0 1.5 0 2.5 0	0.15 sand and gravel, with organics (rootlets) w>PL, firm to stiff, 0.3 pp = 150 Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial 0.3 0.5 pp = 200 - w>PL below 0.8 m depth 0 0.9 0.9 pp = 150 - hard below 1.7 m depth 0 1.5 0 2.5	0.15 sand and gravel, with organics (rootlets) w>PL, firm to stiff 0.3 pp = 150 Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial 0 0.5 0.6 pp = 200 - w>PL below 0.8 m depth D 0.9 0.9 pp = 150 1 - hard below 1.7 m depth D 1.5 0 2.5	0.15 sand and gravel, with organics (rootlets) w>PL, firm to stiff 0.3 pp = 150 Clayey SILT ML: brown, low plasticity, with subangular to subrounded sand and gravel, trace cobble, w=PL, stiff, colluvial 0.3 pp = 200 - w>PL below 0.8 m depth 0.9 0.9 pp = 150 - hard below 1.7 m depth 0 1.5 0 27 0 2.5 0 2.5	27





RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Phot

 P
 Piston sample
 PL(A) Poin
 PL(D) Poin

 U
 Tube sample (x mm dia.)
 PL(D) Poin

 W
 Water sample
 P
 Stan

 Mixture
 Water seep
 S
 Stan

 Imple
 ¥
 Water level
 V
 Sheat

PID Photo ionisation detector (ppm) PL(A) Point load axial test Is(50) (MPa) PL(D) Point load diametral test Is(50) (MPa) pp Pocket penetrometer (kPa) S Standard penetration test V Shear vane (kPa)

LOGGED: McDonald

SURVEY DATUM: GDA94 Zone 55K

□ Sand Penetrometer AS1289.6.3.3



Appendix E

Laboratory Test Results

Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777A
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 16/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 1 , Depth: 0.4
Material:	Soil & Cobble

Particle Size Distribution (AS1289 3.6.1)

T article Olze Distributio	11 () 12 12 00 0.0.1 ()	
Sieve	Passed %	Passing Limits
37.5 mm	100	
26.5 mm	94	
19 mm	84	
13.2 mm	80	
9.5 mm	73	
6.7 mm	69	
4.75 mm	66	
2.36 mm	62	
1.18 mm	57	
0.6 mm	52	
0.425 mm	49	
0.3 mm	45	
0.15 mm	38	
0.075 mm	32	
Moisture Content (AS 1	289 2 1 1)	
Moisture Content (%)	205 2.1.1	14.1

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Douglas Partners Pty Ltd Townsville Laboratory 29 Civil Road Garbutt QLD 4814 Phone: (07) 4779 9866

Email: Townsville@douglaspartners.com.au



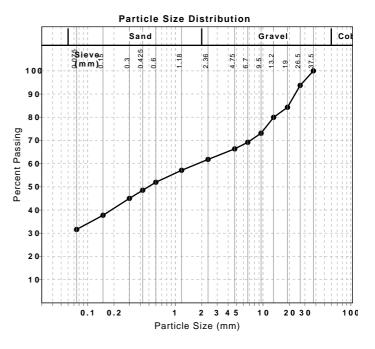
Accredited for compliance with ISO/IEC 17025 - Testing

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Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828



Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777B
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 16/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 4 , Depth: 0.5
Material:	Soil & Cobble

Do rticle Size Distribution (AS1289 3.6.1)

	· · · · · · · · · · · · · · · · · · ·	D 1 1 1 1
Sieve	Passed %	Passing Limits
53 mm	100	
37.5 mm	84	
26.5 mm	70	
19 mm	68	
13.2 mm	62	
9.5 mm	58	
6.7 mm	55	
4.75 mm	52	
2.36 mm	48	
1.18 mm	45	
0.6 mm	42	
0.425 mm	41	
0.3 mm	39	
0.15 mm	35	
0.075 mm	29	

Atterberg Limit (AS1289 3.1.2 & 3.2	.1 & 3.3.1)	Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	28		
Plastic Limit (%)	23		
Plasticity Index (%)	5		
Linear Shrinkage (AS1289 3.4.1)		Min	Max
Linear Shrinkage (AS1289 3.4.1) Moisture Condition Determined By	AS 1289.3.1.2	Min	Max
Ŭ (AS 1289.3.1.2 3.5	Min	Max
Moisture Condition Determined By			Max
Moisture Condition Determined By Linear Shrinkage (%)	3.5		Max

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Email: Townsville@douglaspartners.com.au



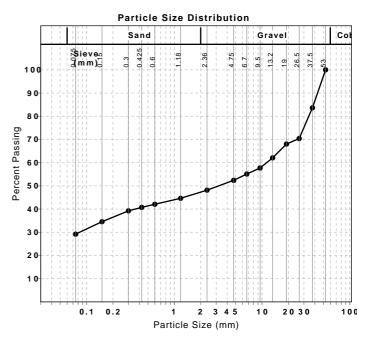
Accredited for compliance with ISO/IEC 17025 - Testing

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Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828



Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777C
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 16/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 5, Depth: 1.0
Material:	Clayey Silt

Atterberg Limit (AS1289 3.1.2 & 3.2	.1 & 3.3.1)	Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	34		
Plastic Limit (%)	23		
Plasticity Index (%)	11		
Linear Shrinkage (AS1289 3.4.1)		Min	Max
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	4.5		
Cracking Crumbling Curling Crackin			
Cracking Crumbling Curling	Crackin	g	
Cracking Crumbling Curling Moisture Content (AS 1289 2.1.1)	Crackin	g	

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Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828

Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777D
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 13/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 8 , Depth: 0.3
Material:	Clayey Silt

Atterberg Limit (AS1289 3.1.2 & 3.2	2.1 & 3.3.1)	Min	Max	
Sample History	Oven Dried			
Preparation Method	Dry Sieve			
Liquid Limit (%)	24			
Plastic Limit (%)	19			
Plasticity Index (%) 5				
Linear Shrinkage (AS1289 3.4.1)		Min	Max	
Moisture Condition Determined By	AS 1289.3.1.2			
Linear Shrinkage (%)	2.5			
Cracking Crumbling Curling Crackin				
Moisture Content (AS 1289 2.1.1)				
Moisture Content (AS 1289 2.1.1)				

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Janya J lley

Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828



Envirolab Services Pty Ltd ABN 37 112 535 645 12 Ashley St Chatswood NSW 2067 ph 02 9910 6200 fax 02 9910 6201 customerservice@envirolab.com.au www.envirolab.com.au

CERTIFICATE OF ANALYSIS 294891

Client Details	
Client	Douglas Partners (Cairns) Pty Ltd
Attention	Aidan McDonald
Address	13 Industrial Ave, Stratford, QLD, 4870

Sample Details	
Your Reference	214803.00 Rocky Point, Proposed Residences
Number of Samples	3 Soil
Date samples received	06/05/2022
Date completed instructions received	06/05/2022

Analysis Details

Please refer to the following pages for results, methodology summary and quality control data.

Samples were analysed as received from the client. Results relate specifically to the samples as received.

Results are reported on a dry weight basis for solids and on an as received basis for other matrices.

Please refer to the last page of this report for any comments relating to the results.

Report Details			
Date results requested by	16/05/2022		
Date of Issue	16/05/2022		
NATA Accreditation Number 2901. This document shall not be reproduced except in full.			
Accredited for compliance with	SO/IEC 17025 - Testing. Tests not covered by NATA are denoted with *		

Results Approved By Diego Bigolin, Inorganics Supervisor Giovanni Agosti, Group Technical Manager Authorised By

Nancy Zhang, Laboratory Manager

Envirolab Reference: 294891 Revision No: R00



Page | 1 of 9

Misc Inorg - Soil				
Our Reference		294891-1	294891-2	294891-3
Your Reference	UNITS	Pit 7	Pit 9	Pit 10
Depth		1.7	1.5	1
Date Sampled		04/05/2022	04/05/2022	04/05/2022
Type of sample		Soil	Soil	Soil
Date prepared	-	12/05/2022	12/05/2022	12/05/2022
Date analysed	-	12/05/2022	12/05/2022	12/05/2022
pH 1:5 soil:water	pH Units	5.2	5.2	5.4
Electrical Conductivity 1:5 soil:water	µS/cm	48	51	45
Total Nitrogen in Soil	mg/kg	170	180	270
Sodium Adsorption Ratio		1.2	1.8	0.56
Emerson Class No.	-	5.0	6.0	5.0
Phosphorus Sorption Capacity	mg/kg	670	560	530

ESP/CEC				
Our Reference		294891-1	294891-2	294891-3
Your Reference	UNITS	Pit 7	Pit 9	Pit 10
Depth		1.7	1.5	1
Date Sampled		04/05/2022	04/05/2022	04/05/2022
Type of sample		Soil	Soil	Soil
Date prepared	-	12/05/2022	12/05/2022	12/05/2022
Date analysed	-	13/05/2022	13/05/2022	13/05/2022
Exchangeable Ca	meq/100g	0.2	0.1	0.5
Exchangeable K	meq/100g	<0.1	<0.1	<0.1
Exchangeable Mg	meq/100g	0.1	<0.1	0.2
Exchangeable Na	meq/100g	<0.1	<0.1	<0.1
Cation Exchange Capacity	meq/100g	<1	<1	<1
ESP	%	<1	<1	<1

Method ID	Methodology Summary
Ext-037	Analysed by Sydney Environmental & Soil Laboratory
Ext-062	Analysed by East West Enviroag
Inorg-001	pH - Measured using pH meter and electrode in accordance with APHA latest edition, 4500-H+. Please note that the results for water analyses are indicative only, as analysis outside of the APHA storage times.
Inorg-002	Conductivity and Salinity - measured using a conductivity cell at 25°C in accordance with APHA latest edition 2510 and Rayment & Lyons.
INORG-127	Total Nitrogen by high temperature catalytic combustion with chemiluminescence detection.
Metals-020	Determination of exchangeable cations and cation exchange capacity in soils using 1M Ammonium Chloride exchange and ICP-OES analytical finish.
Metals-020	Calcium and Magnesium analysed by ICP-AES and SAR calculated.

QUALITY CONTROL: Misc Inorg - Soil						Du	plicate		Spike Re	covery %
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-1	[NT]
Date prepared	-			12/05/2022	1	12/05/2022	12/05/2022		12/05/2022	
Date analysed	-			12/05/2022	1	12/05/2022	12/05/2022		12/05/2022	
pH 1:5 soil:water	pH Units		Inorg-001	[NT]	1	5.2	5.1	2	100	
Electrical Conductivity 1:5 soil:water	µS/cm	1	Inorg-002	<1	1	48	39	21	104	
Total Nitrogen in Soil	mg/kg	10	INORG-127	<10	1	170	170	0	92	
Sodium Adsorption Ratio		0.01	Metals-020	<0.01	1	1.2	[NT]		113	
Emerson Class No.	-	0	Ext-037	[NT]	1	5.0	[NT]		[NT]	
Phosphorus Sorption Capacity	mg/kg	na	Ext-062	[NT]	1	670	[NT]		[NT]	

QUALITY CONTROL: ESP/CEC						Du	plicate		Spike Re	covery %
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date prepared	-			12/05/2022	[NT]		[NT]	[NT]	12/05/2022	
Date analysed	-			13/05/2022	[NT]		[NT]	[NT]	13/05/2022	
Exchangeable Ca	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	84	
Exchangeable K	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	95	
Exchangeable Mg	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	88	
Exchangeable Na	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	97	
ESP	%	1	Metals-020	<1	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]

Result Definiti	ons
NT	Not tested
NA	Test not required
INS	Insufficient sample for this test
PQL	Practical Quantitation Limit
<	Less than
>	Greater than
RPD	Relative Percent Difference
LCS	Laboratory Control Sample
NS	Not specified
NEPM	National Environmental Protection Measure
NR	Not Reported

Quality Contro	Quality Control Definitions						
Blank	This is the component of the analytical signal which is not derived from the sample but from reagents, glassware etc, can be determined by processing solvents and reagents in exactly the same manner as for samples.						
Duplicate	This is the complete duplicate analysis of a sample from the process batch. If possible, the sample selected should be one where the analyte concentration is easily measurable.						
Matrix Spike	A portion of the sample is spiked with a known concentration of target analyte. The purpose of the matrix spike is to monitor the performance of the analytical method used and to determine whether matrix interferences exist.						
LCS (Laboratory Control Sample)	This comprises either a standard reference material or a control matrix (such as a blank sand or water) fortified with analytes representative of the analyte class. It is simply a check sample.						
Surrogate Spike	Surrogates are known additions to each sample, blank, matrix spike and LCS in a batch, of compounds which						

Australian Drinking Water Guidelines recommend that Thermotolerant Coliform, Faecal Enterococci, & E.Coli levels are less than 1cfu/100mL. The recommended maximums are taken from "Australian Drinking Water Guidelines", published by NHMRC & ARMC 2011.

are similar to the analyte of interest, however are not expected to be found in real samples.

The recommended maximums for analytes in urine are taken from "2018 TLVs and BEIs", as published by ACGIH (where available). Limit provided for Nickel is a precautionary guideline as per Position Paper prepared by AIOH Exposure Standards Committee, 2016.

Guideline limits for Rinse Water Quality reported as per analytical requirements and specifications of AS 4187, Amdt 2 2019, Table 7.2

Laboratory Acceptance Criteria

Duplicate sample and matrix spike recoveries may not be reported on smaller jobs, however, were analysed at a frequency to meet or exceed NEPM requirements. All samples are tested in batches of 20. The duplicate sample RPD and matrix spike recoveries for the batch were within the laboratory acceptance criteria.

Filters, swabs, wipes, tubes and badges will not have duplicate data as the whole sample is generally extracted during sample extraction.

Spikes for Physical and Aggregate Tests are not applicable.

For VOCs in water samples, three vials are required for duplicate or spike analysis.

Duplicates: >10xPQL - RPD acceptance criteria will vary depending on the analytes and the analytical techniques but is typically in the range 20%-50% – see ELN-P05 QA/QC tables for details; <10xPQL - RPD are higher as the results approach PQL and the estimated measurement uncertainty will statistically increase.

Matrix Spikes, LCS and Surrogate recoveries: Generally 70-130% for inorganics/metals (not SPOCAS); 60-140% for organics/SPOCAS (+/-50% surrogates) and 10-140% for labile SVOCs (including labile surrogates), ultra trace organics and speciated phenols is acceptable.

In circumstances where no duplicate and/or sample spike has been reported at 1 in 10 and/or 1 in 20 samples respectively, the sample volume submitted was insufficient in order to satisfy laboratory QA/QC protocols.

When samples are received where certain analytes are outside of recommended technical holding times (THTs), the analysis has proceeded. Where analytes are on the verge of breaching THTs, every effort will be made to analyse within the THT or as soon as practicable.

Where sampling dates are not provided, Envirolab are not in a position to comment on the validity of the analysis where recommended technical holding times may have been breached.

Measurement Uncertainty estimates are available for most tests upon request.

Analysis of aqueous samples typically involves the extraction/digestion and/or analysis of the liquid phase only (i.e. NOT any settled sediment phase but inclusive of suspended particles if present), unless stipulated on the Envirolab COC and/or by correspondence. Notable exceptions include certain Physical Tests (pH/EC/BOD/COD/Apparent Colour etc.), Solids testing, total recoverable metals and PFAS where solids are included by default.

Samples for Microbiological analysis (not Amoeba forms) received outside of the 2-8°C temperature range do not meet the ideal cooling conditions as stated in AS2031-2012.

Report Comments

Emerson Class No. & Phosphorus Sorption Capacity analysed by East West Geo Ag Enviro. Report No. EW220977



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ENGINEERS

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ONSITE & SOIL EVALUATION FOR EFFLUENT DISPOSAL

FOR

PROPOSED RESIDENCES MOSSMAN-DAINTREE ROAD ROCKY POINT

Prepared for:Office DeickeProject no:BR220116Date:24th June 2022Revision:02





Revisions

Date	Issue	Revision Description Prepared By		Reviewed By	Approved By	
22.06.22	01	Draft Issue	David Colmer	Karl Paton	David Colmer	
24.06.22	02	Final Issue	David Colmer	Karl Paton	David Colmer	

It is the responsibility of the reader to verify the currency of the version number of this report. All subsequent releases will be made directly to the Client.

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1 INTRODUCTION

ACOR Consultants have been engaged by Office Deicke to prepare an On-site and Soil Evaluation (OSE) for the purposes of effluent disposal on an unsewered property at 1299 Mossman-Daintree Road, Rocky Point.

This OSE addresses management of disposal of effluent generated from the domestic waste stream from three (3) individual residences located on three (3) separate but adjoining allotments in accordance with the "Queensland Plumbing and Wastewater Code" and AS/NZS 1547:2012 "On-site domestic-wastewater management".

2 PROPOSED DEVELOPMENT

The project comprises construction of three (3) individual residences located on three (3) separate but adjoining allotments Lot 2, Lot 3 and Lot 4 of the subdivided property at 1299 Mossman Daintree Road. Each residence to be two storey and constructed on the existing building platforms provisioned when the property was subdivided.

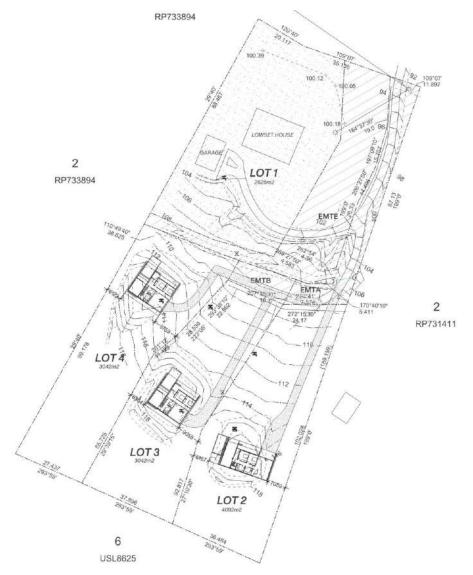


Figure 2.1 – Proposed Site Locality

3 SITE INVESTIGATION

A site and soil investigation was undertaken on 4 May 2022 by Douglas and Partners. The investigation included the excavation of ten test pits and laboratory testing of selected samples. The details of the field and laboratory work are presented in Appendix B – Geotechnical Report.

The investigation assessed the subsurface soil and groundwater conditions to provide:

- site classification to AS 2870 (2011)
- allocation of soil categories and determination of design soil permeability and design effluent loading rates based on AS 1547 (2012)

3.1 Location & Site Description

 Street Address
 1299A, 1299B and 1299C Mossman-Daintree Road, Rocky Point QLD 4874.

 Site Area
 Lot 2 3745m2

 Lot 3 3309m2
 Lot 4 3042m2

 Local Authority
 Douglas Shire Council

 Existing Land Use
 Vacant (part of a recently completed subdivision)

3.2 Topography & Features

A site survey is presented in Appendix A – Site Survey Plan, including contours and levels over the site.

The site comprises three adjoining allotments within a battle-axe area above an existing residence (refer Appendix A - Site Survey Plan). Each individual lot is approximately rectangular in shape.

The entire site (3 lots) is situated on the north-eastern facing foot slopes of the Dagmar Range and site elevations approximately range from 35 AHD to 55 AHD.

The southwestern portion of the site (approximately 25%) comprises steeply sloping hillside vegetated by dense rainforest. Previous earthworks have been undertaken to prepare a building platform at each of the three allotments using localised cut and filling. The natural slope gradients downhill from each building platform at a range of between 10° to 15°.

An open unlined drain has been excavated along much of the site's eastern boundary along the driveway, presumably to divert overland flows down to the road reserve.

Deeply eroded ephemeral water courses / surface drains were observed to the east and north of site indicating the likelihood of significant overland flows during and following significant rainfall events.

The site is bordered by residential properties downhill to the north, to the east and to the west, and steeply sloping, densely vegetated slopes uphill to the south.

3.3 Vegetation

The southwestern portion of the site (approximately 25%) comprises steeply sloping hillside vegetated by dense rainforest.

The main portion of the site, below the south-western steep slopes, has been cleared of native vegetation and is generally vegetated by short grass with several large trees and shrubs around the building platforms.

3.4 Soils

Subsurface conditions may be grouped into two areas for discussion, the Modified Areas or building platforms and the Natural Areas downslope of the building platforms.

- Modified Areas: Fill comprising medium dense clayey sandy gravel was encountered to depths of between 1.2 m and 2.2 m at the building platforms. Natural materials encountered within the building platform test locations are broadly characterised by colluvial clayey or silty sandy gravel with cobbles (generally assessed to be medium dense or denser).
- Natural Areas: The downslope hillside test locations generally comprised a shallow (0.15 m or 0.2 m thick) layer of firm or stronger clayey silt topsoil overlying stiff or stronger clayey silt.

The Geotechnical Report states that the controlling soil for on-site effluent disposal by traditional systems is likely to be the residual clayey silt due to its low permeability and poor soil structure. Based on Table 5.1 of (AS 1547, 2012), for a weakly structured Category 4 soil an indicative permeability of between 0.12 and 0.5 m/day is suggested.

Free groundwater was not observed in any of the test pits.

4 WASTEWATER SOURCES AND QUANTITY

Wastewater generated from the proposed development is anticipated to be of a domestic nature only. Tradewaste discharge (such as oils, paints, etc) is not expected to be generated from any residence. The owners are to consult with the manufacturer of the wastewater treatment plant to ensure that any waste disposed of to the sewer system is capable of being treated by the plant.

Domestic wastewater flow design allowances for each proposed residence are presented below table in accordance with AS:1547 (2012) Table H1 and equivalent Queensland Plumbing and Wastewater Code Table T2.

 Table 4.1 – Domestic Wastewater Flow Design Allowance

Source	Number of Persons	Total Flow (I/person/day)
3 Bedroom Residence	4	600

5 WASTEWATER TREATMENT AND DISPOSAL

5.1 Site Factors

The following factors are identified as those that will govern the level of effluent treatment and the land application system type (Absorption Beds and Trenches, Evapotranspiration Area, Mounds, Subsoil Drip Irrigation and Surface Irrigation).

5.1.1 Soil Category

The soil category for this site is category 4 and comprised a shallow layer of firm clayey topsoil with low permeability. Mounds are suitable for this application.

5.1.2 Setback distances

A 10m offset distance is required from the intermittent water course found on the site provided advanced secondary treatment sewage plant is used. This distance increases for sewage treatment plants that discharge a lower quality effluent.

5.1.3 Vegetation

The main portion of the site, below the south-western steep slopes, has mostly been cleared of native vegetation and is generally vegetated by short grass with several large trees and shrubs around the building platforms. There are significant large trees that will cast shadow over the available areas for effluent application.

5.1.4 Slope gradient

Most of the proposed effluent disposal area is moderately sloped at 10-15%. Although Subsoil Drip Irrigation is most suitable for this application (up to 30°), Mounds are also suitable up to 15°.

5.1.5 Land availability

Due to the steep slope and vegetation at the rear of the sites the land available for land application will be limited. This is further limited by the siting of the building pad for Lot 4. Mound beds with a smaller footprint will be the preferred option for effluent disposal.

5.1.6 Climate factors

The site is in a wet climate. Mounds are preferred in this instance.

5.2 **Proposed Pre-Treatment**

Water reduction facilities are recommended to be installed to all fixtures discharging to the wastewater treatment plant. Note that water reduction facilities will be required under Queensland Development Code MP4.1 – Sustainable Buildings and include installation of 3 star WELS water saving shower roses, 4 Star WELS dual flush (3/4.5 litre) flush toilets, and 3 Star WELS tapware to all other fixtures at minimum. The owners can elect to use further reduction facilities such as front load washing machines, higher WELS rated tapware, and aerator faucets to further reduce hydraulic loading.

5.3 **Proposed Treatment**

To ensure that horizontal separation distances are minimised, and the land application area can adequately fit on the site, it is proposed to utilise a proprietary treatment plant capable of achieving effluent to an advanced secondary standard.

The owners are to ensure the treatment plant selected is approved by the Local Authority and has Chief Executive Approval under the Queensland Plumbing and Wastewater Code.

A maintenance agreement between the property owners and the manufacturer of the treatment plant, or authorized service agent, is to be entered into to ensure the continued proper operation of the treatment plant and that it complies with the secondary effluent compliance criteria in accordance with the Queensland Plumbing and Wastewater Code Appendix 1 (and below).

Parameter	Level
Biochemical Oxygen Demand (BOD₅)	90% of samples taken over test period must have a BOD ₅ less than 10g/m ³ with no sample greater than 20g/m ³
Total Suspended Solids (TSS)	90% of samples taken over test period must have a BOD ₅ less than 30g/m ³ with no sample greater than 20g/m ³
Thermotolerant Coliform (org/100mL)	90% of samples taken over test period must have a thermotolerant coliform count not exceeding 10 organisms per 100mL with no sample exceeding 200 organisms per 100mL.
Chlorination	Where chlorination is used the total chlorination must be greater than or equal to 0.5g/m ³ and less than 2.0g/m ³ in four

Table 5.1 – Advanced Secondary Effluent Compliance Criteria

out of five sample taken.

5.4 **Proposed Land Application Area**

Mounds are proposed to be provided for disbursement of the (treated) advanced secondary effluent. The northern portion of each site has been adopted as the location of the land application area.

The required irrigation area is 171m2 which has been calculated from recommended design irrigation rates in Table 4.2A4 of AS/NZS 1547:2012. Refer to Appendix F for calculations and assumptions. A reserve area of 171m2, representing 100% of the design area, can be accommodated on the site and is to be used for resting of the land-application system, if required, or for duplication of the land application system due to unforeseen circumstances.

The area set aside for subsurface irrigation shall be prepared in accordance with AS/NZS 1547:2012 Appendix M Refer to Appendix B for a copy of the Effluent Disposal Plan, which provides a layout and details of the effluent disposal areas to ensure compliance with AS/NZS 1547:2012. There are proposed to be two irrigation zones for each allotment where an automatic distribution valve alternates irrigation between these areas. This allows for drying out of each area between irrigation events, rather than having a constantly wet area that gets dosed every irrigation event.

The irrigation area shall be planted to ensure the uptake of nutrients and to promote evapotranspiration. Australian natives are typically adversely affected if exposed to regular periods of effluent irrigation and a local nursery should be consulted for types and species of plants suitable for the land application area. The owner is to ensure that any trees planted near the irrigation areas do not cast excessive shade of the irrigation area when mature.

Diversion drains and/or cut off drains are to be installed upslope of all irrigation areas to direct stormwater runoff around the irrigation area. This will also aid in delineating the land application area. Warning signs complying with AS/NZS:1319 at the boundaries of the irrigation area, in at least two places, clearly visible to the occupants with wording such as "Recycled Water – Avoid Contact – DO NOT DRINK" are to be installed.

5.5 Separation Distances

The subsurface land application area shall have minimum separation distances as specified in Table T4 and T7 of the Queensland Plumbing and Wastewater Code. Refer to Table 5.2 and Table 5.3 below which reproduces these required separation distances.

Feature	Horizontal Separation Distance (metres)		
	Up slope	Down slope	Level
Property boundaries, pedestrian paths, walkways, recreation areas, retaining wall, and footings for buildings and other structures.	2	4	2
Inground swimming pools	6	6	6
Inground potable water tank not exposed to primary effluent	6	6	6

Table 5.2 – Setback Distances for Subsurface Land Application Area

Inground potable water tank exposed to primary effluent	15	15	15
---	----	----	----

(Ref: Table T4 Queensland Plumbing and Wastewater Code)

Table 5.3 – Setback Distances for On-site Sewerage Facilities – Protection of Surface Water and Groundwater

Feature	Horizontal Separation Distance (metres)
	Advanced Secondary
Top of bank of permanent water course	10
Top of bank of intermittent water course	
Top of bank of a lake, bay or estuary	
Top water level of a surface water source used for agriculture, aquaculture or stock purposes	
Open stormwater drainage channel or drain	
Bore or a dam	
Unsaturated soil depth to a permanent water table (vertically)	0.3

(Ref: Table T7 Queensland Plumbing and Wastewater Code)

Refer to Appendix B for a copy of the Effluent Disposal Plan, which provide separation distances on the site plan to ensure compliance with the Queensland Plumbing and Wastewater Code.

6 OPERATION AND MAINTENANCE

It is the responsibility of the property owner to ensure the effluent generated does not cause nuisance via seepage, runoff, drift, or spray to adjoin properties or water courses

The property owner is responsible for the operation and maintenance of the installation. This includes engaging an authorized service agent to maintain the facility at appropriate service intervals.

The wastewater treatment plant is to be installed and operated in accordance with the manufacturer's specifications, instructions, and recommendations.

The surface irrigated land application area shall be maintained by the property owner to ensure maximum uptake of nutrients and evapotranspiration. This will include mowing of grassed areas, pruning of all plants as required, weeding and replacement of dead plants as required.

Irrigation of food crops with secondary treated effluent shall not occur.

7 CONCLUSION

Based on the architectural plans presented in Appendix D, the wastewater generated from the proposed 3x residences will be capable of being disposed of on-site in a safe manner in accordance with the Queensland Plumbing and Wastewater Code and AS:1547 (2012).

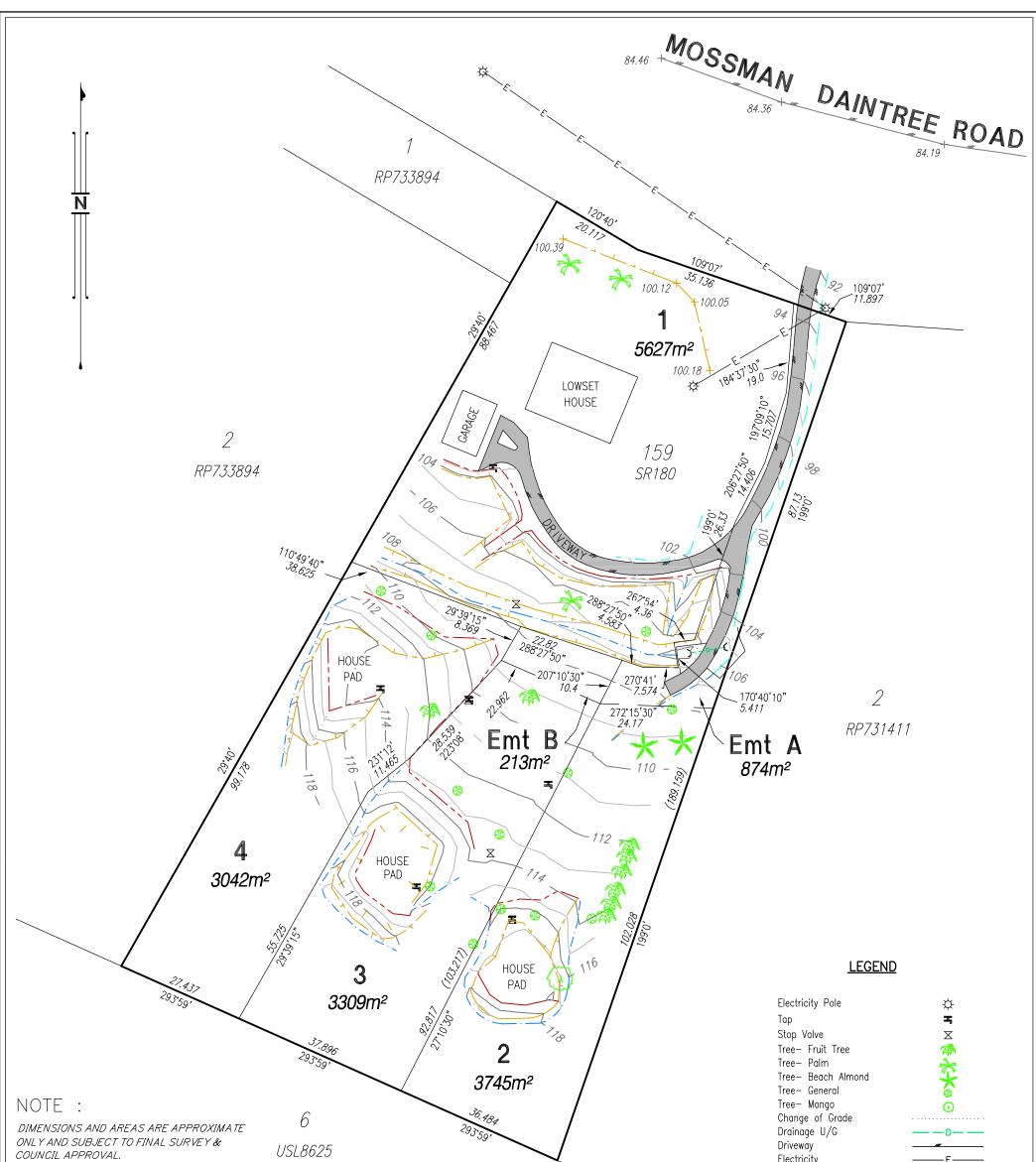
An Effluent Disposal Plan has been prepared and presented in Appendix C to ensure compliance.

If the land application system is not located in accordance with these plans, or if any alterations are made to the Architectural plans presented in Appendix D, or any other alterations occur that may alter the effectiveness of the management of land application system, then ACOR Consultants are to be notified so appropriate advice and suitable amendments to the design and documentation can be completed and resubmitted to the local authority if required.

8 **REFERENCES**

- AS/NZS 1547:2012 Onsite domestic-waste water management, Australia/New Zealand Standard.
- Queensland Plumbing and Wastewater Code, Queensland Government, 26 March 2019.
- MP4.1 Sustainable Buildings, Queensland Development Code, Queensland Government, 17 September 2020.
- FNQROC Development Manual, Design Manual D7, Version 11/19, Far North Queensland Regional Organisation of Council, November 2019

Appendix A – Site Survey Plan



COUNCIL APPROVAL.	USL8625		\checkmark	Electricity E
EASEMENTS A & B ARE FOR ACCESS AND SERVICES.				Toe of Bank Top of Bank Open Unlined Drain
CONTOURS SHOWN AT INTERVALS OF 1m. CONTOUR INDEXES SHOWN AT INTERVALS OF 2	Pm.			Open Lined Drain
ASSUMED DA TUM.				
AMENDMENT A: 03–03–2008 ADDED EDGE OF ROAD AND TOP OF BANK TO THE NORTHERN END OF THE 62493–1.				
0 8 16 24 32 40	PROJECT MANAGER/SURVEYOR A.SOLOMON	DESIGNED	L & M COCKRELL	
	CHECKED	SURVEYED AJS 18/09/07		CONICS 🖊
metres SCALE 1:800 IS APPLICABLE ONLY	drawn JMG 15/10/07	FIELD BK. LEVEL DATUM 1141 Assumed	Proposed Reconfiguration of Lot 159 on SR180	
TO THE ORIGINAL SHEET SIZE. (A3)	DRAFTING CHECKED	SHEET A3 SHEET 1 SIZE A3 SHEETS 1	Plan of Lots 1-4 & Easement A in Lot 2 & Easement B in Lot 3	
	CAD 62493-1.dwg	scale 1:800	AMENDED ISSUE DRAWING NO. 03-03-2008 A 62493-1	Te: (07) 4098 1148 Fs: (07) 4096 1814 mossmen@conics.com.au Conics (Caims) Pty Ltd A.C.N. 055 831 098

Appendix B – Geotechnical Report

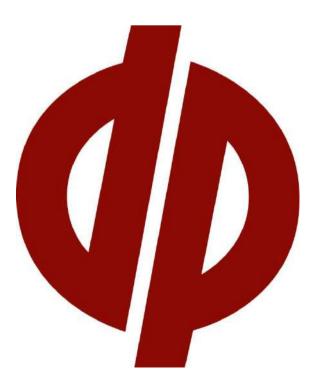


Report on Geotechnical Investigation

Proposed Residences Mossman Daintree Road, Rocky Point

> Prepared for Office Deicke

Project 214803.00 June 2022



Douglas Partners Geotechnics | Environment | Groundwater

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The undersigned, on behalf of Douglas Partners Pty Ltd, confirm that this document and all attached drawings, logs and test results have been checked and reviewed for errors, omissions and inaccuracies.

	Signature	Date
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Appendix A:	Notes About this Report
	Sampling Methods
	Soil Descriptions
	Symbols and Abbreviations
Appendix B:	Drawings
Appendix C:	Effluent Disposal Soil Assessment Summary
	Effluent Disposal Site Assessment Summary



Appendix D:	Field Work Results
Appendix E:	Laboratory test results



Report on Geotechnical Investigation Proposed Residences Mossman Daintree Road, Rocky Point

1. Introduction

This report presents the results of geotechnical investigation undertaken by Douglas Partners Pty Ltd (DP) for three proposed residential dwellings at Mossman Daintree Road, Rocky Point. The investigation was commissioned in an email dated 28 April 2022 by Mr John Deicke of Office Deicke and was undertaken in accordance with DP proposal 214803.00.P.001.Rev0 dated 23 April 2022.

It is understood that the project includes the construction of three residential dwellings on separate but adjoining allotments of the subdivided 1299 Mossman Daintree Road.

The aim of the investigation was to assess the subsurface soil and groundwater conditions at the field test locations to provide:

- site classification to AS 2870 (2011);
- excavation conditions and suitable temporary and permanent batter slopes;
- site preparation earthworks and suitability of site won materials for re-use as engineered fill;
- geotechnical retaining wall design parameters;
- footing options and footing design parameters; and
- allocation of soil categories and determination of design soil permeability and design effluent loading rates based on AS 1547 (2012).

Slope risk assessment was excluded from the requested scope of work.

The investigation included the excavation of ten test pits and laboratory testing of selected samples. The details of the field and laboratory work are presented in this report, together with comments and recommendations on the items listed above.

This report must be read in conjunction with the notes entitled 'About This Report' in Appendix A and other explanatory notes, and should be kept in its entirety without separation of individual pages or sections.

2. Proposed Development

DP was supplied with a preliminary site plan (Drawing SD_A01_0101 dated 4 February 2022) for the purposes of investigation planning which is included in Appendix B for ease of reference. This plan indicated the proposed development to comprise the construction of three, two-level residential dwellings on the existing building platforms. It is assumed the buildings will be constructed using lightweight materials, elevated above ground and supported by steel posts.



3. Site Description

The site (designated as 1299A to 1299C Mossman Daintree Road) comprises three adjoining allotments within a battle-axe area above an existing residence (refer Drawing 1 in Appendix B and Figure 1 below). Each lot is approximately rectangular in shape, and comprising an area of 3042 m^2 (Lots 3 and 4) or 4092 m^2 (Lot 2). For the purposes of this report, 'site' refers to the combined area of the three allotments.

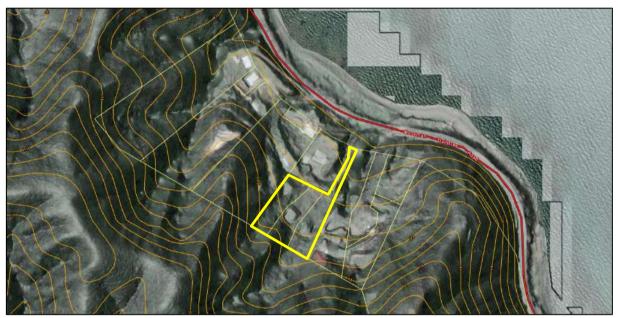


Figure 1: Qld Globe image showing site location in yellow outline.

The site is situated on the north-eastern facing foot slopes of the Dagmar Range and site elevations approximately range from 35 AHD to 55 AHD. Much of the southwestern portion of the site (approximately 25%) comprises steeply sloping hillside vegetated by dense rainforest.

Previous earthworks are evident on site including the preparation of building platforms using 'cut to fill' methodology, and an open unlined drain has been excavated along much of the site's eastern boundary, presumably to divert overland flows down to the road reserve. No structures were observed within site boundaries and other than that for the above earthworks, it appears that no previous site development has occurred.

The main portion of the site, below the south-western steep slopes, has mostly been cleared of native vegetation and is generally vegetated by short grass with several large trees and shrubs present mostly around the building platforms (refer Figures 2 to 5). The terrain in the cleared portions is generally uneven and hummocky, and the building platforms appear to have been created by localised cutting and filling. The natural slope gradients downhill of the of the building platforms generally range between 10° to 15°(ie 'moderately' sloping).

The site is bordered by residential properties downhill to the north, east and west, and steeply sloping, densely vegetated slopes uphill to the south.





Figure 2: Showing general site conditions. Photo is from the end of the shared driveway with a view towards the southwest.



Figure 3: View looking northeast showing the building platform at 1299A.



Figure 4: View looking northeast showing the building platform at 1299B.



Figure 5: View looking west showing the building platform at 1299C.



4. Geology

Reference to the Mossman 1:100,000 Geological Series Sheet (Department of Mines and Energy, 1999) indicates that the site is underlain by Hodgkinson Formation bedrock typically comprising siltstone and fine to coarse grained arenite, and residual soils thereof.

Investigation findings are generally consistent with the published findings however colluvial materials were commonly encountered, generally overlying residual soils in the majority of the test locations.

5. Field Work

5.1 Methods

The field work was completed on 4 May 2022 and comprised the excavation of ten test pits, designated as Pits 1 to 10, to depths ranging from 1.7 m to 3.9 m. Approximate test locations are shown on Drawing 1 in Appendix B.

The pits were excavated using a 5.5 tonne excavator. Strata identification was through observation of excavated spoil, and soil samples were taken at regular depth intervals for laboratory testing. Pocket penetrometer readings were taken in the walls of the pits and dynamic cone penetrometer (DCP) tests were undertaken alongside the pits to provide additional information for assessment of soil strength consistency. On completion of excavation, the pits were backfilled with excavated spoil in layers which were nominally compacted using the back of the excavator bucket. Any excess spoil was mounded and track rolled at the surface.

The field work was carried out in the presence of a DP engineering geologist.

A differential GPS unit was used to record UTM co-ordinates to GDA94. Surface levels were extrapolated from a supplied contour drawing. The co-ordinates and surface levels are presented on the test pit logs in Appendix D.

5.2 Results

The subsurface conditions encountered in the pits are detailed on the logs in Appendix D. Notes defining the sampling methods, soil descriptions, and symbols and abbreviations used in their preparation are given in Appendix A.

Subsurface conditions may be grouped into two areas for discussion, the building platform (ie modified) areas and downslope (ie natural) areas.

Fill, comprising medium dense clayey sandy gravel (similar to the naturally occurring colluvium encountered elsewhere on site) was encountered in Pits 2, 4 and 6 to depths of between 1.2 m and 2.2 m.

Natural materials encountered within the majority of the building platform test locations, either below the fill or from the surface (refer Pits 1, 2 and 4 to 6) are broadly characterised by colluvial clayey or silty



sandy gravel with cobbles (generally assessed to be medium dense or denser), underlain in Pits 5 and 6 by stiff or stronger residual clayey silt, transitioning to weathered siltstone within which virtual excavator refusal occurred on probable very low strength or stronger rock. Colluvium was not encountered in Pit 3, which encountered hard, residual clayey silt to 1.5 m depth, overlying siltstone to virtual excavator refusal at 1.7 m depth.

Within the downslope hillside test locations (refer Pits 7 to 10), ground conditions generally comprised a shallow (0.15 m or 0.2 m thick) layer of firm or stronger clayey silt topsoil overlying stiff or stronger clayey silt. The exception to this generalised profile description was the presence of very stiff silty clay encountered below 1.5 m depth in Pit 7, to the limit of investigation.

Free groundwater was not observed in any of the test pits, however it should be noted that groundwater depths and ground moisture conditions are affected by climatic conditions, soil permeability and human influences, and will therefore vary with time. Rocky Point is in the wet tropics and subject to wet and dry seasons, during which the groundwater levels may be prone to vary considerably.

Deeply eroded ephemeral water courses / surface drains were observed to the east and north of site indicating the likelihood of significant overland flows during and following significant rainfall events.

6. Laboratory Testing

Geotechnical Laboratory Testing 6.1

Geotechnical laboratory testing comprised:

- Atterberg limits, linear shrinkage, and field moisture content (three tests); and
- Particle size distribution, greater than 0.075 mm (two tests).

Geotechnical laboratory results are summarised in Tables 1 and 2, with laboratory certificates presented in Appendix E.

Pit	Depth (m)	Primary Description	FMC (%)	LL (%)	PL (%)	PI (%)	LS (%)
4	0.5	Clayey Sandy GRAVEL	18.1	28	23	5	3.5
5	1.0	Clayey SILT	18.0	34	23	11	4.5
8	0.3	Clayey SILT	14.7	24	19	5	2.5
Notes to				_ '	.0	•	2.0

Table 1: Results of Laboratory Testing – Field Moisture, Atterberg Limits and Linear Shrinkage

FMC - Field Moisture Content LL - Liquid Limit

PL - Plastic Limit

PI - Plasticity Index LS - Linear Shrinkage



Pit	Depth (m)	Primary Description	Gravel (%)	Sand (%)	Silt and clay (%)
1	0.4	Clayey Sandy GRAVEL	38	30	32
4	0.5	Clayey GRAVEL	52	19	29

Table 2: Results of Laboratory Testing - Gradings

6.2 Effluent Disposal Laboratory Testing

Effluent disposal laboratory testing comprised:

- Emerson class number, soil pH, electrical conductivity (EC), cation exchange capacity (three tests); and
- total nitrogen, sodium adsorption ratio, phosphorous absorption capacity (three tests).

Effluent disposal laboratory results are summarised in Tables 3 and 4, with laboratory certificates presented in Appendix E.

Pit	Depth (m)	Primary Description	Soil pH 1:5	EC (µS/cm)	Phosphorous Sorption Capacity (mgP/kg)	Total Nitrogen in Soil (mg/kg)	Sodium Adsorption Ratio	Emerson Class Number
7	1.7	Silty CLAY	5.9	48	670	170	1.2	5
9	1.5	Clayey SILT	5.2	51	560	180	1.8	6
10	1.0	Clayey SILT	5.4	45	530	270	0.56	5

Table 3: Results of pH, EC, Phosphorous Ssorption and Emerson Class Number Testing

Table 4: Results of Cation Exchange Capacity Testing

Pit	Depth (m)	Description	Calcium (meq%)	Potassium (meq%)	Magnesium (meq%)	Sodium (meq%)	CEC (meq%)	ESP (%)
7	1.7	Silty CLAY	0.2	<0.1	0.1	<0.1	<1	<1
9	1.5	Clayey SILT	0.1	<0.1	<0.1	<0.1	<1	<1
10	1.0	Clayey SILT	0.5	<0.1	0.2	<0.1	<1	<1

Legend:

CEC – cation exchange capacity

ESP - exchangeable sodium percentage



7. Comments

7.1 Appreciation of Site Geotechnical Conditions

The natural ground conditions at the field test locations are generally characterised as comprising medium dense or denser colluvial soils consisting of varying proportions of clayey silt, sand, gravel and cobbles, generally overlying residual stiff or stronger clayey silt transitioning with depth to weathered siltstone bedrock. Fill of similar properties to the colluvial soils was encountered to depths of between 1.2 m and 2.2 m within the north-western portions of each of the building platforms. It is assumed that the fill has been site-won from the uphill (south-western) sides of the building platform areas.

In isolation of upslope instability risk, the assessment of which was not requested by the client but is assumed to have been completed by others, the primary geohazard for this project is anticipated to be that of the existing uncontrolled fill, which should not be relied upon for structural support. The design of on-site effluent disposal systems should only be undertaken by appropriately experienced personnel familiar with the site and climatic conditions, giving particular consideration to hydraulic balance during the wet season.

Further comments on design and construction are given in the following sections of the report.

7.2 Site Classification

AS 2870 (2011) states that site classification is based on expected ground surface movement. Surface movement is usually due to soil reactivity under normal moisture conditions, however in some cases the level of ground movement may be controlled by other factors. Sites where ground movements may be significantly affected by factors other than reactive soil movements under normal moisture conditions are classified as 'Class P' under the standard and footing design must by based on engineering principles.

AS2870 (2011) provides a list of scenarios that would require a 'Class P' classification, including the presence of 'uncontrolled' fill, which applies to the existing building platforms in their current conditions.

7.3 Excavation Conditions

Excavations for high level footings and trenches for effluent disposal are anticipated to encounter either colluvial clayey/silty sandy gravel or residual clayey silt transitioning to very low strength weathered siltstone. It is considered that the colluvial and residual soils should be readily excavatable by standard small sized (ie 12 - 20 tonne) earthmoving equipment. Ripping would likely be required for excavations within low strength or stronger siltstone, depending on fracturing and strength variability.



7.4 Batter Slopes

Short-term temporary batter slopes of 1.5H:1V are suggested for unsurcharged, dry, temporary excavations in natural in-situ soils or existing fill up to 3 m deep. Permanent batter slope angles of no steeper than 3H:1V are suggested, with steeper slopes requiring additional geotechnical assessment, possibly requiring the construction of engineer designed retaining walls (refer Section 7.6).

Stockpiles or heavy plant should not be placed near the batter crests, as this may instigate slope failure. Further advice should be sought from DP if such surcharge is located within the batter vertical height laterally behind the crest.

Surface runoff should be diverted away from the crests and toes of the batters to reduce the potential for scour erosion. It is also recommended that all batters incorporate crest and toe drains and be appropriately vegetated with respect to erosion control.

7.5 Site Preparation

It is suggested that site preparation and fill for the support of ground slabs, pavements and upper level footings be carried out in accordance with the following guidelines::

- Remove all existing uncontrolled fill, surface vegetation, organic topsoil, and any deleterious soft, wet or highly compressible material;.
- Tine the subgrade and adjust the moisture content to within 2% of optimum moisture content for standard compaction (SOMC);
- Compact the subgrade with at least six passes of a minimum 12 t static weight smooth drum roller. Test roll the compacted subgrade under careful inspection by a geotechnical engineer to detect any remaining relatively soft or loose zones, which should be excavated out and replaced with approved engineered fill under 'Level 1' inspection and testing;
- If additional fill is required, place approved engineered fill in layers not exceeding 200 mm loose thickness, and compact to at least 98% standard dry density ratio or density index of at least 75%. Moisture contents within cohesive fill should be maintained within 2% of SOMC, during and after compaction; and
- Undertake 'Level 1' inspection and testing for all additional fill placement works, in accordance with AS 3798 (2007).

It is important to note that dry, over-compacted cohesive fill or cohesive subgrade (ie compacted to move than 102% SMDD and dry of OMC) is generally more prone to swelling and softening. For this reason, fill compaction and moisture should be carefully controlled on site, with both compaction and moisture control criteria included in the bulk earthworks specification.

Fill placement and compaction under 'Level 1' inspection and testing in accordance with AS 3798 (2007), is required where structural loads are to be supported by fill. A 'Level 1' inspection and test report must also be prepared at the completion of the works stating that the fill has been completed as recommended above and as required by AS 3798 (2007).



7.6 Retaining Walls

Table 5 presents lateral earth pressure, bulk density and sliding coefficient for the various soils encountered.

Material	Strength Consistency /Relative Density	Ka	Ко	Кр	Bulk Unit Weight (kN/m³)	tanδ⁵	
Clayey Silt / Silty Clay	Stiff	0.4	0.55	2.5	18	0.3	
	Very stiff or hard	0.4	0.55	2.5	20		
Granular	Medium dense (or denser)	0.28	0.45	3.5	20	0.4	

Notes: * for concrete cast directly onto the clean soil surface

For design of retaining walls, due allowance should be made for the following:

- Surcharge loadings (over and above the lateral earth pressure coefficients presented above) where the finished ground level above retaining walls is above horizontal and where additional loading is likely to be applied from existing or future upslope structures, or from traffic.
- Drainage material installed for the full height behind the wall, to a width of at least 0.3 m. The material must be free draining and granular and have a perforated or slotted drainage pipe at the heel of the wall to rapidly remove the water into the stormwater system. If drainage is not provided, the wall loading caused by flooding or inundation, must be considered. Such flooding may penetrate up to 0.9 m depth into cracks behind the wall and result in a hydrostatic load.

Care should be taken when placing soils behind retaining walls, to reduce the risk of damage associated with the use of heavy compaction plant and swelling. Compaction should not exceed 95% to 98% standard maximum dry density ratio and all filling should be placed within 2% of OMC.

It is recommended that factors of safety of 2 against overturning and sliding stability and 1.5 for global stability, be adopted in the design of all retaining walls.

For limit state design methods, the ultimate parameters provided above in Table 5 will need to be factored in accordance with (AS 4678, 2002). Guidance on the selection of material strength partial factors is provided in Section 5.2 of (AS 4678, 2002) and is dependent upon the nature and state of the natural in-situ soil.

7.7 Footings

High level pad and strip footings up to 1 m and 0.6 m wide, respectively, founding in 'controlled' fill, medium dense or denser colluvium or stiff or stronger clayey silt may be sized for a maximum allowable bearing pressure of 100 kPa.

Settlements of approximately 10 mm to 15 mm are estimated for properly designed and constructed pad or strip footings dimensioned and supported as above. Wider footings are possible but would be subject to specific settlement assessment.

7.8 Soil and Site Assessment

The suitability of the site to accept effluent is dependent on a number of factors, including:

- the subsurface profile (soil and groundwater) within the proposed application area;
- the type and output nutrient quality of the treatment system utilised;
- the physical characteristics such as topography, landform, distances to water bodies, etc; and
- prevailing climatic conditions.

The controlling soil for on-site effluent disposal by traditional systems is likely to be the residual clayey silt due to its low permeability and poor soil structure. Based on Table 5.1 of (AS 1547, 2012), for a weakly structured Category 4 soil an indicative permeability of between 0.12 and 0.5 m/day is suggested.

Site and soil characteristics observed during the field work are assigned either a minor, moderate or major limitation depending on the restrictions to the disposal area in accordance with (AS 1547, 2012). The moderate/major limitations for effluent disposal within the site are shown below. Soil and site assessment summaries are provided in Appendix C.

Cation Exchange Capacity (CEC)

The CEC results of <1 meq% fall into the major limitation category due to the inability of these soils to hold plant nutrients.

Soil pH

Soil pH test results of 5.2 and 5.4 fall into the moderate limitation category due to non-optimal plant growth conditions.

Slope Angle

Slope angle of between 10° and 20° (approximately 20% to 40% grade) is a major limiting factor for all disposal systems due to the risk of runoff.

Landform and Site Drainage

The site topography presents a moderate risk of groundwater pollution and transport of waste offsite.

Upslope seepage

The run on and upslope seepage potential is a moderate risk due to transport of wastewater off site.



7.9 Design Effluent Loading Rates

For Category 4 soil it is recommended that the effluent design loading rates and design irrigation rates (DLR/DIR) as presented below in Table 6 be adopted, based on (AS 1547, 2012).

Table 6: Effluent Design Loading / Irrigation Rates

Dispessel System	Effluent Design Loading / Irrigation Rates			
Disposal System	Primary Treated Effluent	Secondary Treated Effluent		
Conventional Trenches or Beds	6 to 10 mm/day	20 mm/day		
Evapotranspiration/absorption (ETA/ETS) system	8 mm/day			
Irrigation System – Drip or spray	3.5 mm/day ^A			
Irrigation System – Low pressure effluent distribution (LPED)	3 mm/day			
Mound System	8 mm/day			

Notes: A requires a minimum of 150 mm of high-quality topsoil

8. Limitations

Douglas Partners Pty Ltd (DP) has prepared this report for this project at 1299 Mossman Daintree Road in accordance with DP's proposal dated 23 April 2022 and acceptance received from John Deicke dated 28 April 2022. The work was carried out under DP's Conditions of Engagement. This report is provided for the exclusive use of Office Deicke for this project only and for the purposes as described in the report. It should not be used by or relied upon for other projects or purposes on the same or other site or by a third party. Any party so relying upon this report beyond its exclusive use and purpose as stated above, and without the express written consent of DP, does so entirely at its own risk and without recourse to DP for any loss or damage. In preparing this report DP has necessarily relied upon information provided by the client and/or their agents.

The results provided in the report are indicative of the sub-surface conditions on the site only at the specific sampling and/or testing locations, and then only to the depths investigated and at the time the work was carried out. Sub-surface conditions can change abruptly due to variable geological processes and also as a result of human influences. Such changes may occur after DP's field testing has been completed.

DP's advice is based upon the conditions encountered during this investigation. The accuracy of the advice provided by DP in this report may be affected by undetected variations in ground conditions across the site between and beyond the sampling and/or testing locations.



The assessment of atypical safety hazards arising from this advice is restricted to the geotechnical components set out in this report and based on known project conditions and stated design advice and assumptions. While some recommendations for safe controls may be provided, detailed 'safety in design' assessment is outside the current scope of this report and requires additional project data and assessment.

This report must be read in conjunction with all of the attached and should be kept in its entirety without separation of individual pages or sections. DP cannot be held responsible for interpretations or conclusions made by others unless they are supported by an expressed statement, interpretation, outcome or conclusion stated in this report.

This report, or sections from this report, should not be used as part of a specification for a project, without review and agreement by DP. This is because this report has been written as advice and opinion rather than instructions for construction.

The scope of work for this investigation/report did not include the assessment of surface or sub-surface materials or groundwater for contaminants, within or adjacent to the site. Should evidence of fill of unknown origin be noted in the report, and in particular the presence of building demolition materials, it should be recognised that there may be some risk that such fill may contain contaminants and hazardous building materials.

9. References

AS 1547. (2012). On-site domestic wastewater management. Standards Australia.

- AS 2870. (2011). Residential Slabs and Footings. Standards Australia.
- AS 3798. (2007). *Guidelines on Earthworks for Commercial and Residential Developments.* Standards Australia.

AS 4678. (2002). *Earth-retaining structures*. Standards Australia.

Department of Mines and Energy. (1999). Mossman. Brisbane, QLD: Queensland Government.

Douglas Partners Pty Ltd

Appendix A

About This Report



Introduction

These notes have been provided to amplify DP's report in regard to classification methods, field procedures and the comments section. Not all are necessarily relevant to all reports.

DP's reports are based on information gained from limited subsurface excavations and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

Copyright

This report is the property of Douglas Partners Pty Ltd. The report may only be used for the purpose for which it was commissioned and in accordance with the Conditions of Engagement for the commission supplied at the time of proposal. Unauthorised use of this report in any form whatsoever is prohibited.

Borehole and Test Pit Logs

The borehole and test pit logs presented in this report are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling or excavation. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable or possible to justify on economic grounds. In any case the boreholes and test pits represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes or pits, the frequency of sampling, and the possibility of other than 'straight line' variations between the test locations.

Groundwater

Where groundwater levels are measured in boreholes there are several potential problems, namely:

 In low permeability soils groundwater may enter the hole very slowly or perhaps not at all during the time the hole is left open;

- A localised, perched water table may lead to an erroneous indication of the true water table;
- Water table levels will vary from time to time with seasons or recent weather changes. They may not be the same at the time of construction as are indicated in the report; and
- The use of water or mud as a drilling fluid will mask any groundwater inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water measurements are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

Reports

The report has been prepared by qualified personnel, is based on the information obtained from field and laboratory testing, and has been undertaken to current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal, the information and interpretation may not be relevant if the design proposal is changed. If this happens, DP will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface conditions, discussion of geotechnical and environmental aspects, and recommendations or suggestions for design and construction. However, DP cannot always anticipate or assume responsibility for:

- Unexpected variations in ground conditions. The potential for this will depend partly on borehole or pit spacing and sampling frequency;
- Changes in policy or interpretations of policy by statutory authorities; or
- The actions of contractors responding to commercial pressures.

If these occur, DP will be pleased to assist with investigations or advice to resolve the matter.

About this Report

Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, DP requests that it be immediately notified. Most problems are much more readily resolved when conditions are exposed rather than at some later stage, well after the event.

Information for Contractual Purposes

Where information obtained from this report is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. DP would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

Site Inspection

The company will always be pleased to provide engineering inspection services for geotechnical and environmental aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

Sampling

Sampling is carried out during drilling or test pitting to allow engineering examination (and laboratory testing where required) of the soil or rock.

Disturbed samples taken during drilling provide information on colour, type, inclusions and, depending upon the degree of disturbance, some information on strength and structure.

Undisturbed samples are taken by pushing a thinwalled sample tube into the soil and withdrawing it to obtain a sample of the soil in a relatively undisturbed state. Such samples yield information on structure and strength, and are necessary for laboratory determination of shear strength and compressibility. Undisturbed sampling is generally effective only in cohesive soils.

Test Pits

Test pits are usually excavated with a backhoe or an excavator, allowing close examination of the insitu soil if it is safe to enter into the pit. The depth of excavation is limited to about 3 m for a backhoe and up to 6 m for a large excavator. A potential disadvantage of this investigation method is the larger area of disturbance to the site.

Large Diameter Augers

Boreholes can be drilled using a rotating plate or short spiral auger, generally 300 mm or larger in diameter commonly mounted on a standard piling rig. The cuttings are returned to the surface at intervals (generally not more than 0.5 m) and are disturbed but usually unchanged in moisture content. Identification of soil strata is generally much more reliable than with continuous spiral flight augers, and is usually supplemented by occasional undisturbed tube samples.

Continuous Spiral Flight Augers

The borehole is advanced using 90-115 mm diameter continuous spiral flight augers which are withdrawn at intervals to allow sampling or in-situ testing. This is a relatively economical means of drilling in clays and sands above the water table. Samples are returned to the surface, or may be collected after withdrawal of the auger flights, but they are disturbed and may be mixed with soils from the sides of the hole. Information from the drilling (as distinct from specific sampling by SPTs or undisturbed samples) is of relatively low reliability, due to the remoulding, possible mixing or softening of samples by groundwater.

Non-core Rotary Drilling

The borehole is advanced using a rotary bit, with water or drilling mud being pumped down the drill rods and returned up the annulus, carrying the drill cuttings. Only major changes in stratification can be determined from the cuttings, together with some information from the rate of penetration. Where drilling mud is used this can mask the cuttings and reliable identification is only possible from separate sampling such as SPTs.

Continuous Core Drilling

A continuous core sample can be obtained using a diamond tipped core barrel, usually with a 50 mm internal diameter. Provided full core recovery is achieved (which is not always possible in weak rocks and granular soils), this technique provides a very reliable method of investigation.

Standard Penetration Tests

Standard penetration tests (SPT) are used as a means of estimating the density or strength of soils and also of obtaining a relatively undisturbed sample. The test procedure is described in Australian Standard 1289, Methods of Testing Soils for Engineering Purposes - Test 6.3.1.

The test is carried out in a borehole by driving a 50 mm diameter split sample tube under the impact of a 63 kg hammer with a free fall of 760 mm. It is normal for the tube to be driven in three successive 150 mm increments and the 'N' value is taken as the number of blows for the last 300 mm. In dense sands, very hard clays or weak rock, the full 450 mm penetration may not be practicable and the test is discontinued.

The test results are reported in the following form.

 In the case where full penetration is obtained with successive blow counts for each 150 mm of, say, 4, 6 and 7 as:

4,6,7 N=13

In the case where the test is discontinued before the full penetration depth, say after 15 blows for the first 150 mm and 30 blows for the next 40 mm as:

15, 30/40 mm

Sampling Methods

The results of the SPT tests can be related empirically to the engineering properties of the soils.

Dynamic Cone Penetrometer Tests / Perth Sand Penetrometer Tests

Dynamic penetrometer tests (DCP or PSP) are carried out by driving a steel rod into the ground using a standard weight of hammer falling a specified distance. As the rod penetrates the soil the number of blows required to penetrate each successive 150 mm depth are recorded. Normally there is a depth limitation of 1.2 m, but this may be extended in certain conditions by the use of extension rods. Two types of penetrometer are commonly used.

- Perth sand penetrometer a 16 mm diameter flat ended rod is driven using a 9 kg hammer dropping 600 mm (AS 1289, Test 6.3.3). This test was developed for testing the density of sands and is mainly used in granular soils and filling.
- Cone penetrometer a 16 mm diameter rod with a 20 mm diameter cone end is driven using a 9 kg hammer dropping 510 mm (AS 1289, Test 6.3.2). This test was developed initially for pavement subgrade investigations, and correlations of the test results with California Bearing Ratio have been published by various road authorities.

Soil Descriptions

Description and Classification Methods

The methods of description and classification of soils and rocks used in this report are generally based on Australian Standard AS1726:2017, Geotechnical Site Investigations. In general, the descriptions include strength or density, colour, structure, soil or rock type and inclusions.

Soil Types

Soil types are described according to the predominant particle size, qualified by the grading of other particles present:

Туре	Particle size (mm)
Boulder	>200
Cobble	63 - 200
Gravel	2.36 - 63
Sand	0.075 - 2.36
Silt	0.002 - 0.075
Clay	<0.002

The sand and gravel sizes can be further subdivided as follows:

Туре	Particle size (mm)
Coarse gravel	19 - 63
Medium gravel	6.7 - 19
Fine gravel	2.36 - 6.7
Coarse sand	0.6 - 2.36
Medium sand	0.21 - 0.6
Fine sand	0.075 - 0.21

Definitions of grading terms used are:

- Well graded a good representation of all particle sizes
- Poorly graded an excess or deficiency of particular sizes within the specified range
- Uniformly graded an excess of a particular particle size
- Gap graded a deficiency of a particular particle size with the range

The proportions of secondary constituents of soils are described as follows:

In fine grained soils (>	>35% fines)
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Term	Proportion	Example			
	of sand or				
	gravel				
And	Specify	Clay (60%) and			
		Sand (40%)			
Adjective	>30%	Sandy Clay			
With	15 – 30%	Clay with sand			
Trace	0 - 15%	Clay with trace			
		sand			

In coarse grained soils (>65% coarse)

 with clays or silts 	6	
Term	Proportion of fines	Example
And	Specify	Sand (70%) and Clay (30%)
Adjective	>12%	Clayey Sand
With	5 - 12%	Sand with clay
Trace	0 - 5%	Sand with trace

clay

In coarse grained soils (>65% coarse)
- with coarser fraction

Term	Proportion of coarser fraction	Example
And	Specify	Sand (60%) and Gravel (40%)
Adjective	>30%	Gravelly Sand
With	15 - 30%	Sand with gravel
Trace	0 - 15%	Sand with trace gravel

The presence of cobbles and boulders shall be specifically noted by beginning the description with 'Mix of Soil and Cobbles/Boulders' with the word order indicating the dominant first and the proportion of cobbles and boulders described together.

Soil Descriptions

Cohesive Soils

Cohesive soils, such as clays, are classified on the basis of undrained shear strength. The strength may be measured by laboratory testing, or estimated by field tests or engineering examination. The strength terms are defined as follows:

Description	Abbreviation	Undrained shear strength (kPa)
Very soft	VS	<12
Soft	S	12 - 25
Firm	F	25 - 50
Stiff	St	50 - 100
Very stiff	VSt	100 - 200
Hard	Н	>200
Friable	Fr	-

Cohesionless Soils

Cohesionless soils, such as clean sands, are classified on the basis of relative density, generally from the results of standard penetration tests (SPT), cone penetration tests (CPT) or dynamic penetrometers (PSP). The relative density terms are given below:

Relative Density	Abbreviation	Density Index (%)
Very loose	VL	<15
Loose	L	15-35
Medium dense	MD	35-65
Dense	D	65-85
Very dense	VD	>85

Soil Origin

It is often difficult to accurately determine the origin of a soil. Soils can generally be classified as:

- Residual soil derived from in-situ weathering of the underlying rock;
- Extremely weathered material formed from in-situ weathering of geological formations. Has soil strength but retains the structure or fabric of the parent rock;
- Alluvial soil deposited by streams and rivers;

- Estuarine soil deposited in coastal estuaries;
- Marine soil deposited in a marine environment;
- Lacustrine soil deposited in freshwater lakes;
- Aeolian soil carried and deposited by wind;
- Colluvial soil soil and rock debris transported down slopes by gravity;
- Topsoil mantle of surface soil, often with high levels of organic material.
- Fill any material which has been moved by man.

Moisture Condition – Coarse Grained Soils For coarse grained soils the moisture condition

should be described by appearance and feel using the following terms:

- Dry (D) Non-cohesive and free-running.
- Moist (M) Soil feels cool, darkened in colour.

Soil tends to stick together. Sand forms weak ball but breaks easily.

Wet (W) Soil feels cool, darkened in colour.

Soil tends to stick together, free water forms when handling.

Moisture Condition – Fine Grained Soils

For fine grained soils the assessment of moisture content is relative to their plastic limit or liquid limit, as follows:

- 'Moist, dry of plastic limit' or 'w <PL' (i.e. hard and friable or powdery).
- 'Moist, near plastic limit' or 'w ≈ PL (i.e. soil can be moulded at moisture content approximately equal to the plastic limit).
- 'Moist, wet of plastic limit' or 'w >PL' (i.e. soils usually weakened and free water forms on the hands when handling).
- 'Wet' or 'w ≈LL' (i.e. near the liquid limit).
- 'Wet' or 'w >LL' (i.e. wet of the liquid limit).

Symbols & Abbreviations

Introduction

These notes summarise abbreviations commonly used on borehole logs and test pit reports.

Drilling or Excavation Methods

С	Core drilling
R	Rotary drilling
SFA	Spiral flight augers
NMLC	Diamond core - 52 mm dia
NQ	Diamond core - 47 mm dia
HQ	Diamond core - 63 mm dia
PQ	Diamond core - 81 mm dia

Water

\triangleright	Water seep
\bigtriangledown	Water level

Sampling and Testing

- A Auger sample
- B Bulk sample
- D Disturbed sample
- E Environmental sample
- U₅₀ Undisturbed tube sample (50mm)
- W Water sample
- pp Pocket penetrometer (kPa)
- PID Photo ionisation detector
- PL Point load strength Is(50) MPa
- S Standard Penetration Test
- V Shear vane (kPa)

Description of Defects in Rock

The abbreviated descriptions of the defects should be in the following order: Depth, Type, Orientation, Coating, Shape, Roughness and Other. Drilling and handling breaks are not usually included on the logs.

Defect Type

Bedding plane
Clay seam
Cleavage
Crushed zone
Decomposed seam
Fault
Joint
Lamination
Parting
Sheared Zone
Vein

Orientation

The inclination of defects is always measured from the perpendicular to the core axis.

- h horizontal
- v vertical
- sh sub-horizontal

ar

sv sub-vertical

Coating or Infilling Term

clean
coating
healed
infilled
stained
tight
veneer

Coating Descriptor

ca	calcite
cbs	carbonaceous
cly	clay
fe	iron oxide
mn	manganese
slt	silty

Shape

cu	curved
ir	irregular
pl	planar
st	stepped
un	undulating

Roughness

ро	polished
ro	rough
sl	slickensided
sm	smooth
vr	very rough

Other

fg	fragmented
bnd	band
qtz	quartz

Symbols & Abbreviations

Graphic Symbols for Soil and Rock

General

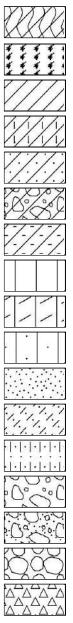
D. D. D. Z	
1. 4. 4. 4. XXXX	
\times	

Asphalt Road base

Concrete

Filling

Soils



Topsoil

.

Peat

Clay

Silty clay

Sandy clay

Gravelly clay

Shaly clay

Silt

Clayey silt

Sandy silt

Sand

Clayey sand

Silty sand

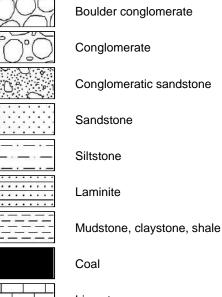
Gravel

Sandy gravel

Cobbles, boulders

Talus

Sedimentary Rocks



Limestone

Metamorphic Rocks

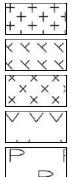
 $\begin{array}{c} & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\$

Slate, phyllite, schist

Quartzite

Gneiss

Igneous Rocks



Granite

Dolerite, basalt, andesite

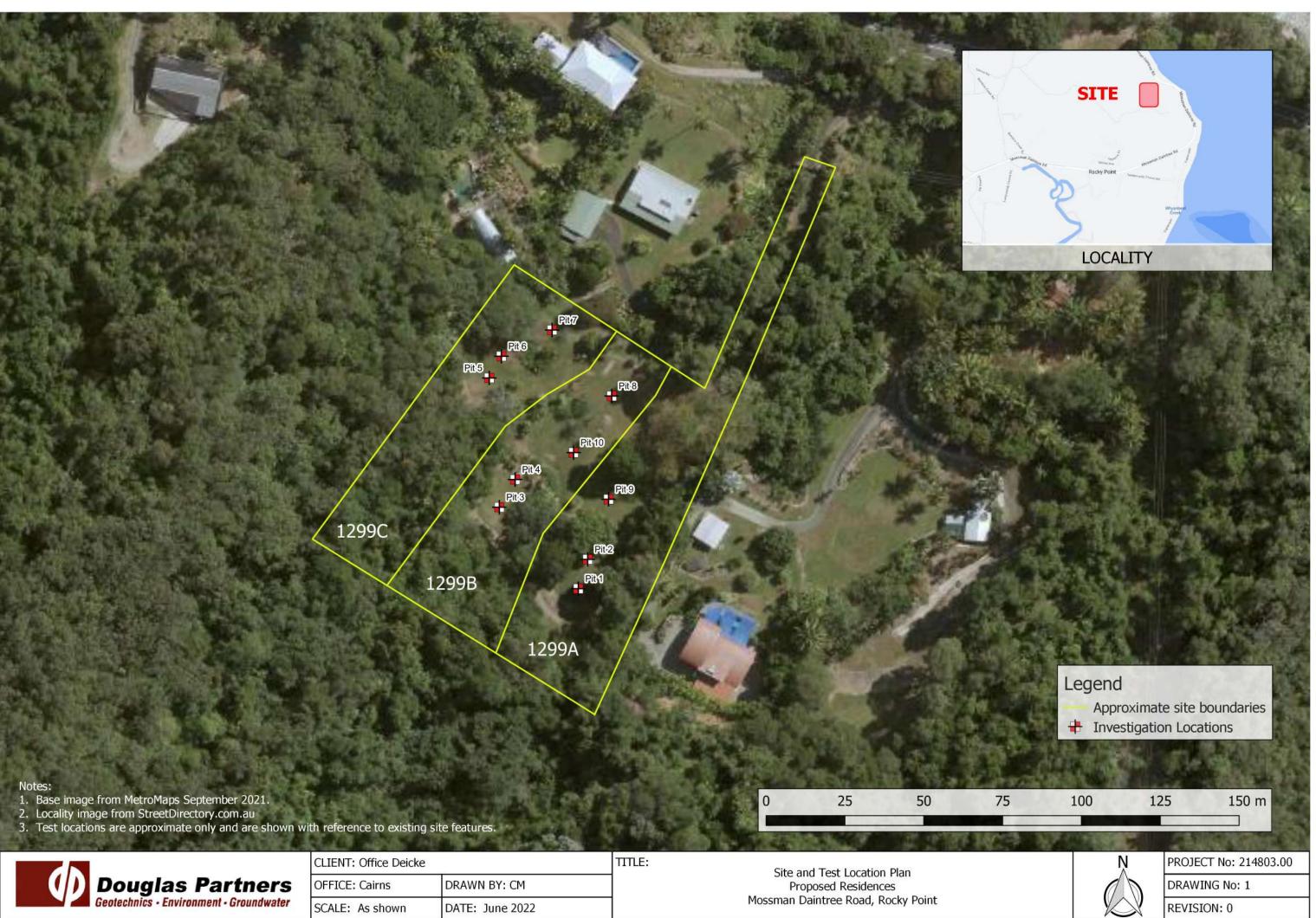
Dacite, epidote

Tuff, breccia

Porphyry

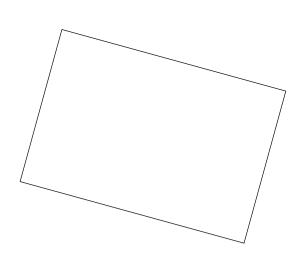
Appendix B

Drawings





	CLIENT: Office Deicke		TITLE: Site and Test Location Plan
;	OFFICE: Cairns	DRAWN BY: CM	Proposed Residences
	SCALE: As shown	DATE: June 2022	Mossman Daintree Road, Rocky Point



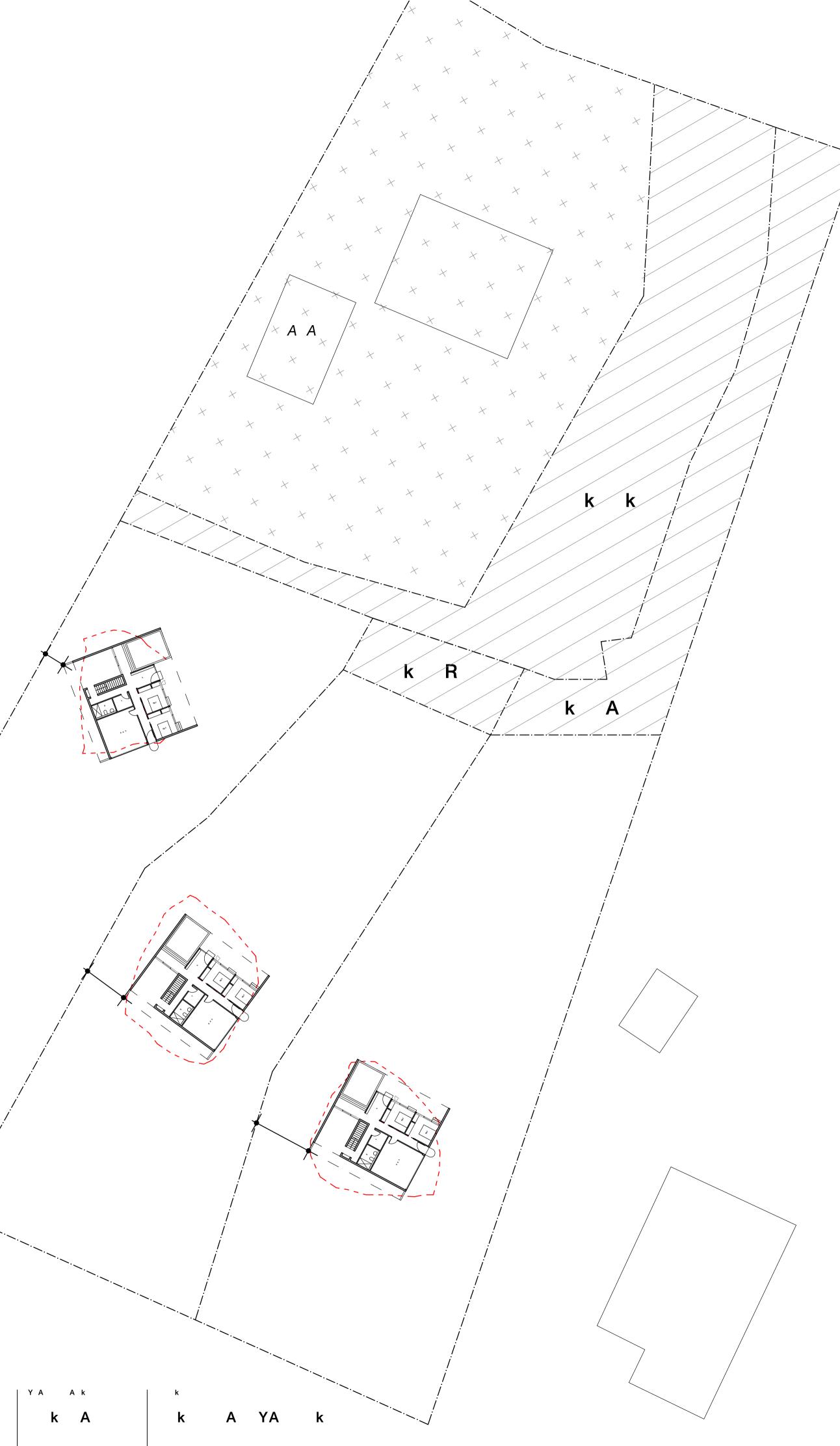


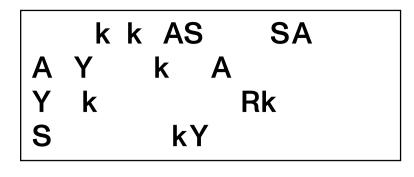
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Appendix C

Table C1 - Effluent Disposal Soil Assessment Summary Table C2 - Effluent Disposal Site Assessment Summary

Table C1 - Soil Assessment Summary

Soil Feature	Relevant System(s)	Minor Limitation	Moderate Limitation	Major Limitation	Restrictive Feature	
Depth to bedrock or hardpan (m)	Surface and sub-surface irrigation	>1.0	0.5 – 1.0	<0.5	Restricts plant growth (trees), excessive runoff, waterlogging	
	Absorption system	>1.5	1.0 – 1.5	<1.0	Groundwater pollution hazard. Resurfacing hazard	
Depth to high episodic or seasonal water table (m)*	Surface and sub-surface irrigation	>1.0	0.5 – 1.0	<0.5	Groundwater pollution hazard. Resurfacing hazard	
	Absorption system	>1.5	1.0 – 1.5	<1.0	Potential for groundwater pollution	
Soil permeability Category	Surface and sub-surface irrigation	2b, 3 and 4	2a, 5	1 and 6	Excessive run-off, waterlogging,	
	Absorption system	3 and 4		1,2,5 and 6	percolation	
Coarse fragments (%)	All land application systems	0-20	20-40	>40	May restrict plant growth, affect trench installation	
рН	All land application systems	>6.0	4.5 - 6.0	<4.5	Reduces optimum plant growth	
Electrical conductivity (dS/m)	All land application systems	<4	4-8	>8	Excessive salt may restrict plant growth	
Sodicity (exchangeable sodium percentage) [#]	Surface and sub-surface irrigation (0-0.4m)	0-5	5-10	>10	Potential for structural degradation	
	Absorption system (0-1.2m)		0.10			
Cation exchange capacity (cmol+/kg) (0-40cm)	Surface and sub-surface irrigation	>15	5-15	<5	Unable to hold plant nutrients	
Phosphorous sorption (kg P/ha) (0-1m for irrigation) (1 m below intended base of trench)	All land application systems	>6000	2000-6000	<2000	Unable to immobilise any excess P	

Bold text indicates applicable limitation

* water table taken as 2.0 m below ground level

Adapted from NSW EPA, "Environmental and Health Protection Guidelines, on-site Sewage Management for Single Households", January 1998

Site Feature	Relevant System(s)	Minor Limitation	Moderate Limitation	Major Limitation	Restrictive Feature
	All land application systems	Rare, above 1 in 20 year flood contour		Frequent, below 1 in 20 year flood contour	Transport of wastewater off-site
Flood potential	All treatment systems	Vents, openings, and electrical components above 1 in 100 year flood contour		Vents, openings, and electrical components below 1 in 100 year flood contour	Transport of wastewater off-site. System failure and electrocution hazard
Exposure	All land application systems	High sun and wind exposure		Low sun and wind exposure	Poor evapotranspiration
	Surface irrigation	0-6	6-12	>12	
Slope %	Sub-surface irrigation	0-10	10-20	>20	Run-off, erosion
	Absorption system	0-10	10-20	>20	
Landform	All systems	Hill crests, convex side slopes and plains	Concave side slopes and foot slopes	Drainage plains and incised channels	Groundwater pollution and resurfacing hazard
Run-on and upslope seepage	All land application systems	None – Iow	Moderate	High – diversion not practical	Transport of wastewater off-site
Erosion potential	All land application systems	No signs of erosion potential present		Signs of erosion, eg rills, mass movement and slope failure present	Soil degradation and transport, system failure
Site drainage	All land application systems	No visible signs of surface dampness		Visible signs of surface dampness, such as moisture-tolerant vegetation, and seepages, soaks and springs	Groundwater pollution hazard. Resurfacing hazard
Fill	All systems	No fill	Fill present		Subsidence. Variable permeability
Land area	All systems	Area is available	Area is limited	Area is not available	Health and pollution risks
Rocks and rock outcrops (% of land surface containing boulders)	All land application systems	<10%	10-20%	>20%	Limits system performance
Geology/ Regolith	All land application systems			Major geological discontinuities, fractured or highly porous regolith	Groundwater pollution hazard

Table C2 - Site Assessment Summary

Bold text indicates applicable limitations

Adapted from NSW EPA, "Environmental and Health Protection Guidelines, on-site Sewage Management for Single Households", January 1998

Appendix D

Field Work Results

CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 38.8 m AHD
 PIT No:
 1

 EASTING:
 330539
 PROJECT

 NORTHING:
 8187979
 DATE:
 4/5

PIT No: 1 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

_	Description	jc		Sam		& In Situ Testing	<u> </u>	
고 Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
98 - 1 - 1 - 2 - 3 - 3	Silty Sandy GRAVEL GM: pale brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, loose, colluvial - medium dense below 0.4 m depth		D	0.4 0.6 0.8 1.0		pp = 170 pp = 200 pp = 180 pp = 170		
32	- boulder at 3.6 m depth							
- 3.9	Pit discontinued at 3.9m depth - limit of excavator reach	υ _α υ						

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Pho

 P
 Piston sample
 PL(A) Poir

 U
 Tube sample (x mm dia.)
 PL(D) Poir

 W
 Water sample
 PL

 Water sample
 Star

 mple
 ¥
 Water level

PID	Photo ionisation detector (ppm)
PL(A)	Point load axial test Is(50) (MPa)
PL(D)	Point load diametral test Is(50) (MPa)
pp	Pocket penetrometer (kPa)
pp S	Standard penetration test
V	Shear vane (kPa)

LOGGED: McDonald

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 39.6 m AHD
 PIT No:
 2

 EASTING:
 330542
 PROJECT

 NORTHING:
 8187989
 DATE:
 4/5

PIT No: 2 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

			Description	ic		Sam		& In Situ Testing	-	Dumon	Demeters	
RL	Dep (m		of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20		
	- - - - - - - - -		FILL / Silty Sandy GRAVEL: dark brown and pale brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, trace rootlet, moist, medium dense		D	0.3 0.5 0.6 0.9		pp = 150 pp = 140 pp = 100				
	- - - - - - - - -	1.2-	Silty Sandy GRAVEL: pale brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense, colluvial									
	-		- very dense below 2.2 m		D	2.5						
	-3	3.0	Pit discontinued at 3.0m depth	<u>11 - X</u>						-3		





LOGGED: McDonald

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample

 P
 Piston sample

 U,
 Tube sample (x mm dia.)

 W
 Water sample (x mm dia.)

 W
 Water sample (x mm dia.)

 W
 Vater sample (x mm dia.)

 W
 Water sample (x mm dia.)

 W
 Water sample (x mm dia.)

 W
 Water sample (x mm dia.)

 V
 Vater sample (x mm dia.)

 V
 Water sample (x mm dia.)

 V
 Water sample (x mm dia.)

 V
 Standard penetration test

 Mater level
 V

 Shear vane (kPa)

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 38.8 m AHD
 PIT No:
 3

 EASTING:
 330514
 PROJECT

 NORTHING:
 8188005
 DATE:
 4/5

PIT No: 3 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

	Description	Graphic Log		Sam		& In Situ Testing	Water	Dynamic Penetrometer Test		
Uepth (m)	of Strata		Type	Depth	Sample	Results & Comments		(blows per 100mm) 5 10 15 20		
	Clayey SILT ML / SILTSTONE HW: Approximately 50% pale brown, low plasticity, w <pl, (extremely<br="" hard="">weathered Argillite) and 50% pale brown, very low strength, highly weathered siltstone</pl,>		D	0.2 0.3 0.4 0.6 0.8		рр >600 рр >600 рр >600 рр >600				
- 1.	Hodgkinsons Formation Pit discontinued at 1.7m depth - refusal on very low strength siltstone	<u>. </u>	_							





LOGGED: McDonald

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Augers B Bulksa BLK Blocks

Cor

SAN	SAMPLING & IN SITU TESTING LEGEND									
sample	G	Gas sample	PID	Photo ionisation detector (ppm)						
ample	Р	Piston sample		Point load axial test Is(50) (MPa)	I					
sample	U,	Tube sample (x mm dia.)	PL(D)	Point load diametral test Is(50) (MPa)	I					
Irilling	Ŵ	Water sample	pp	Pocket penetrometer (kPa)	I					
ped sample	⊳	Water seep	S	Standard penetration test	I					
nmental sample	Ŧ	Water level	V	Shear vane (kPa)						

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT: Office Deicke **PROJECT:** Proposed Residences LOCATION: Mossman Daintree Road, Rocky Point SURFACE LEVEL: 38.6 m AHD PIT No: 4 EASTING: 330519 **NORTHING:** 8188014

PROJECT No: 214803.00 **DATE:** 4/5/2022 SHEET 1 OF 1

			Description	<u>.</u>		Sam	pling &	& In Situ Testing	_	
RL	Dej (n		of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
37	1		FILL / Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense		D	0.2 0.4 0.5 0.6 0.8		pp = 180 pp = 150 pp = 100 pp = 90		
35 36		2.2 -	Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense, colluvial		D	3.0				





RIG: Kubota 6 tonne with 450 mm bladed bucket

₽

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

SAMPLING & IN SITU TESTING LEGEND Gas sample Piston sample Tube sample (x mm dia.) Water sample Water seep Water level G P U, W

LEGEND PID Photo ionisation detector (ppm) PL(A) Point load axial test Is(50) (MPa) PL(D) Point load diametral test Is(50) (MPa) pp Pocket penetrometer (kPa) S Standard penetration test V Shear vane (kPa)

LOGGED: McDonald

□ Sand Penetrometer AS1289.6.3.3

SURVEY DATUM: GDA94 Zone 55K

☑ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 36.6 m AHD
 PIT No:
 5

 EASTING:
 330511
 PROJECT

 NORTHING:
 8188046
 DATE:
 4/5

PIT No: 5 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

\prod	_	Description	jc		Sam		& In Situ Testing	-	Durania Danatan tan Tart
RL	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
35	- 0.5	Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense, colluvial Clayey SILT ML: brown, low plasticity, trace sand and gravel, w <pl, extremely="" hard,="" hodgkinsons<br="" weathered="">Formation SILTSTONE HW: brown and grey, very low strength</pl,>		D	0.4 0.5 0.6 0.9 1.0		рр >600 pp >600 pp >600		
	-2 2.0-	Pit discontinued at 2.0m depth - refusal on very low strength siltstone							





RIG: Kubota 6 tonne with 450 mm bladed bucket

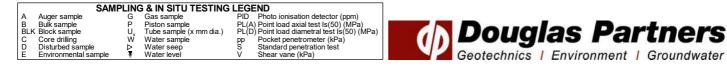
LOGGED: McDonald

SURVEY DATUM: GDA94 Zone 55K

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Dynamic penetrometer test completed at ground surface, and reattempted at 1.1 m depth

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

 SURFACE LEVEL:
 36.4 m AHD
 PIT No:
 6

 EASTING:
 330515
 PROJECT

 NORTHING:
 8188054
 DATE:
 4/5

PIT No: 6 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

Γ	_		Description	jc		Sam		& In Situ Testing	-	
RL	D (epth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20
	-1	2.0 -	FILL / Silty Sandy GRAVEL: brown, fine to coarse subangular gravel and sand, low plasticity silt with subangular cobble, moist, medium dense		D	0.5		pp range from 100-200 kPa in top 1.0 m depth		
34	- 3	3.2 -	gravel, trace cobble, w <pl, residual<br="" stiff,="" very="">Pit discontinued at 3.2m depth - refusal on very low strength siltstone</pl,>		D	3.0				





LOGGED: McDonald

RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Photo ionisation detector (ppm)

 P
 Piston sample (x mm dia.)
 PL(A) Point load axial test Is(50) (MPa)

 U
 Tube sample (x mm dia.)
 PL(D) Point load diametral test Is(50) (MPa)

 W
 Water sample
 PL

 W
 Water sample
 Standard penetration test

 mple
 ¥
 Water level

SURVEY DATUM: GDA94 Zone 55K

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 31 m AHD **EASTING:** 3305314 **NORTHING:** 8188062 PIT No: 7 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

		Description	<u>ic</u>	୍ର Sampling & In Situ Testing			& In Situ Testing	_			
RL	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20		
	- 1 1 	Topsoil / Clayey SILT ML: dark brown, low plasticity, with organics (rootlets), trace sand and gravel, w~PL, firm Clayey SILT ML: brown, low plasticity, with subrounded and subangular sand and gravel, w <pl, colluvial<br="" stiff,="">Silty CLAY CI: red brown, medium plasticity, trace sand and gravel, very stiff, possibly residual</pl,>		D	0.1 0.3 0.4 0.5 0.7 0.9		pp = 100 pp = 180 pp = 220 pp = 190 pp = 200				
29-	-2 2.0	Pit discontinued at 2.0m depth							2		





RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Phol

 P
 Piston sample
 PL(A) Poin
 PL(A) Poin

 U
 Tube sample (x mm dia.)
 PL(D) Poin
 PL(D) Poin

 W Water sample
 Y Water sample
 S Star

 Imple
 ¥
 Water level
 V

	Photo ionisation detector (ppm)
PL(A)	Point load axial test Is(50) (MPa)
PL(D)	Point load axial test Is(50) (MPa) Point load diametral test Is(50) (MPa)
	Pocket penetrometer (kPa)
pp S	Standard penetration test
V	Shear vane (kPa)

LOGGED: McDonald

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



Office Deicke CLIENT: PROJECT: Proposed Residences LOCATION: Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 32 m AHD **EASTING:** 330550 **NORTHING:** 8188041

PIT No: 8 PROJECT No: 214803.00 **DATE:** 4/5/2022 SHEET 1 OF 1

			Description	.cj		Sampling & In Situ Testing		& In Situ Testing	_		
2 RL	Dep (m		of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20	
	-	0.2	Topsoil / Clayey SILT ML: dark brown, low plasticity, with organics (rootlets), trace sand and gravel, w~PL, stiff		D	0.3		pp = 180			
	-		Clayey SILT ML: brown, low plasticity, with subangular and subrounded sand and gravel, trace cobble, w~PL, stiff, colluvial		U	0.3		pp = 180			
			- becoming w>PL below 0.4 m depth								
31	- 1			//// /////	D	0.9 1.0		pp = 190			
-											
-			- very stiff grading to hard below 1.4 m depth	1111							
				//// /////	D	1.8					
30	-2 - -									-2	
				////	D	2.5					
-					5	2.0					
29	-3	3.0	Dit discontinued at 2.0m donth							3	
			Pit discontinued at 3.0m depth								





RIG: Kubota 6 tonne with 450 mm bladed bucket

₽

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

SAMPLING & IN SITU TESTING LEGEND Gas sample Piston sample Tube sample (x mm dia.) Water sample Water seep Water level G P U, W

 PID
 Photo ionisation detector (ppm)

 PL(A) Point load axial test Is(50) (MPa)

 PL(D) Point load diametral test Is(50) (MPa)

 pp
 Pocket penetrometer (kPa)

 S
 Standard penetration test

 V
 Shear vane (kPa)

LOGGED: McDonald

SURVEY DATUM: GDA94 Zone 55K

□ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 35 m AHD EASTING: 330549 NORTHING: 8188008 PIT No: 9 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

		Description	.cj		Sampling & In Situ Testing		& In Situ Testing	5		
	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 100mm) 5 10 15 20	
, - -	0.2-	Topsoil / Clayey SILT ML: dark brown, low plasticity, with organics (rootlets), trace sand and gravel, w>PL, firm to stiff			0.3		pp = 220			
F		Clayey SILT ML: brown, low plasticity, trace subrounded and subangular sand and gravel, trace cobble, w>PL, stiff, colluvial		D	0.5 0.6		pp = 190			
	1	- with subrounded to subangular sand and gravel and subangular to subrounded cobble, below 1.1 m depth		D	0.9 1.0		pp = 240			
	2	- hard below 1.9 m depth							-2	
	3								-3	
-	3.9 -	Pit discontinued at 3.9m depth - limit of excavator reach								





RIG: Kubota 6 tonne with 450 mm bladed bucket

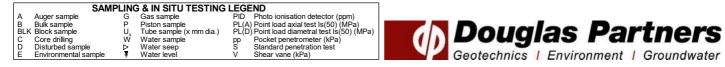
LOGGED: McDonald

SURVEY DATUM: GDA94 Zone 55K

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Dynamic penetrometer test completed at ground surface, and reattempted at 1.0 m depth

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



CLIENT:Office DeickePROJECT:Proposed ResidencesLOCATION:Mossman Daintree Road, Rocky Point

SURFACE LEVEL: 34 m AHD EASTING: 330538 NORTHING: 8188023 PIT No: 10 PROJECT No: 214803.00 DATE: 4/5/2022 SHEET 1 OF 1

enetrometer Test per 100mm) 15 20
1





RIG: Kubota 6 tonne with 450 mm bladed bucket

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

A Auger sample B Bulk sample BLK Block sample C Core drilling D Disturbed sample E Environmental sample

 SAMPLING & IN SITU TESTING LEGEND

 G
 Gas sample
 PID
 Phol

 P
 Piston sample
 PL(A) Poin
 PL(A) Poin

 U
 Tube sample (x mm dia.)
 PL(D) Poin
 PL(D) Poin

 W Water sample
 Y Water sample
 S Star

 Imple
 ¥
 Water level
 V

LLGL	
PID	Photo ionisation detector (ppm)
PL(A)	Point load axial test Is(50) (MPa)
PL(D)	Point load diametral test Is(50) (MPa)
pp	Pocket penetrometer (kPa)
pp S	Standard penetration test
V	Shear vane (kPa)

LOGGED: McDonald

□ Sand Penetrometer AS1289.6.3.3 ⊠ Cone Penetrometer AS1289.6.3.2



Appendix E

Laboratory Test Results

Material Test Report

Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777A
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 16/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 1 , Depth: 0.4
Material:	Soil & Cobble

Particle Size Distribution (AS1289 3.6.1)

Sieve	Passed %	Passing Limits
37.5 mm	100	
26.5 mm	94	
19 mm	84	
13.2 mm	80	
9.5 mm	73	
6.7 mm	69	
4.75 mm	66	
2.36 mm	62	
1.18 mm	57	
0.6 mm	52	
0.425 mm	49	
0.3 mm	45	
0.15 mm	38	
0.075 mm	32	
Moisture Content (AS 1	289 2 1 1)	
Moisture Content (%)	205 2.1.1	14.1

Douglas Partners Geotechnics | Environment | Groundwater

Douglas Partners Pty Ltd Townsville Laboratory 29 Civil Road Garbutt QLD 4814 Phone: (07) 4779 9866

Email: Townsville@douglaspartners.com.au



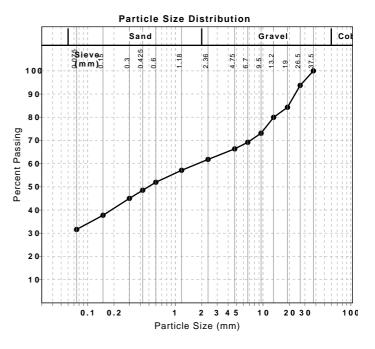
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Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828



Material Test Report

Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777B
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 16/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 4 , Depth: 0.5
Material:	Soil & Cobble

Do rticle Size Distribution (AS1289 3.6.1)

	· · · · · · · · · · · · · · · · · · ·	D 1 1 1 1
Sieve	Passed %	Passing Limits
53 mm	100	
37.5 mm	84	
26.5 mm	70	
19 mm	68	
13.2 mm	62	
9.5 mm	58	
6.7 mm	55	
4.75 mm	52	
2.36 mm	48	
1.18 mm	45	
0.6 mm	42	
0.425 mm	41	
0.3 mm	39	
0.15 mm	35	
0.075 mm	29	

Atterberg Limit (AS1289 3.1.2 & 3.2.1 & 3.3.1)			Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	28		
Plastic Limit (%)	23		
Plasticity Index (%)	5		
Linear Shrinkage (AS1289 3.4.1)		Min	Max
Linear Shrinkage (AS1289 3.4.1) Moisture Condition Determined By	AS 1289.3.1.2	Min	Max
Ŭ (AS 1289.3.1.2 3.5	Min	Max
Moisture Condition Determined By			Max
Moisture Condition Determined By Linear Shrinkage (%)	3.5		Max

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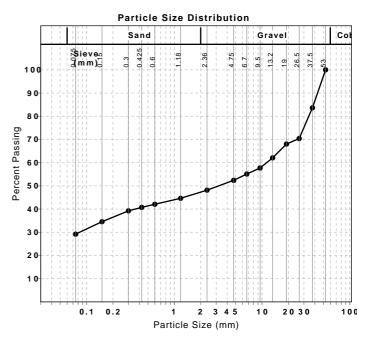
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Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828



Material Test Report

Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777C
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 16/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 5, Depth: 1.0
Material:	Clayey Silt

Atterberg Limit (AS1289 3.1.2 & 3.2.1 & 3.3.1)			Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	34		
Plastic Limit (%)	23		
Plasticity Index (%)	11		
Linear Shrinkage (AS1289 3.4.1)	Min	Max	
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	4.5		
Cracking Crumbling Curling	Crackin	g	
Cracking Crumbling Curling Moisture Content (AS 1289 2.1.1)	Crackin	g	

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Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828

Material Test Report

Report Number:	214803.00-1
Issue Number:	1
Date Issued:	17/05/2022
Client:	Office Deicke
	71 Gilgandra Street, Indooroopilly QLD 4006
Contact:	John Deicke
Project Number:	214803.00
Project Name:	Proposed Residences
Project Location:	Mossman Daintree Road, Rocky Point QLD
Work Request:	2777
Sample Number:	TW-2777D
Date Sampled:	05/05/2022
Dates Tested:	09/05/2022 - 13/05/2022
Sampling Method:	Sampled by Engineering Department
	The results apply to the sample as received
Sample Location:	Pit 8 , Depth: 0.3
Material:	Clayey Silt

Atterberg Limit (AS1289 3.1.2 & 3.2	Min	Max		
Sample History Oven Dried				
Preparation Method	Dry Sieve			
Liquid Limit (%)	24			
Plastic Limit (%)	19			
Plasticity Index (%) 5				
Linear Shrinkage (AS1289 3.4.1)	Min	Max		
Moisture Condition Determined By	AS 1289.3.1.2			
Linear Shrinkage (%)	2.5			
Cracking Crumbling Curling	g			
Moisture Content (AS 1289 2.1.1)				
Moisture Content (AS 1289 2.1.1)				

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Approved Signatory: Tanya Dilley Laboratory Manager Laboratory Accreditation Number: 828



Envirolab Services Pty Ltd ABN 37 112 535 645 12 Ashley St Chatswood NSW 2067 ph 02 9910 6200 fax 02 9910 6201 customerservice@envirolab.com.au www.envirolab.com.au

CERTIFICATE OF ANALYSIS 294891

Client Details	
Client	Douglas Partners (Cairns) Pty Ltd
Attention	Aidan McDonald
Address	13 Industrial Ave, Stratford, QLD, 4870

Sample Details	
Your Reference	214803.00 Rocky Point, Proposed Residences
Number of Samples	3 Soil
Date samples received	06/05/2022
Date completed instructions received	06/05/2022

Analysis Details

Please refer to the following pages for results, methodology summary and quality control data.

Samples were analysed as received from the client. Results relate specifically to the samples as received.

Results are reported on a dry weight basis for solids and on an as received basis for other matrices.

Please refer to the last page of this report for any comments relating to the results.

Report Details			
Date results requested by	16/05/2022		
Date of Issue	16/05/2022		
NATA Accreditation Number 2901. This document shall not be reproduced except in full.			
Accredited for compliance with	SO/IEC 17025 - Testing. Tests not covered by NATA are denoted with *		

Results Approved By Diego Bigolin, Inorganics Supervisor Giovanni Agosti, Group Technical Manager Authorised By

Nancy Zhang, Laboratory Manager

Envirolab Reference: 294891 Revision No: R00



Page | 1 of 9

Misc Inorg - Soil				
Our Reference		294891-1	294891-2	294891-3
Your Reference	UNITS	Pit 7	Pit 9	Pit 10
Depth		1.7	1.5	1
Date Sampled		04/05/2022	04/05/2022	04/05/2022
Type of sample		Soil	Soil	Soil
Date prepared	-	12/05/2022	12/05/2022	12/05/2022
Date analysed	-	12/05/2022	12/05/2022	12/05/2022
pH 1:5 soil:water	pH Units	5.2	5.2	5.4
Electrical Conductivity 1:5 soil:water	µS/cm	48	51	45
Total Nitrogen in Soil	mg/kg	170	180	270
Sodium Adsorption Ratio		1.2	1.8	0.56
Emerson Class No.	-	5.0	6.0	5.0
Phosphorus Sorption Capacity	mg/kg	670	560	530

ESP/CEC				
Our Reference		294891-1	294891-2	294891-3
Your Reference	UNITS	Pit 7	Pit 9	Pit 10
Depth		1.7	1.5	1
Date Sampled		04/05/2022	04/05/2022	04/05/2022
Type of sample		Soil	Soil	Soil
Date prepared	-	12/05/2022	12/05/2022	12/05/2022
Date analysed	-	13/05/2022	13/05/2022	13/05/2022
Exchangeable Ca	meq/100g	0.2	0.1	0.5
Exchangeable K	meq/100g	<0.1	<0.1	<0.1
Exchangeable Mg	meq/100g	0.1	<0.1	0.2
Exchangeable Na	meq/100g	<0.1	<0.1	<0.1
Cation Exchange Capacity	meq/100g	<1	<1	<1
ESP	%	<1	<1	<1

Method ID	Methodology Summary
Ext-037	Analysed by Sydney Environmental & Soil Laboratory
Ext-062	Analysed by East West Enviroag
Inorg-001	pH - Measured using pH meter and electrode in accordance with APHA latest edition, 4500-H+. Please note that the results for water analyses are indicative only, as analysis outside of the APHA storage times.
Inorg-002	Conductivity and Salinity - measured using a conductivity cell at 25°C in accordance with APHA latest edition 2510 and Rayment & Lyons.
INORG-127	Total Nitrogen by high temperature catalytic combustion with chemiluminescence detection.
Metals-020	Determination of exchangeable cations and cation exchange capacity in soils using 1M Ammonium Chloride exchange and ICP-OES analytical finish.
Metals-020	Calcium and Magnesium analysed by ICP-AES and SAR calculated.

QUALITY CONTROL: Misc Inorg - Soil						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-1	[NT]
Date prepared	-			12/05/2022	1	12/05/2022	12/05/2022		12/05/2022	
Date analysed	-			12/05/2022	1	12/05/2022	12/05/2022		12/05/2022	
pH 1:5 soil:water	pH Units		Inorg-001	[NT]	1	5.2	5.1	2	100	
Electrical Conductivity 1:5 soil:water	µS/cm	1	Inorg-002	<1	1	48	39	21	104	
Total Nitrogen in Soil	mg/kg	10	INORG-127	<10	1	170	170	0	92	
Sodium Adsorption Ratio		0.01	Metals-020	<0.01	1	1.2	[NT]		113	
Emerson Class No.	-	0	Ext-037	[NT]	1	5.0	[NT]		[NT]	
Phosphorus Sorption Capacity	mg/kg	na	Ext-062	[NT]	1	670	[NT]		[NT]	

QUALITY CONTROL: ESP/CEC					Duplicate			Spike Recovery %		
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date prepared	-			12/05/2022	[NT]		[NT]	[NT]	12/05/2022	
Date analysed	-			13/05/2022	[NT]		[NT]	[NT]	13/05/2022	
Exchangeable Ca	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	84	
Exchangeable K	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	95	
Exchangeable Mg	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	88	
Exchangeable Na	meq/100g	0.1	Metals-020	<0.1	[NT]		[NT]	[NT]	97	
ESP	%	1	Metals-020	<1	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]

Result Definiti	ons
NT	Not tested
NA	Test not required
INS	Insufficient sample for this test
PQL	Practical Quantitation Limit
<	Less than
>	Greater than
RPD	Relative Percent Difference
LCS	Laboratory Control Sample
NS	Not specified
NEPM	National Environmental Protection Measure
NR	Not Reported

Quality Contro	Quality Control Definitions							
Blank	This is the component of the analytical signal which is not derived from the sample but from reagents, glassware etc, can be determined by processing solvents and reagents in exactly the same manner as for samples.							
Duplicate	This is the complete duplicate analysis of a sample from the process batch. If possible, the sample selected should be one where the analyte concentration is easily measurable.							
Matrix Spike	A portion of the sample is spiked with a known concentration of target analyte. The purpose of the matrix spike is to monitor the performance of the analytical method used and to determine whether matrix interferences exist.							
LCS (Laboratory Control Sample)	This comprises either a standard reference material or a control matrix (such as a blank sand or water) fortified with analytes representative of the analyte class. It is simply a check sample.							
Surrogate Spike	Surrogates are known additions to each sample, blank, matrix spike and LCS in a batch, of compounds which							

Australian Drinking Water Guidelines recommend that Thermotolerant Coliform, Faecal Enterococci, & E.Coli levels are less than 1cfu/100mL. The recommended maximums are taken from "Australian Drinking Water Guidelines", published by NHMRC & ARMC 2011.

are similar to the analyte of interest, however are not expected to be found in real samples.

The recommended maximums for analytes in urine are taken from "2018 TLVs and BEIs", as published by ACGIH (where available). Limit provided for Nickel is a precautionary guideline as per Position Paper prepared by AIOH Exposure Standards Committee, 2016.

Guideline limits for Rinse Water Quality reported as per analytical requirements and specifications of AS 4187, Amdt 2 2019, Table 7.2

Laboratory Acceptance Criteria

Duplicate sample and matrix spike recoveries may not be reported on smaller jobs, however, were analysed at a frequency to meet or exceed NEPM requirements. All samples are tested in batches of 20. The duplicate sample RPD and matrix spike recoveries for the batch were within the laboratory acceptance criteria.

Filters, swabs, wipes, tubes and badges will not have duplicate data as the whole sample is generally extracted during sample extraction.

Spikes for Physical and Aggregate Tests are not applicable.

For VOCs in water samples, three vials are required for duplicate or spike analysis.

Duplicates: >10xPQL - RPD acceptance criteria will vary depending on the analytes and the analytical techniques but is typically in the range 20%-50% – see ELN-P05 QA/QC tables for details; <10xPQL - RPD are higher as the results approach PQL and the estimated measurement uncertainty will statistically increase.

Matrix Spikes, LCS and Surrogate recoveries: Generally 70-130% for inorganics/metals (not SPOCAS); 60-140% for organics/SPOCAS (+/-50% surrogates) and 10-140% for labile SVOCs (including labile surrogates), ultra trace organics and speciated phenols is acceptable.

In circumstances where no duplicate and/or sample spike has been reported at 1 in 10 and/or 1 in 20 samples respectively, the sample volume submitted was insufficient in order to satisfy laboratory QA/QC protocols.

When samples are received where certain analytes are outside of recommended technical holding times (THTs), the analysis has proceeded. Where analytes are on the verge of breaching THTs, every effort will be made to analyse within the THT or as soon as practicable.

Where sampling dates are not provided, Envirolab are not in a position to comment on the validity of the analysis where recommended technical holding times may have been breached.

Measurement Uncertainty estimates are available for most tests upon request.

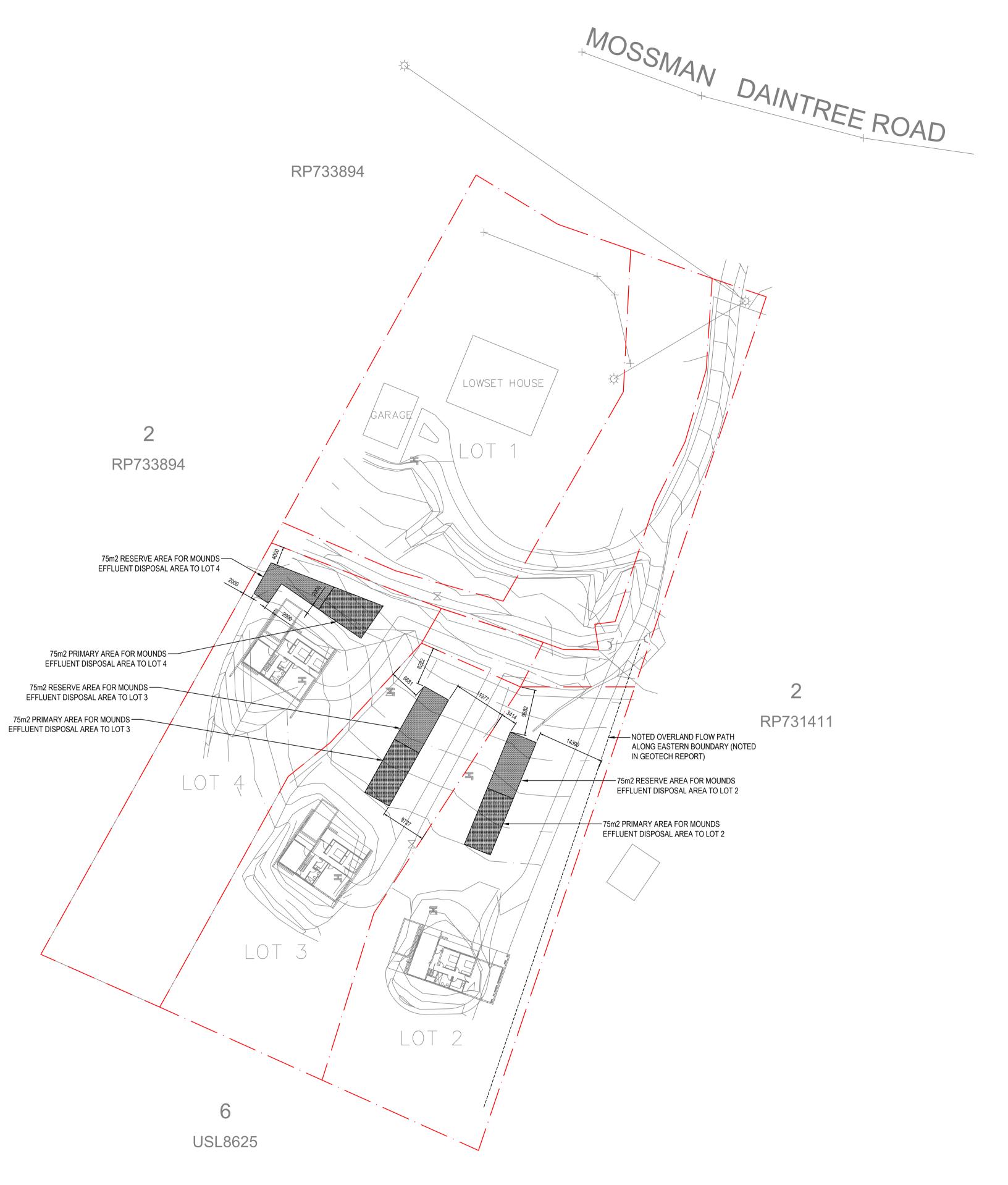
Analysis of aqueous samples typically involves the extraction/digestion and/or analysis of the liquid phase only (i.e. NOT any settled sediment phase but inclusive of suspended particles if present), unless stipulated on the Envirolab COC and/or by correspondence. Notable exceptions include certain Physical Tests (pH/EC/BOD/COD/Apparent Colour etc.), Solids testing, total recoverable metals and PFAS where solids are included by default.

Samples for Microbiological analysis (not Amoeba forms) received outside of the 2-8°C temperature range do not meet the ideal cooling conditions as stated in AS2031-2012.

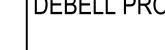
Report Comments

Emerson Class No. & Phosphorus Sorption Capacity analysed by East West Geo Ag Enviro. Report No. EW220977

Appendix C – Site Plan – Effluent Disposal Plan

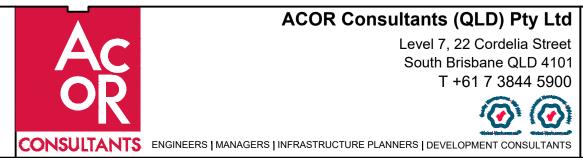


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This drawin	ng has been assigned an electronic code that signifies the drawing has been checked and approved by:					Client
					North	DEB
02	FOR APPROVAL	24.06.22	CC	DC		
01	DRAFT ISSUE	22.06.22	CC	DC		
Issue	Description	Date	Drawn	Approved		
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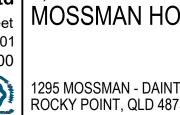


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<u>NOTES</u>

1. THE WHOLE OF THE PLUMBING AND DRAINAGE WORK SHALL BE INSTALLED IN ACCORDANCE WITH QLD PLUMBING AND DRAINAGE ACT 2018, QLD PLUMBING AND DRAINAGE REGULATION 2019, AS/NZS:3500, AS/NZS1547, THE LOCAL AUTHORITY APPROVED DRAWINGS, REQUIREMENTS & COMPLIANCE PERMIT CONDITIONS.

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Appendix D – Architectural Plans

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 AMENDMENTS

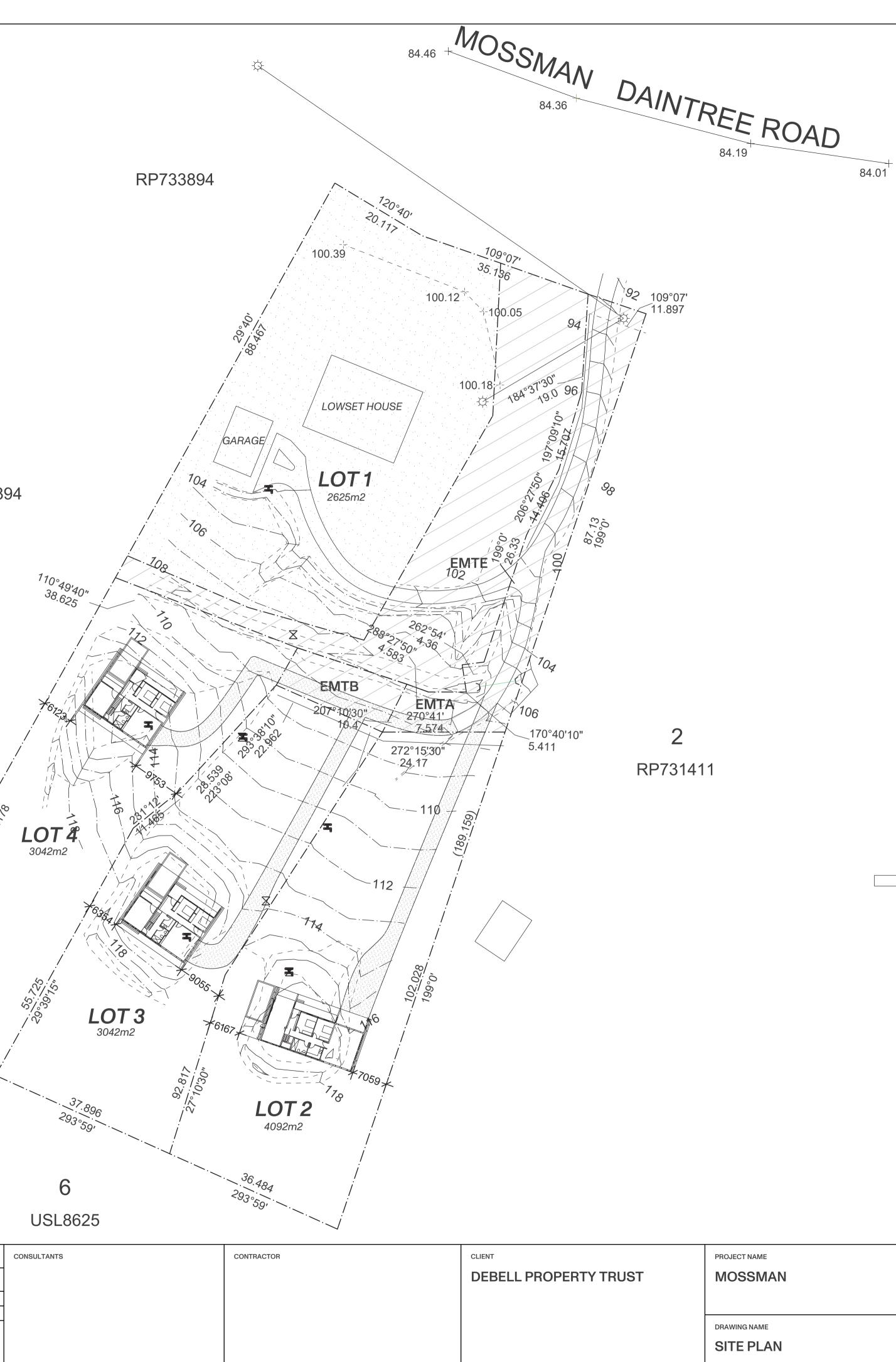
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 ISSUE
 DATE
 SUBJECT
 AUTH

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 25.04.2022
 CONSULTANT ISSUE
 JD

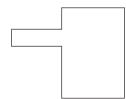
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ARCHITECT

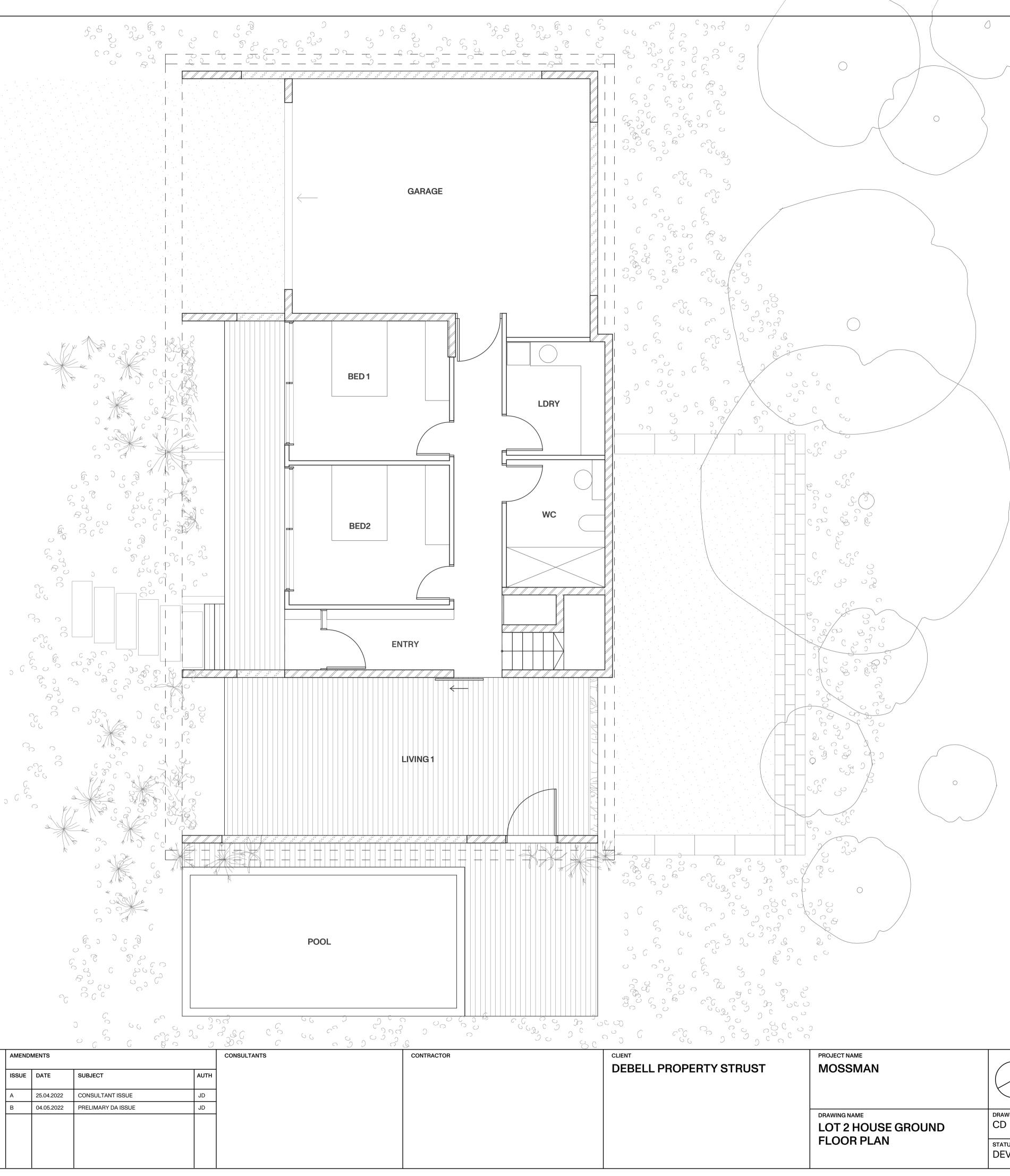
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	status DEVELOPI			



NOTE: EXACT LOCATION AND ORIENTATION OF DWELLING TO BE CONFIRMED



GROUND FLOOR PLAN

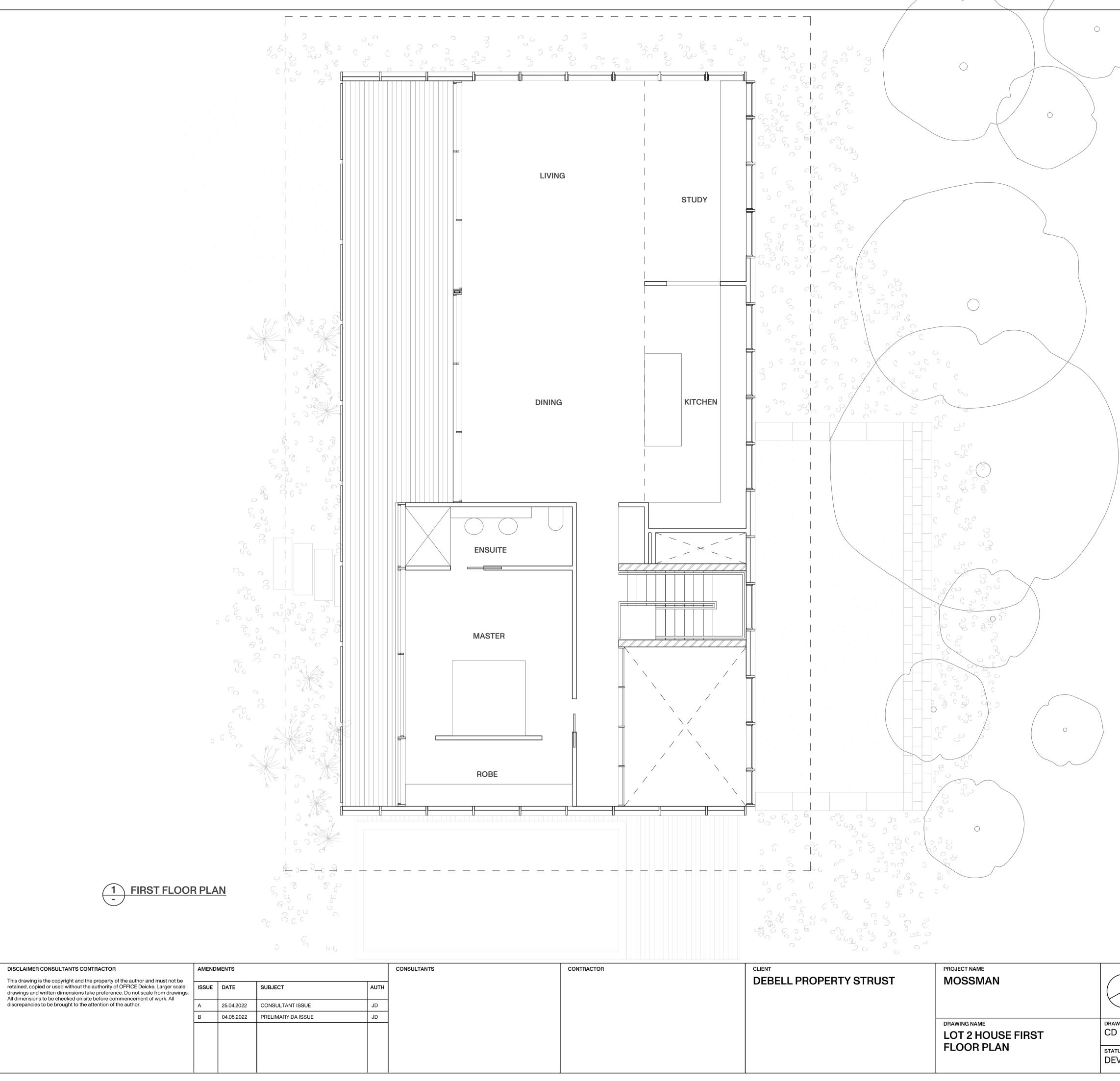
ARCHITECT

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AMENDMENTS							
ISSUE	DATE	SUBJECT	AUTH				
А	25.04.2022	CONSULTANT ISSUE	JD				
В	04.05.2022	PRELIMARY DA ISSUE	JD				

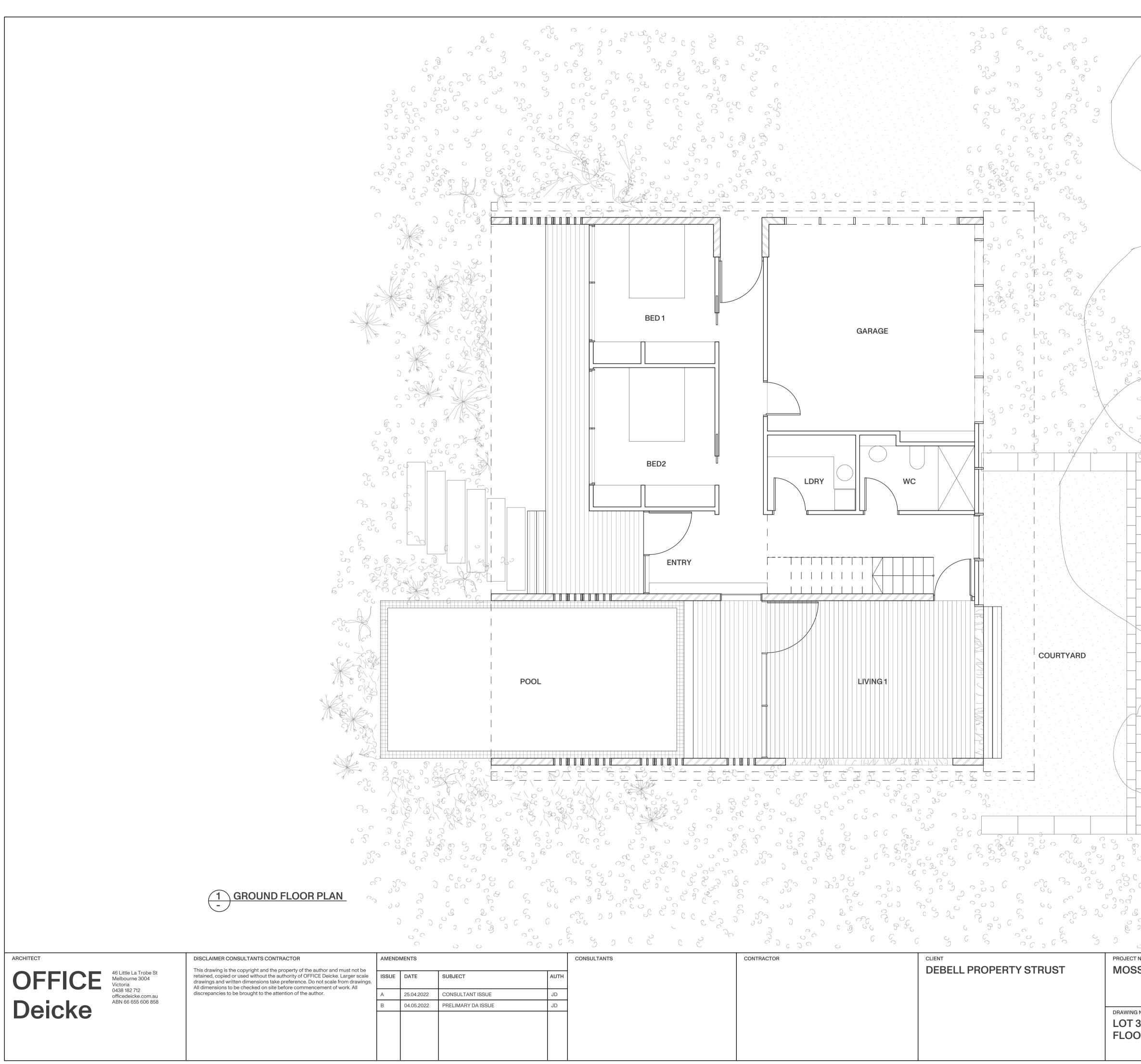
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		DA_A2_0201		В		
G NAME	DRAWN		CHECKED	AUTH.		
2 HOUSE GROUND	CD		JD	JD		
OR PLAN	STATUS					
	DEVELOPMENT APPLICATION					



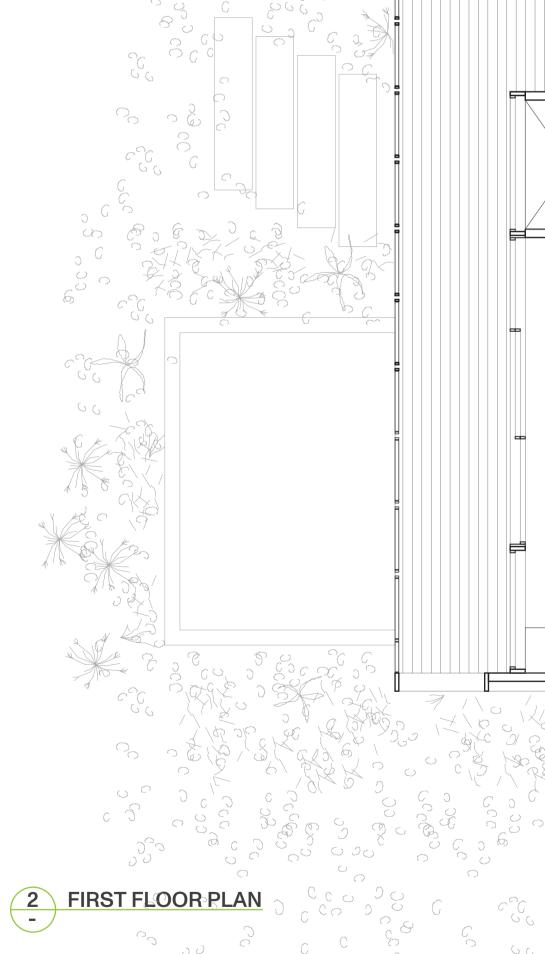
ARCHITECT

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OR PLAN	status DEVELOPI			



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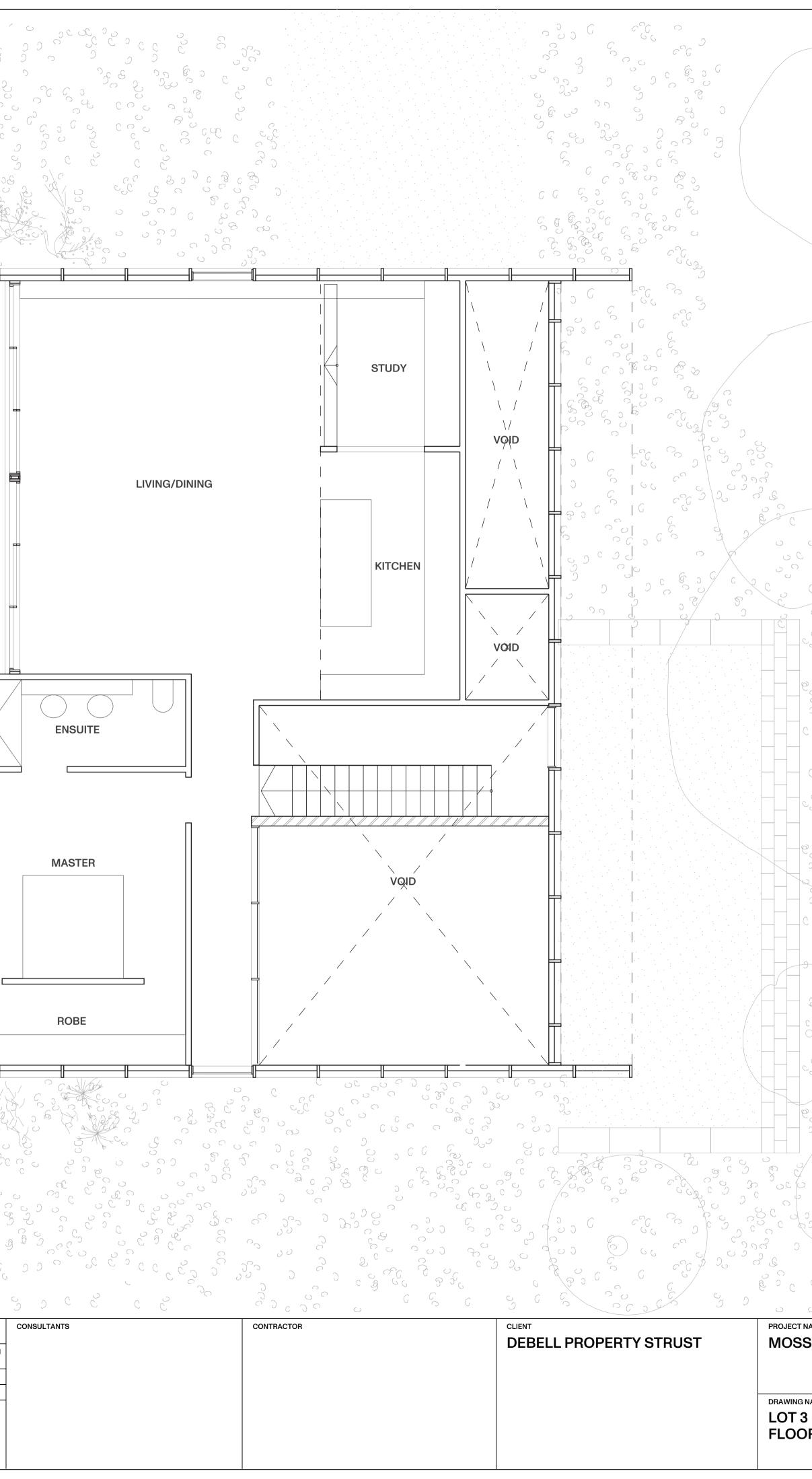


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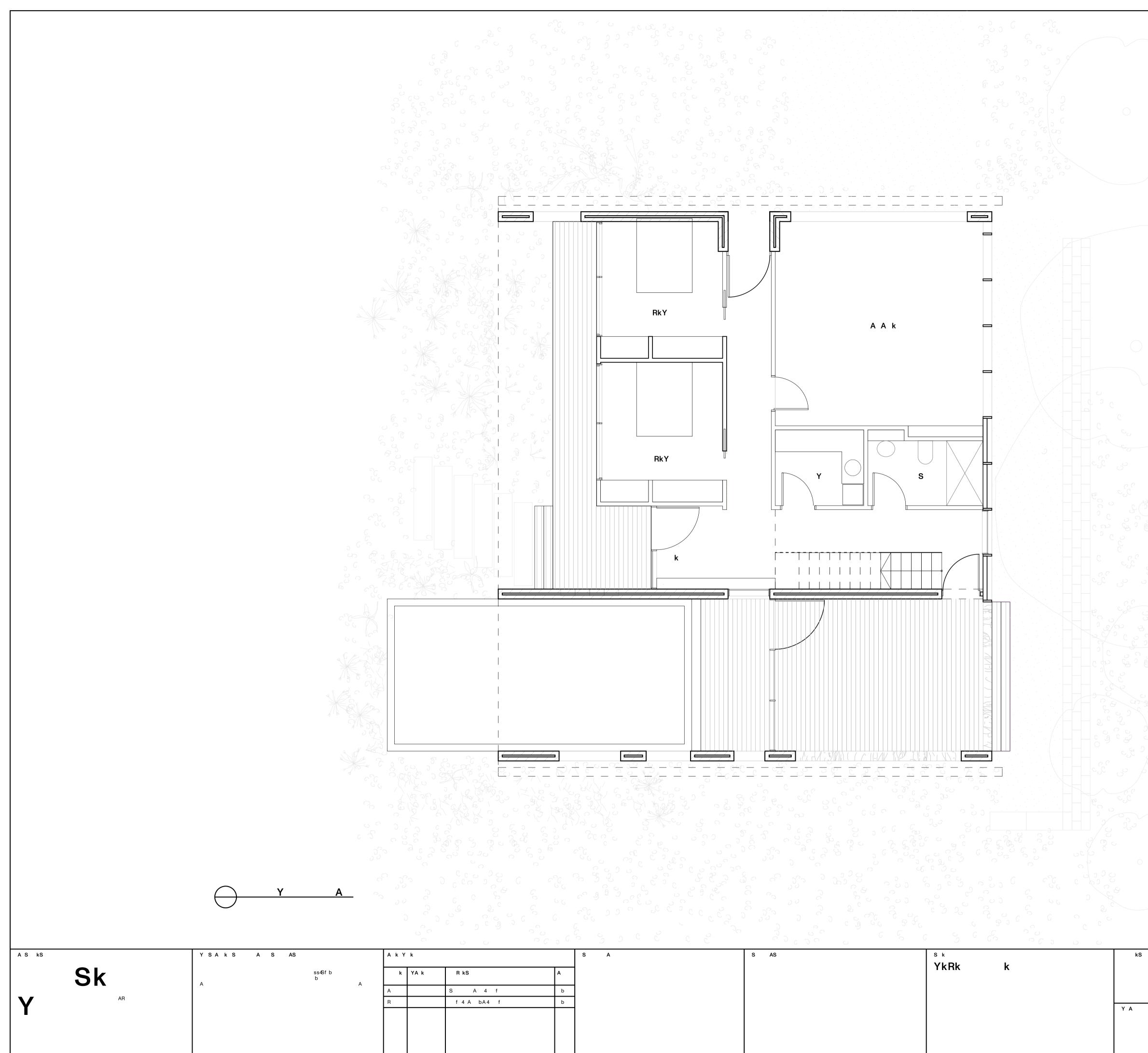
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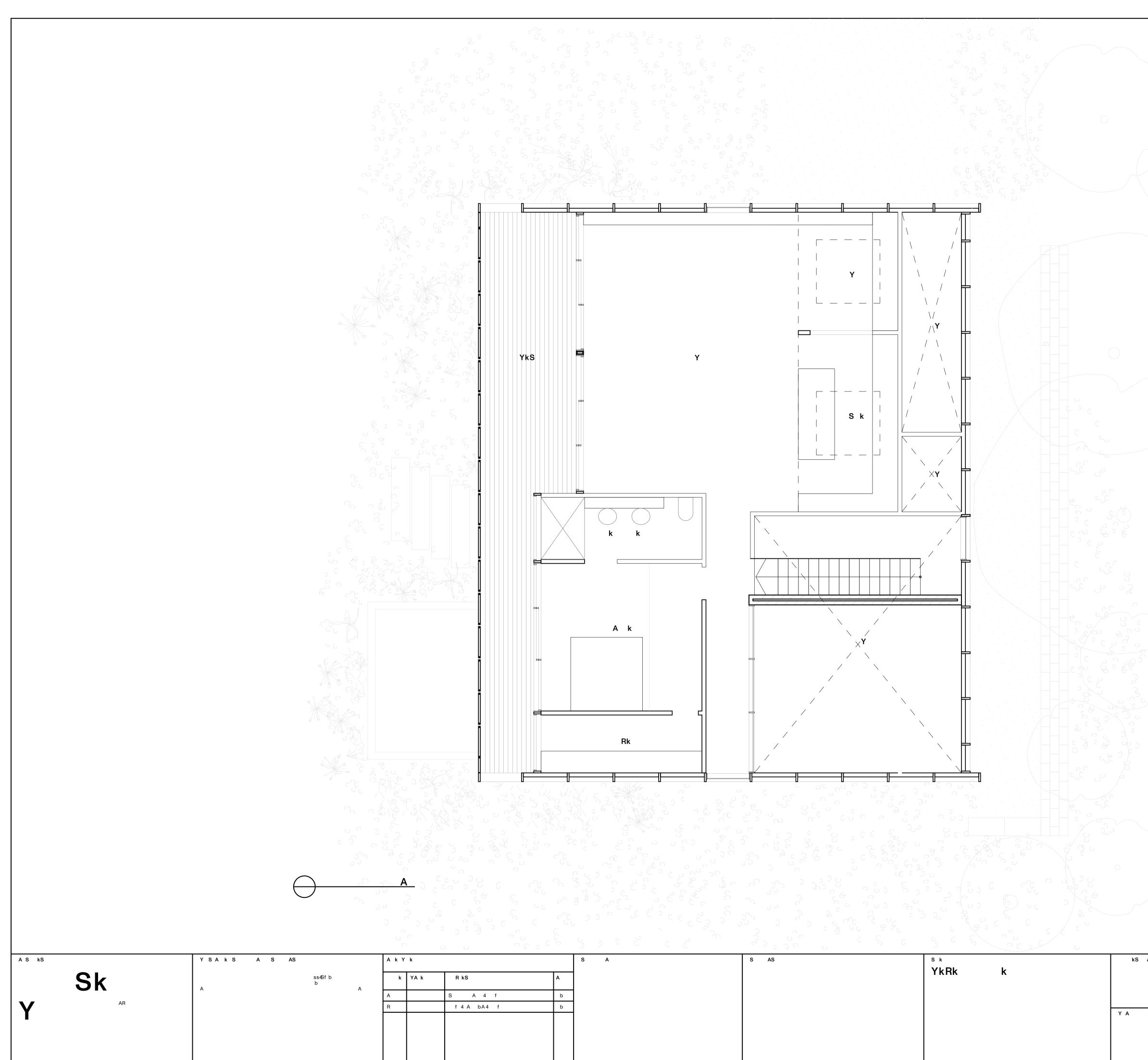
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В	04.05.2022	PRELIMARY DA ISSUE	JD



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Appendix E – Calculations

Job Number -Job Name -

Engineer -Date - BR220116 Proposed Residences Mossman-Daintree Road, Rocky Point DC 22/06/2022

Effluent Land Application System Calculations

1. Effluent Volume Calculations

		Initial		
Source	Flow Allowance (I/person/day)	No. persons	Total Flow (I/day)]
People Residing	150	4	600	<- based on advice from Architect and
TOTAL			600	Appendix H AS1547:2012
TOTAL / week			4200]

Note: 1. Residence uses reticulated community mains water and flow allowance makes the calculated total flow conservative as all new houses require water saving devices to comply with the QDC.

2. Soil Properties

Texture -	Clay Loams	
Structure -	Weekly Structured	
Indicative Permeability -	<0.12mm/d	
Indicative drainage class -	Poorly Drained	
Soil Category -	4	<refer as1547:2012<="" m1="" table="" td=""></refer>

3. Irrigation Area Calculations

Design Irrigation Rate (DIR) -	3.5	mm/day	<refer as1547:2012<="" m1="" table="" th=""></refer>
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Area Requirement (A) - Q(week) / DIR

Irrigation Area Requirement		
sq.m.		
Area	171	

4. Mound Bed Area Calculations

Design Loading Rate (DLR) - 8 mm/day <----- refer table N1 AS1547:2012 (requires further qualificiation)

Basal Area Requirement (A) - Q(day) / DLR

Mound Bed Ba		
	sq.m.	
Area	75	< ADOPTED

5. Evapotranspiration Area Calculations

Design Loading Rate (DLR) - 6 mm/day <-----refer table L1 AS1547:2012

Area Requirement (A) - Q(day) / DLR

Evapotranspira		
	sq.m.	
Area	100	< would not recommend this
Length if 4.0m wide	25	system on a steeply sloping site

6. Absorption Trench and Beds

Design Loading Rate (DLR) - 20 mm/day <-----refer table L1 AS1547:2012

Area Requirement (A) - Q(day) / DLR

Absorption Trench]	
	sq.m.	
Area	30	<based on="" secondary="" th="" treated<=""></based>
Length if 4.0m wide	8	effluent disposal
		(NOT recommended in Class 6 soils)

Notes :

1. Calculations have been based on proccedures set out in AS/NZS 1547:2012 "On-site domestic-wastewater management"



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PO Box 3635 South Brisbane QLD 4101

ENGINEERS

MANAGERS

INFRASTRUCTURE PLANNERS

DEVELOPMENT CONSULTANTS

WATER PRESSURE EVALUATION FOR DOMESTIC WATER SUPPLY

FOR

PROPOSED RESIDENCES MOSSMAN-DAINTREE ROAD ROCKY POINT

Prepared for:Office DeickeProject no:BR220116Date:24th June 2022Revision:01





Revisions

Date	Issue	Revision Description	Prepared By	Reviewed By	Approved By
24.06.22	02	Final Issue	David Colmer	Karl Paton	David Colmer

It is the responsibility of the reader to verify the currency of the version number of this report. All subsequent releases will be made directly to the Client.

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1 INTRODUCTION

ACOR Consultants have been engaged by Office Deicke to prepare water assessment for the domestic water supply at 1299 Mossman-Daintree Road, Rocky Point to determine domestic water storage and pump requirements for the project.

DA conditions from Douglas Shire Council state the client must provide council with flow and pressure testing from a fire hydrant on the authority main at the front of the property to determine the available domestic water flow and pressure to site.

2 PROPOSED DEVELOPMENT

The project comprises construction of three (3) individual residences located on three (3) separate but adjoining allotments Lot 2, Lot 3 and Lot 4 of the subdivided property at 1299 Mossman Daintree Road. Each residence to be two storey and constructed on the existing building platforms provisioned when the property was subdivided.

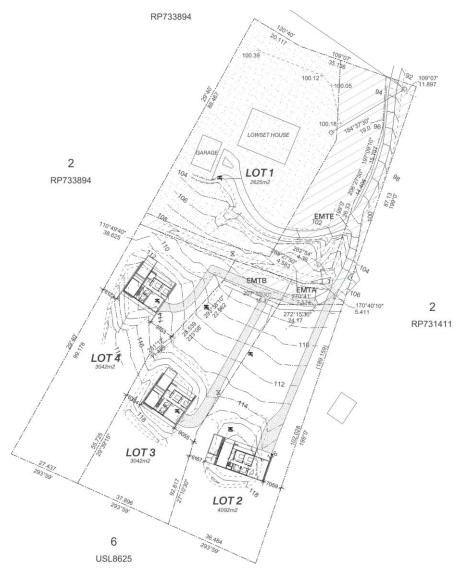


Figure 2.1 – Proposed Site Locality

3 SITE INVESTIGATION

A flow and pressure investigation was undertaken on the 30 May 2022 by Budget Fire Protection.

There is a Ø150mm water main on the opposite side of Mossman-Daintree Road. Council advised this service is to be made redundant in the future. There is a Ø225mm water main on the same side of Mossman-Daintree Road with a fire hydrant located towards the end of this main. This street hydrant was used for testing purposes.

The site has three water meters located at the north-east corner of the site which are fed via a Ø50mm branch pipe connected to the Ø225mm authority main.

To ascertain the available flow and pressure in the Ø225mm authority main, water was drawn at 5L/s increments from the fire hydrant while recording the pressure at each increment up to a maximum flow of 16L/s @ 0kPa. This was done while simultaneously recording the residual pressure at each increment at the water supply entering the property. Refer Appendix A – Flow and Pressure Testing.

4 **PROPOSED SOLUTION**

We note the following design parameters:

- Flow and pressure testing indicated there is 480kPa of pressure available in the authority main at static (no water being drawn).
- The authority main is located at approx. RL 91m and the most disadvantaged
- The most disadvantaged fixture in the Lot 2 residence will be at approx. RL 120m
- The most disadvantaged fixture in the Lot 3 residence will be at approx. RL 120m
- The most disadvantaged fixture in the Lot 4 residence will be at approx. RL 116m
- The minimum design pressure to be provided at fixtures is 250kPa
- The maximum design pressure to be provided at fixtures is 500kPa

It is not proposed to provide any domestic water storage at each residence noting the authority main is capable of suppling 5L/s @ 400kPa, with each residence only requiring 0.38L/s.

It is proposed to provide a domestic water pressure booster pump at each residence to ensure minimum domestic water pressures for the residences. Refer Appendix B site plan for proposed locations.

Each pump to be a Grundfos JP PT-V booster (or approved equivalent) that consists of a vertical pressure tank and a self-priming, single-stage centrifugal pump. Refer to Appendix C for pump specification and Appendix B for calculations.

Appendix A – Flow and Pressure Testing

TOWNSVILLE

PO Box 1133 Oonoonba QLD 4811 Phone 4780 4339 Fax 4778 8299

BURDEKIN

PO Box 9 Brandon QLD 4808 Phone 4782 5691 Fax 4778 8299

> QBSA 1126843 ABN 27-127-284-539

BUDGET FIRE PROTECTION

Australian Standards

Accor Consultants PO Box 3635 South Brisbane 4101 30/4/2022

Attention David Colmer

Report on 1299 Mossman Daintree Road flow readings

At the front of the property, you have a 150-mil pipe on the opposite side of the highway to the property.

On the same side as 1299 you have a 225-mil pipe that is feeding the properties.1299 has 3 water meters entering the property via a 50-mil pipe.

The water authority officer for the shire said that the 150-mil pipe will become redundant due to age and condition.

He advised that we would be wasting our time taking readings from this main.

There is a hydrant nearly at the end of the 225-mil pipe line so I connected my flow meter up to it and discharged water at 5 litres per second up to a maximum flow of 16 litres per second where the pressure was 0 at my flow meter.

The council fellows took pressure readings going into the property as follows

Flow meter at hydrant (close to the end of the line) Static 480kPa 5 Litres a second 440Kpa 10 Litres a second 300Kpa 15 Litres a second 120Kpa 16 litres a second 0Kpa Pressure readings going into 1299 400Kpa 400Kpa 390Kpa 380Kpa 380Kpa

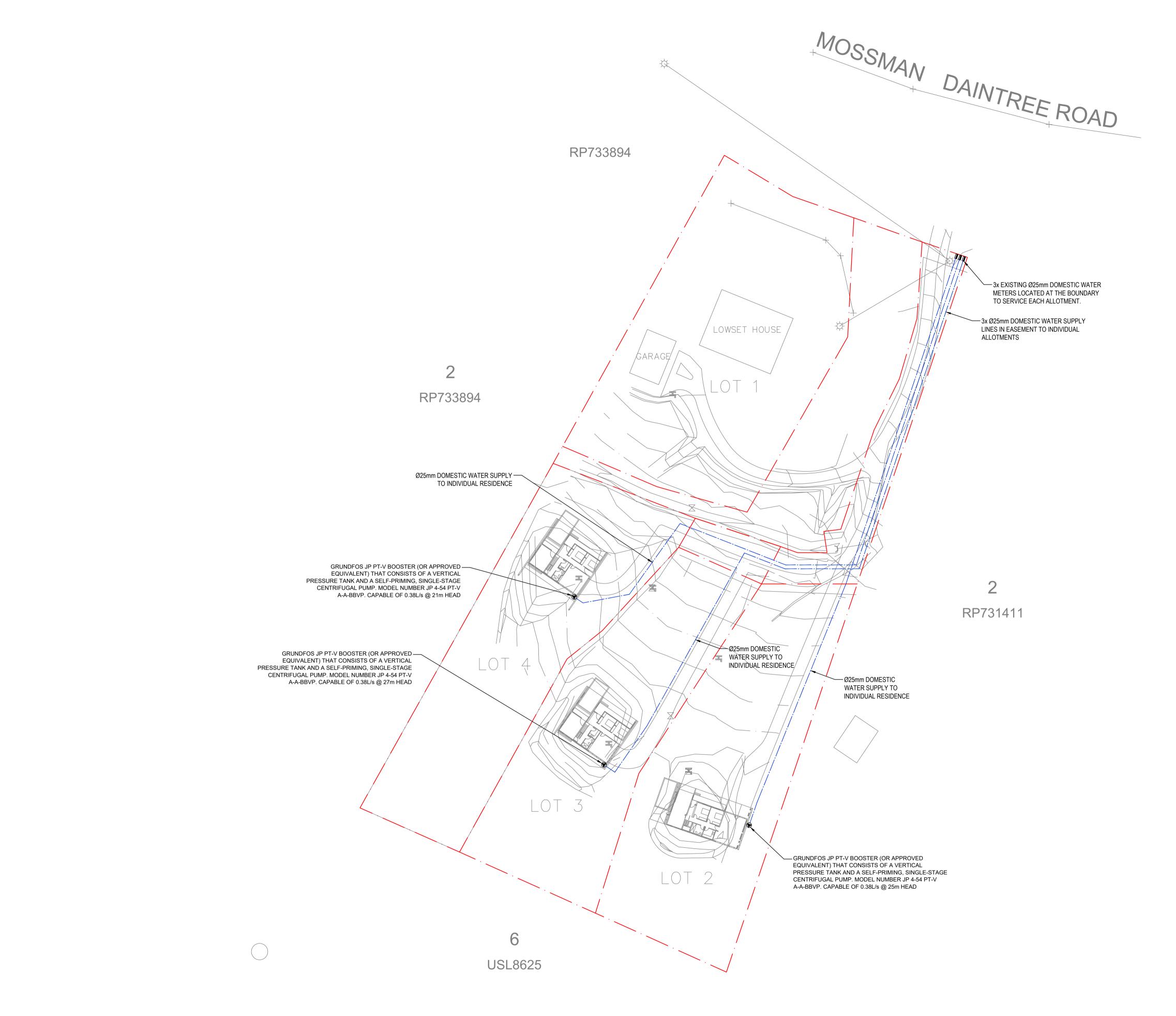
If you require additional information in regards to the above please ring me on 0408 279 322.

Kind Regards,

3. VJate

Brian Bates Budget Fire Protection

Appendix B – Site Plan – Water Services



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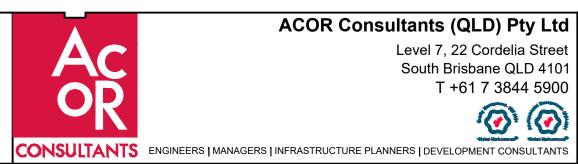
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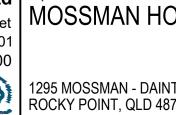
DEBELL PROPERTY TRUST

Architect

OFFICE Deicke 46 Little La Trobe Street Melbourne, VIC 3004 PHONE : 0438 182 712



ACOR Consultants (QLD) Pty Ltd Level 7, 22 Cordelia Street South Brisbane QLD 4101 T +61 7 3844 5900



<u>NOTES</u>

THE WHOLE OF THE PLUMBING AND DRAINAGE WORK SHALL BE INSTALLED IN ACCORDANCE WITH QLD PLUMBING AND DRAINAGE ACT 2018, QLD PLUMBING AND DRAINAGE REGULATION 2019, AS/NZS:3500, THE LOCAL AUTHORITY APPROVED DRAWINGS, REQUIREMENTS & COMPLIANCE PERMIT CONDITIONS.

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Appendix C – Pump Specification





Company name: Created by: Phone:

			Date:	24/06/2022	
Qty.	Description				
	Max. ambient: Maximum operating pressure: Pump inlet:	40 °C PN 6 bar G 1			
	Pump outlet:	G 1			
	Electrical data:				
	Power input - P1:	1130 W			
	Rated power - P2:	0.746 kW			
	Mains frequency:	50 Hz			
	Rated voltage:	1 x 230 V			
	Rated current:	5.1 A			
	Starting current:	17.8 A			
	Rated speed:	2800 rpm			
	Capacitor size - run:	16 µF/450 V			
	Enclosure class (IEC 34-5):	IP44			
	Insulation class (IEC 85):	F			
	Length of cable:	1.5 m			
	Type of cable plug:	AUS			
	Tank:				
	Volume of pressure tank:	18			
	Others:				
	Net weight:	18 kg			
	Gross weight:	19 kg			
	Country of origin:	CN			
	Custom tariff no.:	8413709062			

Appendix D – Calculations

LOT 2 RESIDENCE CALCULATIONS

Job Name: Mossman Houses	Job No: B	R220116	Date: 23.06.22	
Minimum Pressure in Main at desig	n flow rate:	470.88 m/head	4619.333 kPa	
Desigr	n Flow Rate:	0.38 L's		
Approximate	RL of Main:	91 m	892.71 kPa	
Pressure Loss throug	h backflow:	5.1 m/head	50 kPa	
RL of most disadvant	aged point:	120 m	1177.2 kPa	
RL of most advant	aged point:	114 m	1118.34 kPa	
Minimum require	ed pressure:	22.4 m/head	250 kPa	
Pressure Loss From Mai	in to Meter:	5.2 m/head	50.96 kPa	
Pressure Loss After WaterMeter an	d backflow:	8.39 m/head	82.30 kPa	
Pressure loss at Water main Conne	ction Point:	5.16 m/head	50.60 kPa	
Total F	Pipe Losses:	18.74 m/head	183.86 kPa	
Residu	al Pressure:	-25.23 m/head	-247.47 kPa	
Is a Pum	p Required: P	ump is Required		
	Pump Flow:	0.38 L's		
<mark>۱</mark>	Pump Head:	25 m/head	247 kPa	
Pressure at most advantaged point with Pur	mp running:	45 m/head/static	443 kPa/Static	
Pressure at most advantaged point without Pur	mp running:	6 m/head	61 kPa	
Minimum pressure provided at most disadvant	aged Point:	25.5 m/head	250 kPa	

LOT 3 RESIDENCE CALCULATIONS

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bb Name: Mossman Houses Job No: B		R220116	Date: 23.06.22
Minimum Pressure in Main at	design flow rate:	470.88 m/head	4619.333 kPa
[Design Flow Rate:	0.38 L's	
Approxi	mate RL of Main:	91 m	892.71 kPa
Pressure Loss t	hrough backflow:	5.1 m/head	50 kPa
RL of most disa	dvantaged point:	120 m	1177.2 kPa
RL of most a	dvantaged point:	114 m	1118.34 kPa
Minimum re	equired pressure:	22.4 m/head	250 kPa
Pressure Loss From Main to Meter:		5.2 m/head	50.96 kPa
Pressure Loss After WaterMe	ter and backflow:	9.77 m/head	95.80 kPa
Pressure loss at Water main C	Connection Point:	5.16 m/head	50.60 kPa
1	Total Pipe Losses:	20.12 m/head	197.36 kPa
Residual Pressure:		-26.60 m/head	-260.97 kPa
ls a	Pump Required: Pu	Imp is Required	
	Pump Flow:	0.38 L's	
	Pump Head:	27 m/head	261 kPa
Pressure at most advantaged point wit	th Pump running:	47 m/head/static	456 kPa/Static
Pressure at most advantaged point without	ut Pump running:	5 m/head	48 kPa
Minimum pressure provided at most disa	dvantaged Point:	25.5 m/head	250 kPa

LOT 4 RESIDENCE CALCULATIONS

Job Name: Mossman Houses	Job No: B	R220116	Date: 23.06.22
Minimum Pressure in Main at	design flow rate:	470.88 m/head	4619.333 kPa
D	esign Flow Rate:	0.38 L's	
Approxir	nate RL of Main:	91 m	892.71 kPa
Pressure Loss th	rough backflow:	5.1 m/head	50 kPa
RL of most disad	lvantaged point:	116 m	1137.96 kPa
RL of most ad	lvantaged point:	112 m	1098.72 kPa
Minimum re	quired pressure:	22.4 m/head	250 kPa
Pressure Loss From	n Main to Meter:	5.2 m/head	50.96 kPa
Pressure Loss After WaterMet	er and backflow:	8.39 m/head	82.30 kPa
Pressure loss at Water main Co	onnection Point:	5.16 m/head	50.60 kPa
т	otal Pipe Losses:	18.74 m/head	183.86 kPa
Re	esidual Pressure:	-21.23 m/head	-208.23 kPa
ls a	Pump Required: Pu	ump is Required	
	Pump Flow:	0.38 L's	
	Pump Head:	21 m/head	208 kPa
Pressure at most advantaged point wit	n Pump running:	43 m/head/static	423 kPa/Static
Pressure at most advantaged point withou	t Pump running:	8 m/head	81 kPa
Minimum pressure provided at most disa	Ivantaged Point:	25.5 m/head	250 kPa