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WATER SUPPLY RETICULATION REPORT FOR OCEAN BREEZE ESTATE A RESIDENTIAL SUBDIVISION AT COOYA BEACH ROAD COOYA BEACH

Date: June 28, 2013

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1. INTRODUCTION

This report on the water supply reticulation for Ocean Breeze Estate, a residential subdivision at Cooya Beach, is provided to show that a satisfactory level of water supply service is available to meet the interim and ultimate development requirements.

This report provides the necessary input model assumptions and results.

This report complements another report prepared by SKM on the Cooya Beach water supply in March 2010 and supersedes our earlier report.

2. WATER RETICULATION NETWORK

The existing water supply network is shown on Dwgs 1187-OA Water 1 and Water 2. Dwg 1187-OA Water1 shows the proposed extension to the existing 225Ø water main from Point A to the existing reservoirs. Based on information provided by Cairns Water and Waste we understand that the intention is to convert the existing 150Ø water main to a dedicated inlet main and use the 225Ø water main as a dedicated outlet main to supply the Cooya Beach community including a fully developed Ocean Breeze Estate.

We have modeled the fully developed scenarios on this basis.

Water and Waste provided the following information regarding the reservoir servicing the site:

1. TWL 69.34m
2. BWL 60.07m
3. Volume 3.5ML (From SKM)

Therefore: 95% of TWL = RL 68.88
15% of TWL = RL 61.47

The connection between Nodes 101 and 102 across Cooya Beach Road is provided to allow an alternative source of water supply to some parts of the development. This measure is good practice and enhances firefighting flows but is not necessary during stage construction.

3. ASSUMPTIONS

1. No abnormal conditions affect the water supply
2. 50 rider mains are not modeled.
3. Cairns Water advised SKM that all property services include a pressure-reducing device to cut pressure to approximately 50m head. Therefore, separate and/or additional pressure reducing values are not required on any reticulation mains within the development.

4. WATER SUPPLY REQUIREMENTS

The level of water supply service expected of the reticulated system is in accordance with the requirements of:

1. FNQROC Development Manual 'Water Reticulation Design Guidelines'
2. Department of Natural Resources and Mines 'Planning Guidelines for Water Supply and Sewerage'

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Based on information from those documents the following data is used in the model:

Average daily consumption (AD) = 500L/person

Single family dwelling on

Lots < 900m ²	2.8 persons per connection
Lots > 900m ²	3.1 persons per connection

$$\begin{aligned}\text{Peak day (PD)} &= 2.25 \times \text{AD} \\ \text{Peak hour} &= \text{PD}/12\end{aligned}$$

Average Day demand for lot < 900m² = 0.016L/sec

Average Day demand for lot > 900m² = 0.018L/sec

Peak hour demand for lot < 900m² = 0.073L/sec 2/3 Peak Hour Demand = 0.049L/sec

Peak hour demand for lot > 900m² = 0.081L/sec 2/3 Peak Hour Demand = 0.054L/sec

From the figures shown above it can be determined that the demand at 2/3 of the Peak Hour is not less than the Average Day demand.

The peak hour flows were allocated to the nodes and a static analysis used. The residential pressure for domestic flows at peak hour is required to be in the range of 22m to 60m head.

5. FIRE FIGHTING FLOW

The 'Planning Guidelines for Water Supply and Sewerage' provides that a system with a population of less than 2000 persons have the firefighting flows imposed on 2/3 of the peak hour demand. Cooya Beach has an approximate population of about 1700 persons, which is well below the threshold noted above.

The water source for Cooya Beach is a reservoir reported by SKM to be 3.5ML capacity. The required firefighting flow is 15 L/sec for two hours and there is sufficient reservoir capacity for the firefighting flow.

The minimum permitted residential pressure during firefighting flow is 12m head. This is based on a reservoir level at 15% of top water level or RL 61.47. The maximum permitted pressure is 60.00m.

6. MAXIMUM RESIDUAL PRESSURE

The maximum residual head available, assuming no demand whatsoever, is about 66m. As noted above Cairns Water advised SKM that all property services include a pressure-reducing device to cut pressure to approximately 50m head.

7. RESIDENTIAL DEMANDS

Demand

This model is based on the following demands:

1. Existing Cooya Beach demands
2. Proposed subdivision demands

The existing Cooya Beach demands are based on information provided by the former DCS to SKM. This information is provided in Appendix A.

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The proposed subdivision demand is based on the current lot layout. These are summarised as follows:

Existing Cooya Beach demand external to the subject site: (Includes an undeveloped lot comprising potentially 20 lots)	280 lots
Existing demand from previous stages of the estate:	121 lots
Ocean Breeze Stage 5 demand:	10 lots
Demand in remaining stages of Ocean Breeze Estate:	157 lots
Total demand all lots at Cooya Beach:	568 lots

Surface elevations for the model nodes have been determined from the existing survey information provided by RPS. The survey data, existing and proposed lot layouts are shown on Dwgs 1187-OA Water 1 and Water 2, which are attached as Appendix B.

It is beyond the scope of this report to model the remainder of the Cooya Beach community. Therefore we have modeled the existing Cooya Beach consumption as demands at Nodes 118 and 146. Refer the above mentioned drawings for details.

8. MODELLING PARAMETERS

The reticulation network is modelled using EPA Net Program Version 2. The model is a static analysis at peak hour or part thereof as applicable.

This program analysed the reticulation network using Hazen-Williams head loss formula. Values of roughness coefficient 'c' used are in accordance with the FNQROC Development Manual requirements, which are:

$$\begin{array}{ll} \text{Diameter} < 150 & c=100 \\ \text{Diameter} > 150 < 300 & c=110 \end{array}$$

The following different scenarios are modeled:

- Scenario 1 – Existing development only at peak hour flow with reservoir level at RL 61.47m and with the sole supply line being the existing 150Ø water main in Boonie Doon Road.
- Scenario 2 – Existing development at peak hour flow with reservoir level at RL 61.47m and with a single 150Ø supply line from the reservoir to Point A as shown on Dwg 1187 - OA Water 1 and 150Ø augmented by a 225Ø supply lines from this point to the intersection of Bonnie Doon and Cooya Beach Roads.
- Scenario 3 – Existing development at peak hour flow with reservoir level at RL 61.47m and with a 150Ø a 225Ø supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.
- Scenario 4 - Full development at peak hour flow with reservoir level at RL 61.47m with a 150Ø and a 225Ø supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.
- Scenario 5 - Full developed at peak hour flow with reservoir level at RL 68.88m with the supply line being the existing 150Ø water main in Boonie Doon Road fully augmented by the new 225Ø main. This scenario represents the maximum pressure present in the system.
- Scenario 6 – Firefighting flows imposed on 2/3 of existing peak hour flows with reservoir level at RL 61.47m and with the sole supply line being the existing 150Ø water main in Boonie Doon Road.

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- Scenario 7 – Firefighting flows imposed on 2/3 existing peak hour flows with reservoir level at RL 61.47m and with a single 150Ø supply line from the reservoir to Point A as shown on Dwg 1187 - OA Water 1 and 150Ø augmented by a 225Ø supply lines from this point to the intersection of Bonnie Doon and Cooya Beach
- Scenario 8 – Firefighting Flows imposed on 2/3 of existing peak hour flows with reservoir level at RL 61.47m with a 150Ø and a 225Ø supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.
- Scenario 9 – Firefighting Flows imposed on 2/3 peak hour flows for full development with reservoir level at RL 61.47m with a 150Ø and a 225Ø supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.
- Scenario 10 – Firefighting flow imposed on the peak hour flow for the full development with reservoir level at RL 61.47m and with the supply line being the existing 150Ø water main in Boonie Doon Road fully augmented by the new 225Ø main.
- Scenario 11 – Firefighting flow imposed on the peak hour flow for the full development with reservoir level at RL 68.88m and with the supply line being the existing 150Ø water main in Boonie Doon Road fully augmented by the new 225Ø main.

9. PEAK HOUR FLOWS

Model results

The results of the modeling are shown in Appendix C. The results of the modeling for the various peak hour scenarios are summarised below:

Scenario 1

This scenario models the existing development only at peak hour flow with reservoir level at RL 61.47m and with the sole supply line being the existing 150Ø water main in Boonie Doon Road.

The modeling results show that this system is unsatisfactory with numerous nodes having less than the minimum pressure of 22m head.

Scenario 2

This scenario models the existing development at peak hour flow with reservoir level at RL 61.47m and with a single 150Ø supply line from the reservoir to Point A as shown on Dwg 1187 - OA Water 1 and 150Ø augmented by a 225Ø supply lines from this point to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service for the domestic supply but is deficient when the firefighting flows are considered. Refer to Scenario 7.

Scenario 3

This scenario models the existing development at peak hour flow with reservoir level at RL 61.47m and with a 150Ø a 225Ø supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service with the all nodes predicted to have strong residual pressures well in excess of the minimum 22m head.

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Scenario 4

This scenario models the full development at peak hour flow with reservoir level at RL 61.47m and with the sole supply line being the 225Ø water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service with the all nodes predicted to have strong residual pressures well in excess of the minimum 22m head.

Scenario 5

This scenario models the full development at peak hour flow with reservoir level at RL 68.88m and with the sole supply line being the 225Ø water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service with the all nodes predicted to have strong residual pressures well in excess of the minimum 22m head and no node with a predicted pressure in excess of 60.0m head.

10. FIRE FIGHTING FLOWS

Model results

The model was examined on a number of occasions to establish the most hydraulically disadvantaged hydrant. In the existing development the worst case is Node 131 and in the fully developed scenario Node 148 in Stage 3. The results of the modeling are shown in Appendix C and are summarised below:

Scenario 6

This scenario models the firefighting flows imposed on 2/3 of existing peak hour flows with reservoir level at RL 61.47m and with the sole supply line being the existing 150Ø water main in Bonnie Doon Road.

The modeling results show that this system is totally unsatisfactory with a predicted pressure at Node 131 of -22.59m

Scenario 7

This scenario models the firefighting flows imposed on 2/3 existing peak hour flows with reservoir level at RL 61.47m and with a single 150Ø supply line from the reservoir to Point A as shown on Dwg 1187 - OA Water and 150Ø augmented by a 225Ø supply lines from this point to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system is totally unsatisfactory with a predicted pressure at Node 131 of -0.43m

Scenario 8

This scenario models the firefighting flows imposed on 2/3 of existing peak hour flows with reservoir level at RL 61.47m with a 150Ø and a 225Ø supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service with a minimum residual pressure considerably greater than 12m at all nodes and with a predicted pressure at Node 131 of 20.78m

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Scenario 9

This scenario models the firefighting flows imposed on 2/3 peak hour flows for the full development with reservoir level at RL 61.47m and with the sole supply line being the 225Ø water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service for the fully developed estate and adjoining residential area with a minimum residual pressure considerably greater than 12m head at all nodes within the system with a predicted pressure at Node 148 of 21.75m.

Scenario 10

This scenario models the firefighting flows imposed on the full peak hour flows for the full development with the reservoir level at RL 61.47m and with the sole supply line being the 225Ø water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service for the fully developed estate and adjoining residential area with no negative pressures predicted.

Scenario 11

This scenario models the firefighting flows imposed on the full peak hour flows for the full development with the reservoir level at RL 68.88m and with the sole supply line being the 225Ø water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

The modeling results show that this system provides a satisfactory level of service for the fully developed estate and adjoining residential area with a minimum residual pressure considerably greater than 12m head at all nodes within the system with a predicted pressure at Node 148 of 18.65m.

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11. CONCLUSIONS

From the above analyses, it is obvious that to provide a water supply network to service the Cooya Beach community in accordance with the requirements of both FNQROC Development Manual, 'Water Reticulation Design Guidelines' and Department of Natural Resources and Mines 'Planning Guidelines for Water Supply and Sewerage', the existing 225Ø water main must be extended to the reservoir immediately.

When this work is completed, then:

1. The proposed water supply network provides a satisfactory level of domestic water supply for all the proposed development and the wider Cooya Beach community.
2. That the reticulation network within Cooya Beach has adequate capacity to meet future demands
3. No further upgrading of existing water supply infrastructure is required as a result of the development
4. The proposed water supply network within the subdivision as shown in Appendix B conforms with the requirements of both FNQROC Development Manual 'Water Reticulation Design Guidelines' and Department of Natural Resources and Mines 'Planning Guidelines for Water Supply and Sewerage'

We recommend that Council approve this report.

Attachments:

Appendix A	Existing Cooya Beach Demands.
Appendix B	Water Supply Reticulation Master Plans
Appendix C	Model Results

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APPENDIX A

EXISTING COOYA BEACH DEMANDS

facsimile transmission

TO: Wade Quinn

COMPANY: SKM

FAX NO: 4031 3967

SINCLAIR KNIGHT MERZ		
REC'D	14 FEB 2005	PMCR
WHO	Ned	
<i>[initials]</i>	<i>(initials)</i>	
PROJECT No.	6622501	
OUR REF: FILE		
YOUR REF:		



DATE: 14 February 2005

PAGES: 4
(including this page)

FROM: Peter Cymbala

DEPT: Engineering Services

Enquiries to: Peter Cymbala

Douglas Shire Council, PO Box 357, Mossman Qld 4873

Phone: (07) 4099 9462

Fax: (07) 4098 2902

Email: douglas@dsc.qld.gov.au

This facsimile is confidential and may be the subject of legal privilege. It is intended for the named addressee. If you are not the addressee, any use of this facsimile whatsoever or the information contained in it is prohibited. Please let us know immediately if you have received this communication in error so that we can arrange for it to be returned.

MESSAGE: Cooya Beach

Wade

Find attach sketches of
existing seines and Review site

Peter Cymbala
Engineering Services

WATOL.

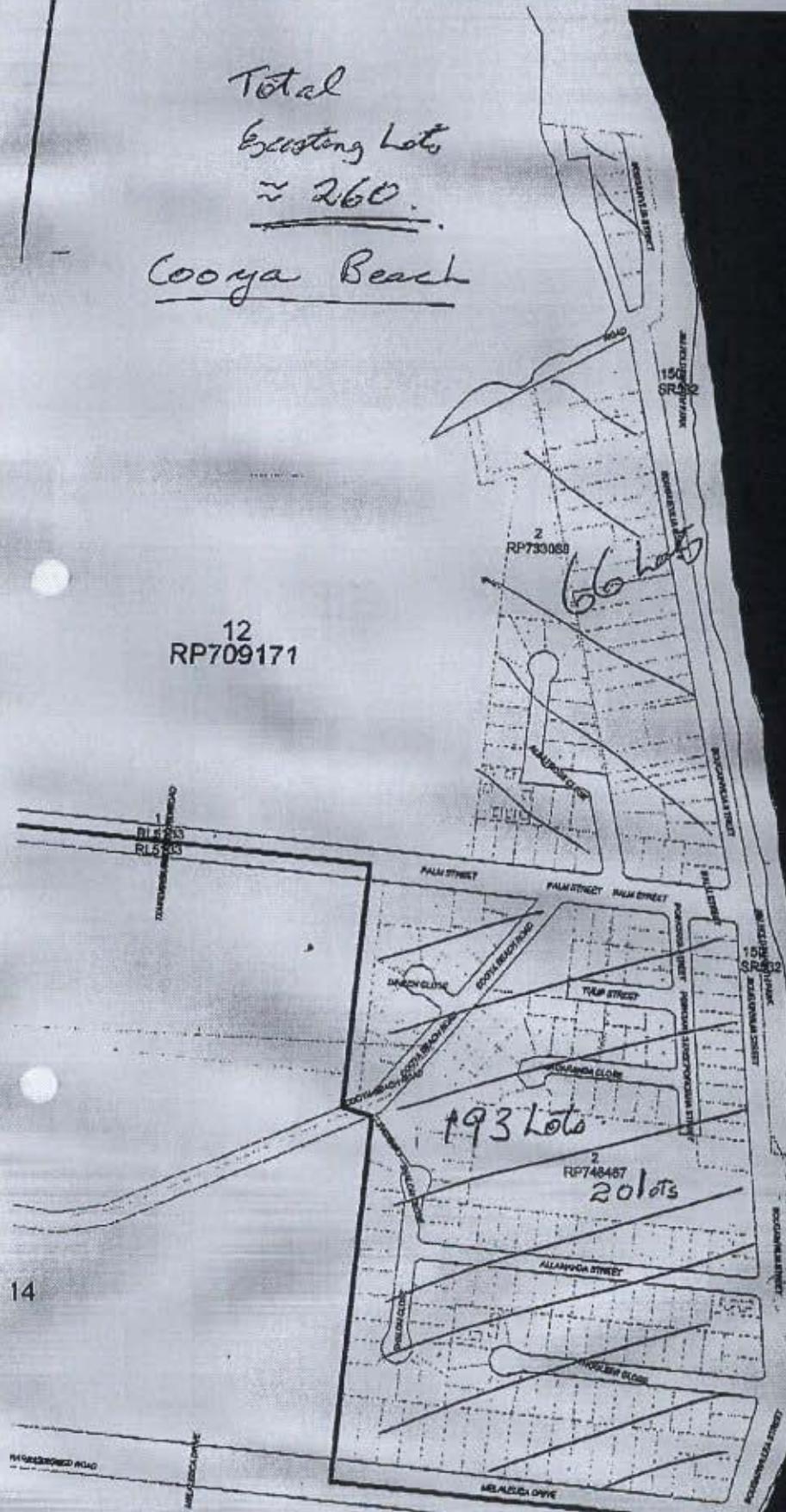
3967
4031

Douglas Shire Council

1



Total
existing lots
 ≈ 260 .
Cooya Beach

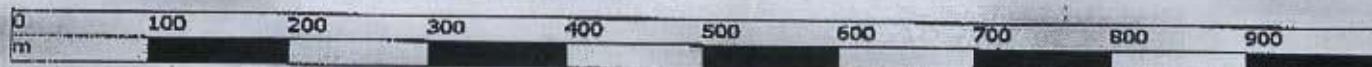
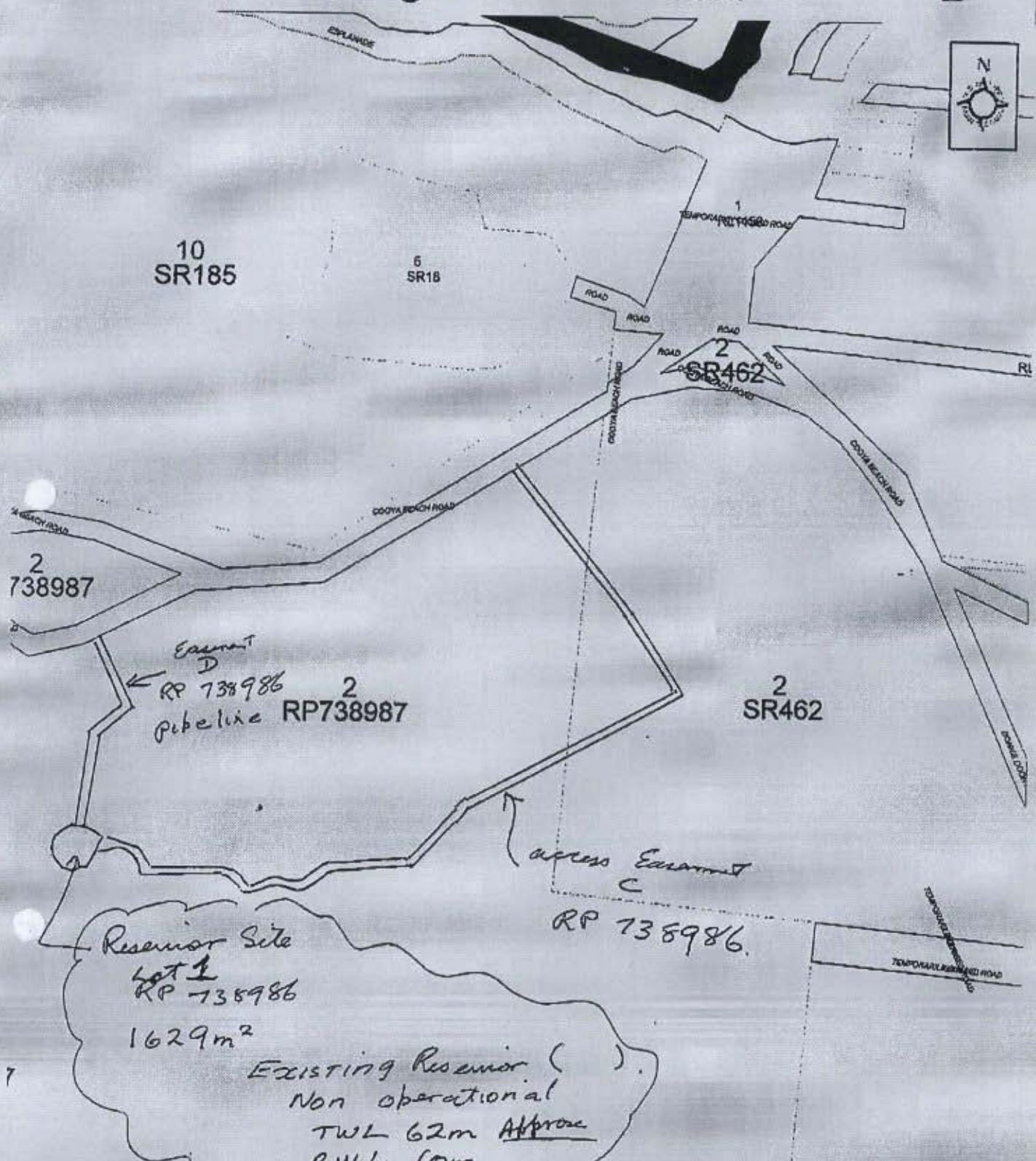
12
RP709171

0 100 200 300 400 500 600 700 800 900 1000
m

Scale: 1:5918.507 Date: 14/2/2005

Douglas Shire Council

3



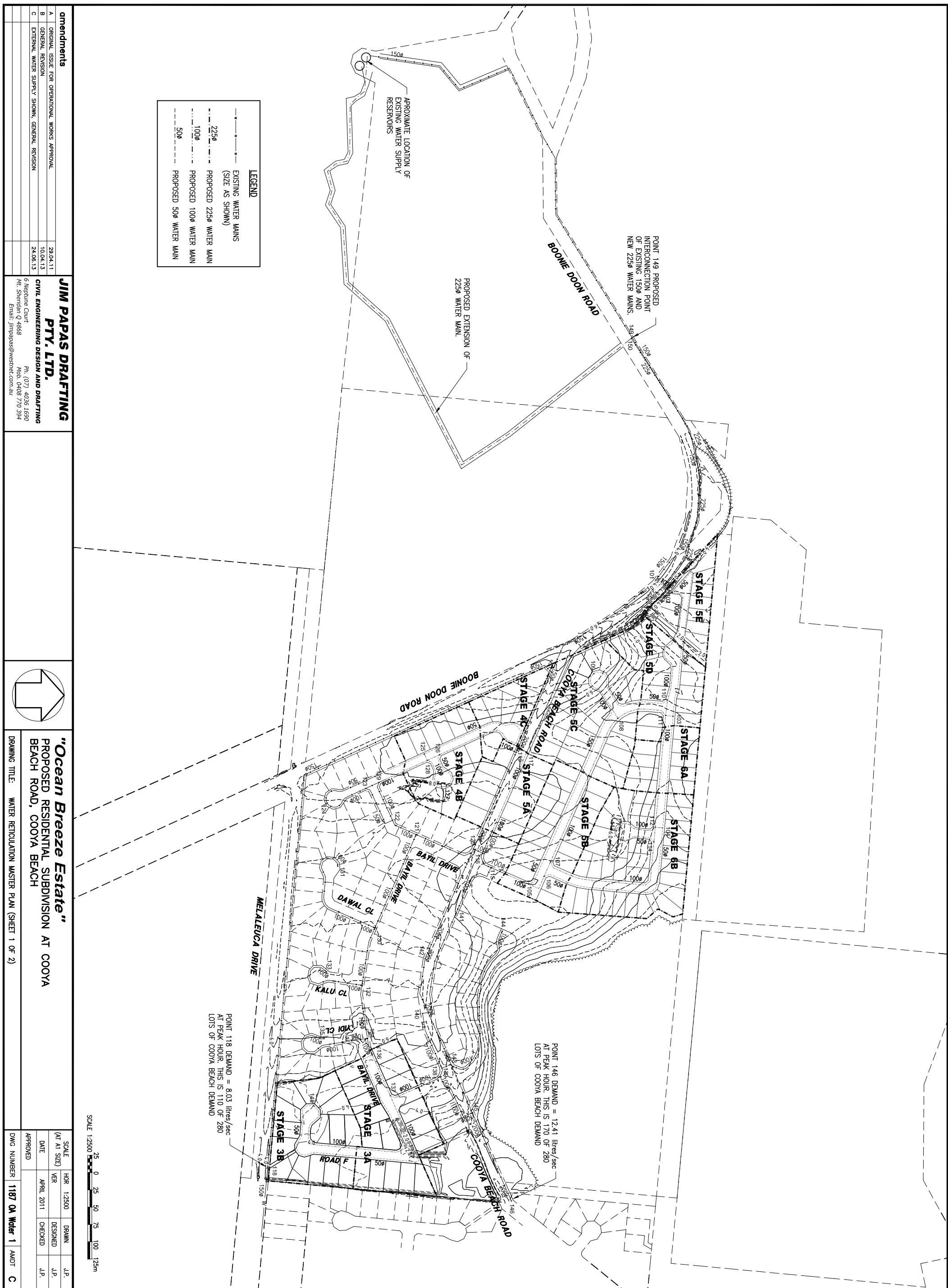
JIM PAPAS DRAFTING

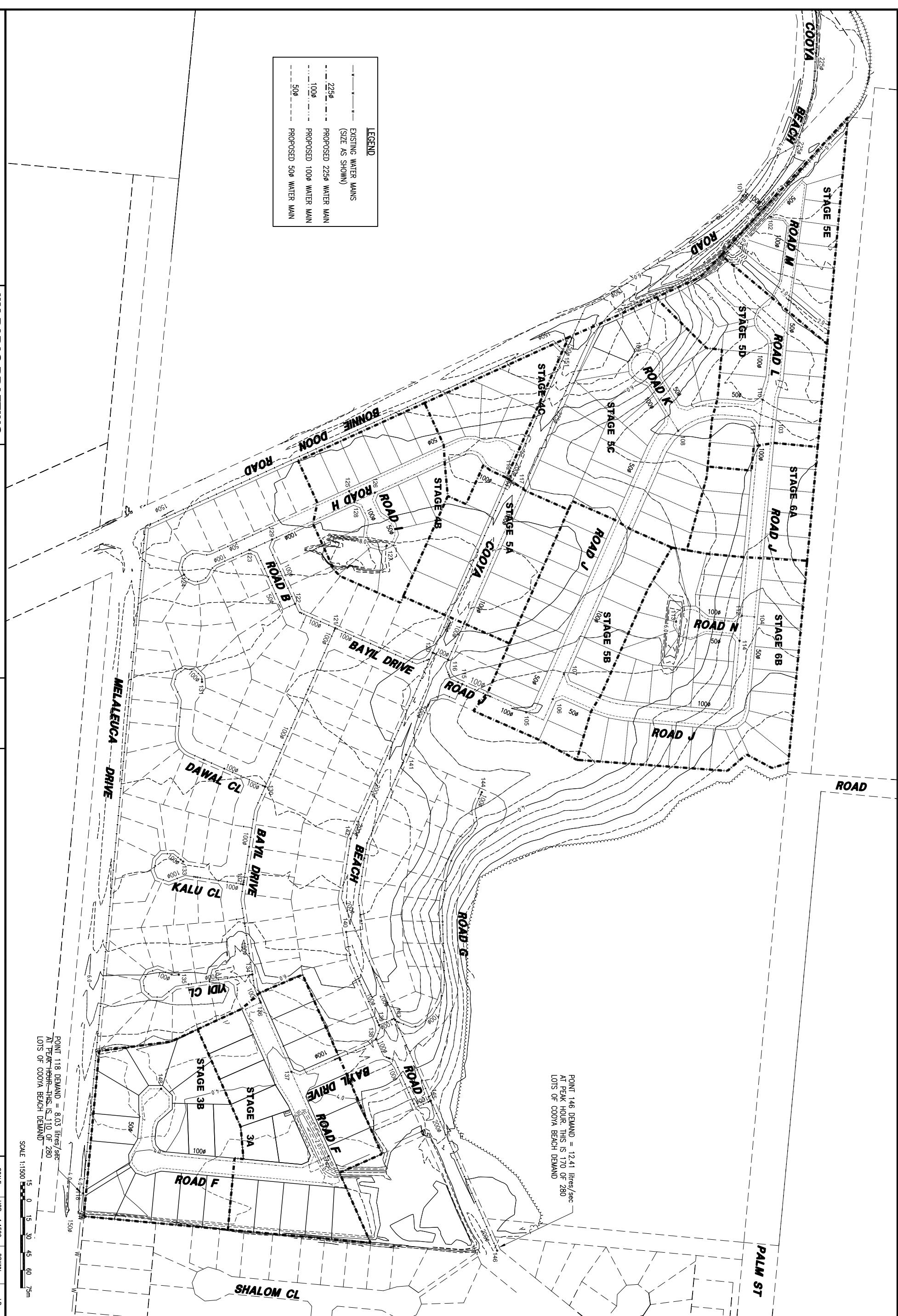
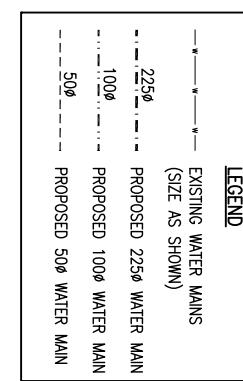
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APPENDIX B

WATER RETICULATION MASTER PLAN





"Ocean Breeze Estate" PROPOSED RESIDENTIAL SUBDIVISION AT COOYA BEACH ROAD, COOYA BEACH	
6 Neptune Court Mt. Sheridan Q 4868 Ph. (07) 4036 1690 Mob. 048 770 394 Email: jimpapas@westnet.com.au	
DRAWING TITLE:	WATER RETICULATION MASTER PLAN (SHEET 2 OF 2)
SCALE:	1:1500

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APPENDIX C

MODEL RESULTS

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*****
*          E P A N E T
*          Hydraulic and Water Quality
*          Analysis for Pipe Networks
*          Version 2.0
*****
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Input File: 1253-01 Scenario 1 Existing PH Flows 150 Only.NET

Scenario 1

Existing development at peak hour flow with reservoir level at RL 61.47m and with the sole supply line being the existing 1500 water main in Boonie Doon Road.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3
39	132	134	79	104.3
40	134	135	145	104.3
41	134	136	25	104.3
45	138	139	13	104.3
46	138	145	85	104.3
47	139	140	86	104.3
48	139	143	15	104.3
49	140	141	173	104.3
50	141	115	90	104.3
51	142	143	167	202.2
52	143	144	241	104.3
53	143	145	76	202.2
54	145	146	149	202.2
21	117	119	15	225
22	101	151	210	152
59	151	119	96	152
60	151	118	982	152
61	50	150	1000	225
62	150	149	5	225
63	149	50	671	152

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Link ID	Start Node	End Node	Length m	Diameter mm
64	149	101	404	152.2

66	1253-01	Scenario 1	Existing	PH Flows	150 Only	
67	150	101		404	225	
68	101	151		205	225	
	151	117		136	225	

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality	
101	0.00	32.08	28.58	0.00	
115	0.07	24.24	19.34	0.00	
116	0.49	24.29	18.69	0.00	
117	0.57	24.81	18.21	0.00	
119	0.54	24.82	19.52	0.00	
120	0.39	24.29	18.49	0.00	
121	0.15	23.57	16.67	0.00	
122	0.15	23.55	16.65	0.00	
123	0.27	23.51	17.21	0.00	
124	0.73	23.50	16.80	0.00	
129	0.07	23.52	17.32	0.00	
130	0.51	22.92	15.87	0.00	
131	0.51	22.90	15.90	0.00	
132	0.37	22.71	15.71	0.00	
133	0.80	22.68	15.58	0.00	
134	0.22	22.64	17.34	0.00	
135	0.95	22.59	16.59	0.00	
136	0.49	22.64	18.04	0.00	
138	0.22	23.81	18.61	0.00	
139	0.16	23.82	18.52	0.00	
140	0.32	23.89	17.09	0.00	
141	0.41	24.09	18.19	0.00	
142	0.56	24.04	16.94	0.00	
143	0.31	23.82	18.42	0.00	
144	0.58	23.79	18.59	0.00	
145	0.69	23.74	20.34	0.00	
146	12.41	23.55	20.55	0.00	
151	0.00	26.32	18.32	0.00	
118	8.03	24.12	20.02	0.00	
150	0.00	43.08	31.08	0.00	
149	0.00	43.08	31.08	0.00	
50	-30.97	61.47	0.00	0.00	Reservoir

♀

Page 3
Link Results:

Scenario 1

Link ID	Flow LPS	Velocity Uni m/s	Headloss m/km	Status
18	-2.36	0.28	1.75	Open
19	-2.65	0.31	2.16	Open
20	-0.20	0.02	0.02	Open
23	19.18	0.60	2.81	Open
25	5.22	0.61	7.57	Open
26	13.37	0.42	1.44	Open
27	1.22	0.14	0.51	Open
28	3.85	0.45	4.31	Open
29	1.07	0.13	0.40	Open
30	1.00	0.12	0.35	Open
31	0.73	0.09	0.20	Open
36	0.51	0.06	0.10	Open
37	2.83	0.33	2.44	Open

	1253-01 Scenario	1 Existing	PH Flows	150 Only
38	0.80	0.09	0.23	Open
39	1.66	0.19	0.91	Open
40	0.95	0.11	0.32	Open
41	0.49	0.06	0.09	Open
45	-1.78	0.21	1.04	Open
46	1.56	0.18	0.81	Open
47	-1.56	0.18	0.81	Open
48	-0.38	0.04	0.06	Open
49	-1.88	0.22	1.15	Open
50	-2.29	0.27	1.65	Open
51	12.81	0.40	1.33	Open
52	0.58	0.07	0.13	Open
53	11.54	0.36	1.10	Open
54	12.41	0.39	1.25	Open
21	-3.22	0.08	0.06	Open
22	30.97	1.71	27.41	Open
59	22.94	1.26	15.72	Open
60	8.03	0.44	2.25	Open
61	0.00	0.00	0.00	Closed
62	0.00	0.00	0.00	Open
63	-30.97	1.71	27.41	Open
64	30.97	1.70	27.23	Open
66	0.00	0.00	0.00	Closed
67	0.00	0.00	0.00	Closed
68	0.00	0.00	0.00	Closed

1253-02 Scenario 2 Existing PH Flows 225 + 150 Mains
 Page 1 27-Jun-13 3:59:03 PM

 * E P A N E T *
 * Hydraulic and Water Quality *
 * Analysis for Pipe Networks *
 * Version 2.0 *

Input File: 1253-02 Scenario 2 Existing PH Flows 225 + 150 Mains.NET

Scenario 2

Existing development at peak hour flow with reservoir level at RL 61.47m and with a single 1500 supply line from the reservoir to Point A as shown on Dwg 1187 - OA Water and 1500 augmented by a 2250 supply lines from this point to the intersection of Bonnie Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3
39	132	134	79	104.3
40	134	135	145	104.3
41	134	136	25	104.3
45	138	139	13	104.3
46	138	145	85	104.3
47	139	140	86	104.3
48	139	143	15	104.3
49	140	141	173	104.3
50	141	115	90	104.3
51	142	143	167	202.2
52	143	144	241	104.3
53	143	145	76	202.2
54	145	146	149	202.2
21	117	119	15	225
22	101	151	210	152
59	151	119	96	152
60	151	118	982	152
61	50	150	1000	225
62	150	149	5	225

♀

Page 2 Scenario 2
 Link - Node Table: (continued)

Link ID	Start Node	End Node	Length m	Diameter mm
63	149	50	671	152

64	1253-02	Scenario 2	Existing	PH Flows	225	+	150	Mains
66	149		101		404		152.	2
67	150		101		404		225	
68	101		151		205		225	
	151		117		136		225	

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0. 00	42. 14	38. 64	0. 00
115	0. 07	40. 92	36. 02	0. 00
116	0. 49	40. 97	35. 37	0. 00
117	0. 57	41. 51	34. 91	0. 00
119	0. 54	41. 50	36. 20	0. 00
120	0. 39	40. 97	35. 17	0. 00
121	0. 15	40. 25	33. 35	0. 00
122	0. 15	40. 23	33. 33	0. 00
123	0. 27	40. 20	33. 90	0. 00
124	0. 73	40. 18	33. 48	0. 00
129	0. 07	40. 20	34. 00	0. 00
130	0. 51	39. 60	32. 55	0. 00
131	0. 51	39. 59	32. 59	0. 00
132	0. 37	39. 39	32. 39	0. 00
133	0. 80	39. 36	32. 26	0. 00
134	0. 22	39. 32	34. 02	0. 00
135	0. 95	39. 27	33. 27	0. 00
136	0. 49	39. 32	34. 72	0. 00
138	0. 22	40. 49	35. 29	0. 00
139	0. 16	40. 50	35. 20	0. 00
140	0. 32	40. 57	33. 77	0. 00
141	0. 41	40. 77	34. 87	0. 00
142	0. 56	40. 72	33. 62	0. 00
143	0. 31	40. 50	35. 10	0. 00
144	0. 58	40. 47	35. 27	0. 00
145	0. 69	40. 42	37. 02	0. 00
146	12. 41	40. 23	37. 23	0. 00
151	0. 00	41. 67	33. 67	0. 00
118	8. 03	39. 46	35. 36	0. 00
150	0. 00	43. 07	31. 07	0. 00
149	0. 00	43. 08	31. 08	0. 00
50	-30. 97	61. 47	0. 00	0. 00 Reservoir

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Link Results:

Scenario 2

Link ID	Flow LPS	Velocity Uni m/s	Headloss m/km	Status
18	-2. 36	0. 28	1. 75	Open
19	-2. 68	0. 31	2. 21	Open
20	-0. 17	0. 02	0. 01	Open
23	19. 15	0. 60	2. 80	Open
25	5. 22	0. 61	7. 57	Open
26	13. 37	0. 42	1. 44	Open
27	1. 22	0. 14	0. 51	Open
28	3. 85	0. 45	4. 31	Open
29	1. 07	0. 13	0. 40	Open
30	1. 00	0. 12	0. 35	Open
31	0. 73	0. 09	0. 20	Open
36	0. 51	0. 06	0. 10	Open

	1253-02	Scenario 2 Existing	PH Flows	225 + 150 Mais
37	2.83	0.33	2.44	Open
38	0.80	0.09	0.23	Open
39	1.66	0.19	0.91	Open
40	0.95	0.11	0.32	Open
41	0.49	0.06	0.09	Open
45	-1.78	0.21	1.04	Open
46	1.56	0.18	0.81	Open
47	-1.56	0.18	0.81	Open
48	-0.38	0.04	0.06	Open
49	-1.88	0.22	1.15	Open
50	-2.29	0.27	1.65	Open
51	12.81	0.40	1.33	Open
52	0.58	0.07	0.13	Open
53	11.54	0.36	1.10	Open
54	12.41	0.39	1.25	Open
21	12.60	0.32	0.77	Open
22	8.06	0.44	2.27	Open
59	7.08	0.39	1.78	Open
60	8.03	0.44	2.25	Open
61	0.00	0.00	0.00	Closed
62	-22.77	0.57	2.29	Open
63	-30.97	1.71	27.41	Open
64	8.20	0.45	2.32	Open
66	22.77	0.57	2.29	Open
67	22.91	0.58	2.32	Open
68	15.86	0.40	1.17	Open

1253-03 Scenario 3 Existing PH Flows 225 + 150 Mains
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Input File: 1253-03 Scenario 3 Existing PH Flows 225 + 150 Mains.NET

Scenario 3

Existing development at peak hour flow with reservoir level at RL 61.47m and with a 150Ø a 225Ø supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3
39	132	134	79	104.3
40	134	135	145	104.3
41	134	136	25	104.3
45	138	139	13	104.3
46	138	145	85	104.3
47	139	140	86	104.3
48	139	143	15	104.3
49	140	141	173	104.3
50	141	115	90	104.3
51	142	143	167	202.2
52	143	144	241	104.3
53	143	145	76	202.2
54	145	146	149	202.2
21	117	119	15	225
22	101	151	210	152
59	151	119	96	152
60	151	118	982	152
61	50	150	1000	225
62	150	149	5	225
63	149	50	671	152

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Page 2 Scenario 3
 Link - Node Table: (continued)

Link ID	Start Node	End Node	Length m	Diameter mm
64	149	101	404	152.2

66	1253-03	Scenario 3	Existing	PH Flows	225	+	150	Mains
67			150	101		404	225	
68			101	151		205	225	
			151	117		136	225	

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0.00	58.48	54.98	0.00
115	0.07	57.26	52.36	0.00
116	0.49	57.31	51.71	0.00
117	0.57	57.85	51.25	0.00
119	0.54	57.83	52.53	0.00
120	0.39	57.31	51.51	0.00
121	0.15	56.59	49.69	0.00
122	0.15	56.57	49.67	0.00
123	0.27	56.53	50.23	0.00
124	0.73	56.52	49.82	0.00
129	0.07	56.54	50.34	0.00
130	0.51	55.94	48.89	0.00
131	0.51	55.92	48.92	0.00
132	0.37	55.73	48.73	0.00
133	0.80	55.70	48.60	0.00
134	0.22	55.66	50.36	0.00
135	0.95	55.61	49.61	0.00
136	0.49	55.66	51.06	0.00
138	0.22	56.83	51.63	0.00
139	0.16	56.84	51.54	0.00
140	0.32	56.91	50.11	0.00
141	0.41	57.11	51.21	0.00
142	0.56	57.06	49.96	0.00
143	0.31	56.84	51.44	0.00
144	0.58	56.81	51.61	0.00
145	0.69	56.76	53.36	0.00
146	12.41	56.57	53.57	0.00
151	0.00	58.01	50.01	0.00
118	8.03	55.80	51.70	0.00
150	0.00	59.41	47.41	0.00
149	0.00	59.41	47.41	0.00
50	-30.97	61.47	0.00	Reservoir

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Link Results:

Scenario 3

Link ID	Flow LPS	Velocity Uni m/s	Headloss m/km	Status
18	-2.36	0.28	1.75	Open
19	-2.68	0.31	2.21	Open
20	-0.17	0.02	0.01	Open
23	19.15	0.60	2.80	Open
25	5.22	0.61	7.57	Open
26	13.37	0.42	1.44	Open
27	1.22	0.14	0.51	Open
28	3.85	0.45	4.31	Open
29	1.07	0.13	0.40	Open
30	1.00	0.12	0.35	Open
31	0.73	0.09	0.20	Open
36	0.51	0.06	0.10	Open
37	2.83	0.33	2.44	Open

	1253-03	Scenario 3	Existing	PH Flows	225 + 150	Mains
38		0. 80	0. 09	0. 23	Open	
39		1. 66	0. 19	0. 91	Open	
40		0. 95	0. 11	0. 32	Open	
41		0. 49	0. 06	0. 09	Open	
45		-1. 78	0. 21	1. 04	Open	
46		1. 56	0. 18	0. 81	Open	
47		-1. 56	0. 18	0. 81	Open	
48		-0. 38	0. 04	0. 06	Open	
49		-1. 88	0. 22	1. 15	Open	
50		-2. 29	0. 27	1. 65	Open	
51		12. 81	0. 40	1. 33	Open	
52		0. 58	0. 07	0. 13	Open	
53		11. 54	0. 36	1. 10	Open	
54		12. 41	0. 39	1. 25	Open	
21		12. 60	0. 32	0. 77	Open	
22		8. 06	0. 44	2. 27	Open	
59		7. 08	0. 39	1. 78	Open	
60		8. 03	0. 44	2. 25	Open	
61		21. 48	0. 54	2. 06	Open	
62		-1. 34	0. 03	0. 01	Open	
63		-9. 49	0. 52	3. 07	Open	
64		8. 16	0. 45	2. 30	Open	
66		22. 81	0. 57	2. 30	Open	
67		22. 91	0. 58	2. 32	Open	
68		15. 86	0. 40	1. 17	Open	

1253-04 Scenario 4 Full Dev PH Flows 225 + 150 Mains
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Input File: 1253-04 Scenario 4 Full Dev PH Flows 225 + 150 Mains.net

Scenario 4

Full development at peak hour flow with reservoir level at RL 61.47m with the sole supply line being the 2250 water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
2	101	102	30	104.3
3	102	110	183	104.3
5	110	111	23	104.3
6	111	103	11	104.3
7	111	108	77	104.3
8	111	112	165	104.3
9	112	113	67	104.3
10	112	114	22	104.3
11	112	104	11	104.3
12	114	107	216	104.3
13	107	106	24	104.3
14	106	105	36	104.3
15	107	108	233	104.3
16	108	109	83	104.3
17	105	115	60	104.3
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
24	119	126	154	104.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
32	126	128	20	104.3
33	128	125	10	104.3
34	128	127	57	104.3
35	128	129	78	104.3
36	130	131	175	104.3
37	130	132	87	104.3

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Page 2 Scenario 4
 Link - Node Table: (continued)

Link ID	Start Node	End Node	Length m	Diameter mm
38	132	133	121	104.3

	1253-04	Scenario 4	Full Dev PH Flows	225 + 150	Mains
39	132	134		79	104.3
40	134	135		145	104.3
41	134	136		25	104.3
42	136	137		65	104.3
43	137	147		80	104.3
44	137	138		81	104.3
45	138	139		13	104.3
46	138	145		85	104.3
47	139	140		86	104.3
48	139	143		15	104.3
49	140	141		173	104.3
50	141	115		90	104.3
51	142	143		167	202.2
52	143	144		241	104.3
53	143	145		76	202.2
54	145	146		149	202.2
55	147	148		216	104.3
56	109	110		157	48.4
57	105	108		288	48.4
21	117	119		15	225
22	101	151		210	152
59	151	119		96	152
60	151	118		982	152
61	50	150		1000	225
62	150	149		5	225
63	149	50		671	152
64	149	101		404	152.2
66	150	101		404	225
67	101	151		205	225
68	151	117		136	225

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0.00	51.63	48.13	0.00
102	0.66	51.32	47.92	0.00
103	0.66	49.70	45.90	0.00
104	0.66	49.55	46.15	0.00
105	0.80	49.56	45.46	0.00
106	0.73	49.56	45.46	0.00
107	0.44	49.56	45.16	0.00
108	0.44	49.61	45.61	0.00
109	0.80	49.61	42.51	0.00
110	0.58	49.84	46.44	0.00

♀

Page 3
Node Results: (continued)

Scenario 4

Node ID	Demand LPS	Head m	Pressure m	Quality
111	0.37	49.70	45.90	0.00
112	0.29	49.55	46.15	0.00
113	0.44	49.55	44.35	0.00
114	0.51	49.55	46.15	0.00
115	0.07	49.60	44.70	0.00
116	0.49	49.72	44.12	0.00
117	0.57	50.33	43.73	0.00
119	0.54	50.31	45.01	0.00
120	0.39	49.73	43.93	0.00

	1253-04	Scenario 4	Full Dev PH Flows	225 + 150 Mains
121		0.15	49.55	42.65 0.00
122		0.15	49.57	42.67 0.00
123		0.27	49.59	43.29 0.00
124		0.73	49.58	42.88 0.00
125		0.51	49.72	44.02 0.00
126		0.29	49.78	44.08 0.00
127		0.29	49.72	43.32 0.00
128		0.07	49.72	43.92 0.00
129		0.07	49.60	43.40 0.00
130		0.51	49.05	42.00 0.00
131		0.51	49.03	42.03 0.00
132		0.37	48.90	41.90 0.00
133		0.80	48.87	41.77 0.00
134		0.22	48.86	43.56 0.00
135		0.95	48.81	42.81 0.00
136		0.49	48.86	44.26 0.00
137		0.08	48.86	44.96 0.00
138		0.22	49.03	43.83 0.00
139		0.16	49.08	43.78 0.00
140		0.32	49.17	42.37 0.00
141		0.41	49.42	43.52 0.00
142		0.56	49.40	42.30 0.00
143		0.31	49.10	43.70 0.00
144		0.58	49.07	43.87 0.00
145		0.69	49.01	45.61 0.00
146	12.41	48.82	45.82	0.00
147	0.81	48.76	45.36	0.00
148	1.22	48.65	43.55	0.00
151	0.00	50.55	42.55	0.00
118	8.03	48.34	44.24	0.00
150	0.00	54.46	42.46	0.00
149	0.00	55.85	43.85	0.00
50	-41.62	61.47	0.00	0.00 Reservoir

♀

Page 4
Link Results:

Scenario 4

Link ID	Flow LPS	Velocity m/s	Headloss m/km	Status
2	6.08	0.71	10.05	Open
3	5.42	0.63	8.13	Open
5	4.56	0.53	5.89	Open
6	0.66	0.08	0.16	Open
7	1.87	0.22	1.13	Open
8	1.66	0.19	0.91	Open
9	0.44	0.05	0.08	Open
10	0.27	0.03	0.03	Open
11	0.66	0.08	0.16	Open
12	-0.24	0.03	0.03	Open
13	0.14	0.02	0.01	Open
14	-0.59	0.07	0.13	Open
15	-0.82	0.10	0.25	Open
16	0.51	0.06	0.10	Open
17	-1.30	0.15	0.57	Open
18	-3.89	0.46	4.40	Open
19	-2.86	0.33	2.48	Open
20	-1.52	0.18	0.77	Open
23	20.13	0.63	3.07	Open
24	3.41	0.40	3.46	Open
25	2.49	0.29	1.93	Open

	1253-04	Scenario 4	Full Dev PH Flows	225 + 150 Mains
26	15.72	0.49	1.94	Open
27	-1.03	0.12	0.38	Open
28	3.37	0.39	3.37	Open
29	-1.18	0.14	0.48	Open
30	1.00	0.12	0.35	Open
31	0.73	0.09	0.20	Open
32	3.12	0.37	2.92	Open
33	0.51	0.06	0.10	Open
34	0.29	0.03	0.04	Open
35	2.25	0.26	1.59	Open
36	0.51	0.06	0.10	Open
37	2.35	0.28	1.73	Open
38	0.80	0.09	0.23	Open
39	1.18	0.14	0.48	Open
40	0.95	0.11	0.32	Open
41	0.01	0.00	0.00	Open
42	-0.48	0.06	0.09	Open
43	2.03	0.24	1.32	Open
44	-2.59	0.30	2.06	Open
45	-3.66	0.43	3.92	Open
46	0.85	0.10	0.26	Open
47	-1.80	0.21	1.05	Open
48	-2.02	0.24	1.31	Open
49	-2.12	0.25	1.42	Open
50	-2.53	0.30	1.98	Open
51	15.16	0.47	1.82	Open

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Page 5
Link Results: (continued)

Scenario 4

Link ID	Flow LPS	Velocity Uni m/s	Headloss m/km	Status
52	0.58	0.07	0.13	Open
53	12.25	0.38	1.22	Open
54	12.41	0.39	1.25	Open
55	1.22	0.14	0.51	Open
56	-0.29	0.16	1.47	Open
57	-0.09	0.05	0.18	Open
21	15.57	0.39	1.14	Open
22	0.00	0.00	0.00	Closed
59	8.51	0.47	2.50	Open
60	8.03	0.44	2.25	Open
61	41.62	1.05	7.01	Open
62	0.00	0.00	0.00	Closed
63	0.00	0.00	0.00	Closed
64	0.00	0.00	0.00	Closed
66	41.62	1.05	7.01	Open
67	35.54	0.89	5.23	Open
68	19.00	0.48	1.64	Open

1253-05 Scenario 5 Full Dev PH Flows Max. Pressure
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Input File: 1253-05 Scenario 5 Full Dev PH Flows Max. Pressure.NET

Scenario 5

Full development at peak hour flow with reservoir level at RL 68.88m with a 1500 and a 2250 supply lines from the reservoir to the intersection of Bonnike Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
2	101	102	30	104.3
3	102	110	183	104.3
5	110	111	23	104.3
6	111	103	11	104.3
7	111	108	77	104.3
8	111	112	165	104.3
9	112	113	67	104.3
10	112	114	22	104.3
11	112	104	11	104.3
12	114	107	216	104.3
13	107	106	24	104.3
14	106	105	36	104.3
15	107	108	233	104.3
16	108	109	83	104.3
17	105	115	60	104.3
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
24	119	126	154	104.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
32	126	128	20	104.3
33	128	125	10	104.3
34	128	127	57	104.3
35	128	129	78	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3

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Page 2 Scenario 5
 Link - Node Table: (continued)

Link ID	Start Node	End Node	Length m	Diameter mm
39	132	134	79	104.3

	1253-05	Scenario 5	Full Dev PH Flows	Max.	Pressure
40	134	135		145	104.3
41	134	136		25	104.3
42	136	137		65	104.3
43	137	147		80	104.3
44	137	138		81	104.3
45	138	139		13	104.3
46	138	145		85	104.3
47	139	140		86	104.3
48	139	143		15	104.3
49	140	141		173	104.3
50	141	115		90	104.3
51	142	143		167	202.2
52	143	144		241	104.3
53	143	145		76	202.2
54	145	146		149	202.2
55	147	148		216	104.3
56	109	110		157	48.4
57	105	108		288	48.4
21	117	119		15	225
22	101	151		210	152
59	151	119		96	152
60	151	118		982	152
61	50	150		1000	225
62	150	149		5	225
63	149	50		671	152
64	149	101		404	152.2
66	150	101		404	225
67	101	151		205	225
68	151	117		136	225

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0.00	59.04	55.54	0.00
102	0.66	58.73	55.33	0.00
103	0.66	57.11	53.31	0.00
104	0.66	56.96	53.56	0.00
105	0.80	56.97	52.87	0.00
106	0.73	56.97	52.87	0.00
107	0.44	56.97	52.57	0.00
108	0.44	57.02	53.02	0.00
109	0.80	57.02	49.92	0.00
110	0.58	57.25	53.85	0.00
111	0.37	57.11	53.31	0.00

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Page 3
Node Results: (continued)

Scenario 5

Node ID	Demand LPS	Head m	Pressure m	Quality
112	0.29	56.96	53.56	0.00
113	0.44	56.96	51.76	0.00
114	0.51	56.96	53.56	0.00
115	0.07	57.01	52.11	0.00
116	0.49	57.13	51.53	0.00
117	0.57	57.74	51.14	0.00
119	0.54	57.72	52.42	0.00
120	0.39	57.14	51.34	0.00
121	0.15	56.96	50.06	0.00

	1253-05	Scenario 5	Full Dev PH Flows	Max. Pressure
122	0.15	56.98	50.08	0.00
123	0.27	57.00	50.70	0.00
124	0.73	56.99	50.29	0.00
125	0.51	57.13	51.43	0.00
126	0.29	57.19	51.49	0.00
127	0.29	57.13	50.73	0.00
128	0.07	57.13	51.33	0.00
129	0.07	57.01	50.81	0.00
130	0.51	56.46	49.41	0.00
131	0.51	56.44	49.44	0.00
132	0.37	56.31	49.31	0.00
133	0.80	56.28	49.18	0.00
134	0.22	56.27	50.97	0.00
135	0.95	56.22	50.22	0.00
136	0.49	56.27	51.67	0.00
137	0.08	56.27	52.37	0.00
138	0.22	56.44	51.24	0.00
139	0.16	56.49	51.19	0.00
140	0.32	56.58	49.78	0.00
141	0.41	56.83	50.93	0.00
142	0.56	56.81	49.71	0.00
143	0.31	56.51	51.11	0.00
144	0.58	56.48	51.28	0.00
145	0.69	56.42	53.02	0.00
146	12.41	56.23	53.23	0.00
147	0.81	56.17	52.77	0.00
148	1.22	56.06	50.96	0.00
151	0.00	57.96	49.96	0.00
118	8.03	55.75	51.65	0.00
150	0.00	61.87	49.87	0.00
149	0.00	63.26	51.26	0.00
50	-41.62	68.88	0.00	0.00 Reservoir

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Page 4
Link Results:

Scenario 5

Link ID	Flow LPS	Velocity m/s	Headloss m/km	Status
2	6.08	0.71	10.05	Open
3	5.42	0.63	8.13	Open
5	4.56	0.53	5.89	Open
6	0.66	0.08	0.16	Open
7	1.87	0.22	1.13	Open
8	1.66	0.19	0.91	Open
9	0.44	0.05	0.08	Open
10	0.27	0.03	0.03	Open
11	0.66	0.08	0.16	Open
12	-0.24	0.03	0.03	Open
13	0.14	0.02	0.01	Open
14	-0.59	0.07	0.13	Open
15	-0.82	0.10	0.25	Open
16	0.51	0.06	0.10	Open
17	-1.30	0.15	0.57	Open
18	-3.89	0.46	4.40	Open
19	-2.86	0.33	2.48	Open
20	-1.52	0.18	0.77	Open
23	20.13	0.63	3.07	Open
24	3.41	0.40	3.46	Open
25	2.49	0.29	1.93	Open
26	15.72	0.49	1.94	Open

	1253-05	Scenario 5	Full Dev PH Flows	Max.	Pressure
27	-1.03	0.12	0.38	Open	
28	3.37	0.39	3.37	Open	
29	-1.18	0.14	0.48	Open	
30	1.00	0.12	0.35	Open	
31	0.73	0.09	0.20	Open	
32	3.12	0.37	2.92	Open	
33	0.51	0.06	0.10	Open	
34	0.29	0.03	0.04	Open	
35	2.25	0.26	1.59	Open	
36	0.51	0.06	0.10	Open	
37	2.35	0.28	1.73	Open	
38	0.80	0.09	0.23	Open	
39	1.18	0.14	0.48	Open	
40	0.95	0.11	0.32	Open	
41	0.01	0.00	0.00	Open	
42	-0.48	0.06	0.09	Open	
43	2.03	0.24	1.32	Open	
44	-2.59	0.30	2.06	Open	
45	-3.66	0.43	3.92	Open	
46	0.85	0.10	0.26	Open	
47	-1.80	0.21	1.05	Open	
48	-2.02	0.24	1.31	Open	
49	-2.12	0.25	1.42	Open	
50	-2.53	0.30	1.98	Open	
51	15.16	0.47	1.82	Open	

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Link Results: (continued)

Scenario 5

Link ID	Flow LPS	Velocity Unit m/s	Headloss m/km	Status
52	0.58	0.07	0.13	Open
53	12.25	0.38	1.22	Open
54	12.41	0.39	1.25	Open
55	1.22	0.14	0.51	Open
56	-0.29	0.16	1.47	Open
57	-0.09	0.05	0.18	Open
21	15.57	0.39	1.14	Open
22	0.00	0.00	0.00	Closed
59	8.51	0.47	2.50	Open
60	8.03	0.44	2.25	Open
61	41.62	1.05	7.01	Open
62	0.00	0.00	0.00	Closed
63	0.00	0.00	0.00	Closed
64	0.00	0.00	0.00	Closed
66	41.62	1.05	7.01	Open
67	35.54	0.89	5.23	Open
68	19.00	0.48	1.64	Open

1253-06 Scenario 6 FFF + 0.667 Exist PH Flows
 Page 1 28-Jun-13 1:54:28 AM

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Input File: 1253-06 Scenario 6 FFF + 0.667 Exist PH Flows.NET

Scenario 6

Firefighting flows imposed on 2/3 of existing peak hour flows with reservoir level at RL 61.47m and with the sole supply line being the existing 1500 water main in Boone Doon Road.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3
39	132	134	79	104.3
40	134	135	145	104.3
41	134	136	25	104.3
45	138	139	13	104.3
46	138	145	85	104.3
47	139	140	86	104.3
48	139	143	15	104.3
49	140	141	173	104.3
50	141	115	90	104.3
51	142	143	167	202.2
52	143	144	241	104.3
53	143	145	76	202.2
54	145	146	149	202.2
21	117	119	15	225
22	101	151	210	152
59	151	119	96	152
60	151	118	982	152
61	50	150	1000	225
62	150	149	5	225
63	149	50	671	152

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Page 2 Scenario 6
 Link - Node Table: (continued)

Link ID	Start Node	End Node	Length m	Diameter mm
64	149	101	404	152.2

1253-06 Scenario 6 FFF + 0.667 Exist PH Flows
 66 150 101 404 225
 67 101 151 205 225
 68 151 117 136 225

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0.00	23.31	19.81	0.00
115	0.05	12.38	7.48	0.00
116	0.33	12.40	6.80	0.00
117	0.38	13.31	6.71	0.00
119	0.36	13.32	8.02	0.00
120	0.26	12.39	6.59	0.00
121	0.10	4.91	-1.99	0.00
122	0.10	4.90	-2.00	0.00
123	0.18	4.88	-1.42	0.00
124	0.49	4.88	-1.82	0.00
129	0.05	4.89	-1.31	0.00
130	0.34	-5.84	-12.89	0.00
131	15.34	-15.59	-22.59	0.00
132	0.25	-5.94	-12.94	0.00
133	0.53	-5.95	-13.05	0.00
134	0.15	-5.97	-11.27	0.00
135	0.63	-5.99	-11.99	0.00
136	0.33	-5.97	-10.57	0.00
138	0.15	12.16	6.96	0.00
139	0.11	12.17	6.87	0.00
140	0.21	12.20	5.40	0.00
141	0.27	12.30	6.40	0.00
142	0.37	12.27	5.17	0.00
143	0.21	12.17	6.77	0.00
144	0.39	12.15	6.95	0.00
145	0.46	12.13	8.73	0.00
146	8.28	12.04	9.04	0.00
151	0.00	15.84	7.84	0.00
118	5.36	14.80	10.70	0.00
150	0.00	40.79	28.79	0.00
149	0.00	37.60	25.60	0.00
50	-35.66	61.47	0.00	Reservoir

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Page 3
 Link Results:

Scenario 6

Link ID	Flow LPS	Velocity Uni m/s	Headloss m/km	Status
18	-1.61	0.19	0.86	Open
19	-3.57	0.42	3.74	Open
20	1.63	0.19	0.88	Open
23	25.99	0.81	4.93	Open
25	18.48	2.16	78.71	Open
26	8.88	0.28	0.67	Open
27	0.81	0.10	0.24	Open
28	17.57	2.06	71.65	Open
29	0.71	0.08	0.19	Open
30	0.67	0.08	0.17	Open
31	0.49	0.06	0.09	Open
36	15.34	1.80	55.74	Open
37	1.89	0.22	1.15	Open

	1253-06 Scenario	FFF	+	0.667 Exist	PH Flows
38	0.53	0.06		0.11	Open
39	1.11	0.13		0.43	Open
40	0.63	0.07		0.15	Open
41	0.33	0.04		0.04	Open
45	-1.19	0.14		0.49	Open
46	1.04	0.12		0.38	Open
47	-1.08	0.13		0.41	Open
48	-0.22	0.03		0.02	Open
49	-1.29	0.15		0.57	Open
50	-1.57	0.18		0.81	Open
51	8.51	0.26		0.62	Open
52	0.39	0.05		0.06	Open
53	7.69	0.24		0.52	Open
54	8.28	0.26		0.59	Open
21	-3.95	0.10		0.09	Open
22	35.66	1.97		35.58	Open
59	30.30	1.67		26.32	Open
60	5.36	0.30		1.06	Open
61	0.00	0.00		0.00	Closed
62	0.00	0.00		0.00	Closed
63	-35.66	1.97		35.58	Open
64	35.66	1.96		35.35	Open
66	0.00	0.00		0.00	Closed
67	0.00	0.00		0.00	Closed
68	0.00	0.00		0.00	Closed

1253-07 Scenario 7 FFF + 0.667 Exist PH Flows 225 + 150 Mains
 Page 1 28-Jun-13 2:02:20 AM

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Input File: 1253-07 Scenario 7 FFF + 0.667 Exist PH Flows 225 + 150 Mains.NET

Scenario 7

Firefighting flows imposed on 2/3 existing peak hour flows with reservoir level at RL 61.47m and with a single 1500 supply line from the reservoir to Point A as shown on Dwg 1187 - OA Water and 1500 augmented by a 2250 supply lines from this point to the intersection of Bonni e Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3
39	132	134	79	104.3
40	134	135	145	104.3
41	134	136	25	104.3
45	138	139	13	104.3
46	138	145	85	104.3
47	139	140	86	104.3
48	139	143	15	104.3
49	140	141	173	104.3
50	141	115	90	104.3
51	142	143	167	202.2
52	143	144	241	104.3
53	143	145	76	202.2
54	145	146	149	202.2
21	117	119	15	225
22	101	151	210	152
59	151	119	96	152
60	151	118	982	152
61	50	150	1000	225

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Link - Node Table: (continued)

Scenario 7

Link ID	Start Node	End Node	Length m	Diameter mm
62	150	149	5	225

63	149	50		671	152
64	149	101		404	152.2
66	150	101		404	225
67	101	151		205	225
68	151	117		136	225

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0.00	36.38	32.88	0.00
115	0.05	34.54	29.64	0.00
116	0.33	34.56	28.96	0.00
117	0.38	35.49	28.89	0.00
119	0.36	35.47	30.17	0.00
120	0.26	34.55	28.75	0.00
121	0.10	27.07	20.17	0.00
122	0.10	27.06	20.16	0.00
123	0.18	27.04	20.74	0.00
124	0.49	27.04	20.34	0.00
129	0.05	27.05	20.85	0.00
130	0.34	16.32	9.27	0.00
131	15.34	6.57	-0.43	0.00
132	0.25	16.22	9.22	0.00
133	0.53	16.21	9.11	0.00
134	0.15	16.19	10.89	0.00
135	0.63	16.17	10.17	0.00
136	0.33	16.19	11.59	0.00
138	0.15	34.32	29.12	0.00
139	0.11	34.33	29.03	0.00
140	0.21	34.36	27.56	0.00
141	0.27	34.46	28.56	0.00
142	0.37	34.43	27.33	0.00
143	0.21	34.33	28.93	0.00
144	0.39	34.31	29.11	0.00
145	0.46	34.29	30.89	0.00
146	8.28	34.20	31.20	0.00
151	0.00	35.76	27.76	0.00
118	5.36	34.72	30.62	0.00
150	0.00	37.58	25.58	0.00
149	0.00	37.60	25.60	0.00
50	-35.66	61.47	0.00	Reservoir

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Link Results:

Scenario 7

Link ID	Flow LPS	Velocity m/s	Headloss m/km	Status
18	-1.61	0.19	0.86	Open
19	-3.61	0.42	3.82	Open
20	1.67	0.19	0.91	Open
23	25.95	0.81	4.92	Open
25	18.48	2.16	78.71	Open
26	8.88	0.28	0.67	Open
27	0.81	0.10	0.24	Open
28	17.57	2.06	71.65	Open
29	0.71	0.08	0.19	Open
30	0.67	0.08	0.17	Open
31	0.49	0.06	0.09	Open

	1253-07 Scenario	7 FFF + 0.667	Exist	PH	Flows	225 + 150 Mains
36	15.34	1.80	55.74		Open	
37	1.89	0.22	1.15		Open	
38	0.53	0.06	0.11		Open	
39	1.11	0.13	0.43		Open	
40	0.63	0.07	0.15		Open	
41	0.33	0.04	0.04		Open	
45	-1.19	0.14	0.49		Open	
46	1.04	0.12	0.38		Open	
47	-1.08	0.13	0.41		Open	
48	-0.22	0.03	0.02		Open	
49	-1.29	0.15	0.57		Open	
50	-1.57	0.18	0.82		Open	
51	8.50	0.26	0.62		Open	
52	0.39	0.05	0.06		Open	
53	7.69	0.24	0.52		Open	
54	8.28	0.26	0.59		Open	
21	16.95	0.43	1.33		Open	
22	9.28	0.51	2.94		Open	
59	9.36	0.52	2.99		Open	
60	5.36	0.30	1.06		Open	
61	0.00	0.00	0.00		Closed	
62	-26.22	0.66	2.98		Open	
63	-35.66	1.97	35.58		Open	
64	9.44	0.52	3.02		Open	
66	26.22	0.66	2.98		Open	
67	26.38	0.66	3.01		Open	
68	20.94	0.53	1.96		Open	

1253-08 Scenario 8 FFF + 0.667 Exist PH Flows 225 + 150 Mains
 Page 1 28-Jun-13 2:09:42 AM

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Input File: 1253-08 Scenario 8 FFF + 0.667 Exist PH Flows 225 + 150 Mains.NET

Scenario 8

Firefighting flows imposed on 2/3 of existing peak hour flows with reservoir level at RL 61.47m with a 150Ø and a 225Ø supply lines from the reservoir to the intersection of Bonniedoon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3
39	132	134	79	104.3
40	134	135	145	104.3
41	134	136	25	104.3
45	138	139	13	104.3
46	138	145	85	104.3
47	139	140	86	104.3
48	139	143	15	104.3
49	140	141	173	104.3
50	141	115	90	104.3
51	142	143	167	202.2
52	143	144	241	104.3
53	143	145	76	202.2
54	145	146	149	202.2
21	117	119	15	225
22	101	151	210	152
59	151	119	96	152
60	151	118	982	152
61	50	150	1000	225
62	150	149	5	225
63	149	50	671	152

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Link - Node Table: (continued)

Scenario 8

Link ID	Start Node	End Node	Length m	Diameter mm
64	149	101	404	152.2

1253-08 Scenario 8 FFF + 0.667 Exist PH Flows 225 + 150 Mains

66	150	101	404	225
67	101	151	205	225
68	151	117	136	225

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0.00	57.59	54.09	0.00
115	0.05	55.75	50.85	0.00
116	0.33	55.77	50.17	0.00
117	0.38	56.70	50.10	0.00
119	0.36	56.68	51.38	0.00
120	0.26	55.76	49.96	0.00
121	0.10	48.28	41.38	0.00
122	0.10	48.27	41.37	0.00
123	0.18	48.26	41.96	0.00
124	0.49	48.25	41.55	0.00
129	0.05	48.26	42.06	0.00
130	0.34	37.53	30.48	0.00
131	15.34	27.78	20.78	0.00
132	0.25	37.43	30.43	0.00
133	0.53	37.42	30.32	0.00
134	0.15	37.40	32.10	0.00
135	0.63	37.38	31.38	0.00
136	0.33	37.40	32.80	0.00
138	0.15	55.53	50.33	0.00
139	0.11	55.54	50.24	0.00
140	0.21	55.58	48.78	0.00
141	0.27	55.67	49.77	0.00
142	0.37	55.64	48.54	0.00
143	0.21	55.54	50.14	0.00
144	0.39	55.53	50.33	0.00
145	0.46	55.50	52.10	0.00
146	8.28	55.41	52.41	0.00
151	0.00	56.97	48.97	0.00
118	5.36	55.93	51.83	0.00
150	0.00	58.80	46.80	0.00
149	0.00	58.80	46.80	0.00
50	-35.66	61.47	0.00	Reservoir

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Scenario 8

Link Results:

Link ID	Flow LPS	Velocity Uni m/s	Headloss m/km	Status
18	-1.61	0.19	0.86	Open
19	-3.61	0.42	3.82	Open
20	1.67	0.19	0.91	Open
23	25.95	0.81	4.92	Open
25	18.48	2.16	78.71	Open
26	8.88	0.28	0.67	Open
27	0.81	0.10	0.24	Open
28	17.57	2.06	71.65	Open
29	0.71	0.08	0.19	Open
30	0.67	0.08	0.17	Open
31	0.49	0.06	0.09	Open
36	15.34	1.80	55.74	Open
37	1.89	0.22	1.15	Open

	1253-08 Scenario	8 FFF	+ 0. 667	Exist	PH	Flows	225	+ 150	Mains
38		0. 53	0. 06	0. 11		Open			
39		1. 11	0. 13	0. 43		Open			
40		0. 63	0. 07	0. 15		Open			
41		0. 33	0. 04	0. 04		Open			
45		-1. 19	0. 14	0. 49		Open			
46		1. 04	0. 12	0. 38		Open			
47		-1. 08	0. 13	0. 41		Open			
48		-0. 22	0. 03	0. 02		Open			
49		-1. 29	0. 15	0. 57		Open			
50		-1. 57	0. 18	0. 82		Open			
51		8. 50	0. 26	0. 62		Open			
52		0. 39	0. 05	0. 06		Open			
53		7. 69	0. 24	0. 52		Open			
54		8. 28	0. 26	0. 59		Open			
21		16. 95	0. 43	1. 33		Open			
22		9. 28	0. 51	2. 94		Open			
59		9. 36	0. 52	2. 99		Open			
60		5. 36	0. 30	1. 06		Open			
61		24. 73	0. 62	2. 67		Open			
62		-1. 54	0. 04	0. 02		Open			
63		-10. 93	0. 60	3. 98		Open			
64		9. 39	0. 52	2. 99		Open			
66		26. 26	0. 66	2. 99		Open			
67		26. 38	0. 66	3. 01		Open			
68		20. 94	0. 53	1. 96		Open			

1253-09 Scenario FFF + 0.667 Full Dev. PH Flows 225 + 150 Mains
 Page 1 28-Jun-13 3:43:42 AM

 * E P A N E T *
 * Hydraulic and Water Quality *
 * Analysis for Pipe Networks *
 * Version 2.0 *

Input File: 1253-09 Scenario FFF + 0.667 Full Dev. PH Flows 225 + 150 Mains.NET

Scenario 9

Firefighting flows imposed on 2/3 peak hour flows for the full development with reservoir level at RL 61.47m with the sole supply line being the 2250 water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
2	101	102	30	104.3
3	102	110	183	104.3
5	110	111	23	104.3
6	111	103	11	104.3
7	111	108	77	104.3
8	111	112	165	104.3
9	112	113	67	104.3
10	112	114	22	104.3
11	112	104	11	104.3
12	114	107	216	104.3
13	107	106	24	104.3
14	106	105	36	104.3
15	107	108	233	104.3
16	108	109	83	104.3
17	105	115	60	104.3
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
24	119	126	154	104.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
32	126	128	20	104.3
33	128	125	10	104.3
34	128	127	57	104.3
35	128	129	78	104.3
36	130	131	175	104.3
37	130	132	87	104.3

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Page 2

Link - Node Table: (continued)

Scenario 9

Link ID	Start Node	End Node	Length m	Diameter mm
38	132	133	121	104.3

	1253-09 Scenario FFF	+ 0.667	Ful l Dev.	PH Fl ows	225	+ 150	Mai ns
39	132	134		79	104. 3		
40	134	135		145	104. 3		
41	134	136		25	104. 3		
42	136	137		65	104. 3		
43	137	147		80	104. 3		
44	137	138		81	104. 3		
45	138	139		13	104. 3		
46	138	145		85	104. 3		
47	139	140		86	104. 3		
48	139	143		15	104. 3		
49	140	141		173	104. 3		
50	141	115		90	104. 3		
51	142	143		167	202. 2		
52	143	144		241	104. 3		
53	143	145		76	202. 2		
54	145	146		149	202. 2		
55	147	148		216	104. 3		
56	109	110		157	48. 4		
57	105	108		288	48. 4		
21	117	119		15	225		
22	101	151		210	152		
59	151	119		96	152		
60	151	118		982	152		
61	50	150		1000	225		
62	150	149		5	225		
63	149	50		671	152		
64	149	101		404	152. 2		
66	150	101		404	225		
67	101	151		205	225		
68	151	117		136	225		

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Qual i ty
101	0. 00	51. 12	47. 62	0. 00
102	0. 44	50. 84	47. 44	0. 00
103	0. 44	49. 18	45. 38	0. 00
104	0. 44	48. 97	45. 57	0. 00
105	0. 53	48. 87	44. 77	0. 00
106	0. 49	48. 89	44. 79	0. 00
107	0. 29	48. 92	44. 52	0. 00
108	0. 29	49. 07	45. 07	0. 00
109	0. 53	49. 07	41. 97	0. 00
110	0. 39	49. 33	45. 93	0. 00

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Page 3
Node Results: (continued)

Scenario 9

Node ID	Demand LPS	Head m	Pressure m	Qual i ty
111	0. 25	49. 18	45. 38	0. 00
112	0. 19	48. 97	45. 57	0. 00
113	0. 29	48. 97	43. 77	0. 00
114	0. 34	48. 96	45. 56	0. 00
115	0. 05	48. 85	43. 95	0. 00
116	0. 33	48. 90	43. 30	0. 00
117	0. 38	49. 69	43. 09	0. 00
119	0. 36	49. 67	44. 37	0. 00
120	0. 26	48. 90	43. 10	0. 00

	1253-09	Scenario	FFF	+ 0. 667	Ful l Dev.	PH	Fl ows	225	+ 150	Mai ns
121			0. 10	48. 38	41. 48		0. 00			
122			0. 10	48. 46	41. 56		0. 00			
123			0. 18	48. 59	42. 29		0. 00			
124			0. 49	48. 58	41. 88		0. 00			
125			0. 34	48. 84	43. 14		0. 00			
126			0. 19	48. 93	43. 23		0. 00			
127			0. 19	48. 84	42. 44		0. 00			
128			0. 05	48. 84	43. 04		0. 00			
129			0. 05	48. 59	42. 39		0. 00			
130			0. 34	46. 55	39. 50		0. 00			
131			0. 34	46. 55	39. 55		0. 00			
132			0. 25	45. 68	38. 68		0. 00			
133			0. 53	45. 67	38. 57		0. 00			
134			0. 15	45. 07	39. 77		0. 00			
135			0. 63	45. 05	39. 05		0. 00			
136			0. 33	44. 92	40. 32		0. 00			
137			0. 05	44. 60	40. 70		0. 00			
138			0. 15	47. 56	42. 36		0. 00			
139			0. 11	47. 82	42. 52		0. 00			
140			0. 21	48. 04	41. 24		0. 00			
141			0. 27	48. 54	42. 64		0. 00			
142			0. 37	48. 42	41. 32		0. 00			
143			0. 21	47. 97	42. 57		0. 00			
144			0. 39	47. 95	42. 75		0. 00			
145			0. 46	47. 87	44. 47		0. 00			
146			8. 28	47. 78	44. 78		0. 00			
147			0. 54	39. 58	36. 18		0. 00			
148			15. 81	26. 85	21. 75		0. 00			
151			0. 00	49. 98	41. 98		0. 00			
118			5. 36	48. 93	44. 83		0. 00			
150			0. 00	54. 10	42. 10		0. 00			
149			0. 00	54. 10	42. 10		0. 00			
50			-42. 75	61. 47	0. 00		0. 00	Reservoi r		

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Page 4
Link Results:

Scenario 9

Link ID	Flow LPS	Velocity m/s	Headloss m/km	Status
2	5. 91	0. 69	9. 53	Open
3	5. 47	0. 64	8. 25	Open
5	4. 78	0. 56	6. 43	Open
6	0. 44	0. 05	0. 08	Open
7	2. 10	0. 25	1. 41	Open
8	1. 99	0. 23	1. 27	Open
9	0. 29	0. 03	0. 04	Open
10	1. 06	0. 12	0. 40	Open
11	0. 44	0. 05	0. 08	Open
12	0. 72	0. 08	0. 19	Open
13	1. 82	0. 21	1. 07	Open
14	1. 33	0. 16	0. 60	Open
15	-1. 39	0. 16	0. 65	Open
16	0. 23	0. 03	0. 02	Open
17	0. 99	0. 12	0. 35	Open
18	-2. 45	0. 29	1. 87	Open
19	-3. 29	0. 38	3. 21	Open
20	0. 51	0. 06	0. 10	Open
23	23. 39	0. 73	4. 06	Open
24	4. 07	0. 48	4. 79	Open
25	4. 38	0. 51	5. 46	Open

	1253-09	Scenario	FFF	+	0.667	Ful l	Dev.	PH	Fl ows	225	+	150	Mai ns
26			19. 26		0. 60		2. 83		Open				
27			-2. 48		0. 29		1. 91		Open				
28			6. 75		0. 79		12. 20		Open				
29			-2. 58		0. 30		2. 05		Open				
30			0. 67		0. 08		0. 17		Open				
31			0. 49		0. 06		0. 09		Open				
32			3. 87		0. 45		4. 36		Open				
33			0. 34		0. 04		0. 05		Open				
34			0. 19		0. 02		0. 02		Open				
35			3. 29		0. 39		3. 23		Open				
36			0. 34		0. 04		0. 05		Open				
37			6. 07		0. 71		10. 02		Open				
38			0. 53		0. 06		0. 11		Open				
39			5. 29		0. 62		7. 77		Open				
40			0. 63		0. 07		0. 15		Open				
41			4. 51		0. 53		5. 78		Open				
42			4. 19		0. 49		5. 03		Open				
43			16. 35		1. 91		62. 71		Open				
44			-12. 21		1. 43		36. 55		Open				
45			-8. 82		1. 03		19. 99		Open				
46			-3. 54		0. 41		3. 69		Open				
47			-2. 91		0. 34		2. 56		Open				
48			-6. 02		0. 70		9. 85		Open				
49			-3. 12		0. 37		2. 92		Open				
50			-3. 40		0. 40		3. 41		Open				
51			18. 89		0. 59		2. 73		Open				

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Page 5
Link Results: (continued)

Scenario 9

Link ID	Flow LPS	Veloci tyUni t m/s	Headl oss m/km	Status
52	0. 39	0. 05	0. 06	Open
53	12. 28	0. 38	1. 23	Open
54	8. 28	0. 26	0. 59	Open
55	15. 81	1. 85	58. 92	Open
56	-0. 30	0. 17	1. 64	Open
57	-0. 19	0. 10	0. 69	Open
21	18. 08	0. 45	1. 50	Open
22	0. 00	0. 00	0. 00	Closed
59	9. 74	0. 54	3. 22	Open
60	5. 36	0. 30	1. 06	Open
61	42. 75	1. 08	7. 37	Open
62	0. 00	0. 00	0. 00	Open
63	0. 00	0. 00	0. 00	Closed
64	0. 00	0. 00	0. 00	Closed
66	42. 75	1. 08	7. 37	Open
67	36. 84	0. 93	5. 60	Open
68	21. 75	0. 55	2. 11	Open

1253-10 Scenario FFF + Full Dev. PH Flows Min Pressure
 Page 1 28-Jun-13 3:44:32 AM

 * E P A N E T *
 * Hydraulic and Water Quality *
 * Analysis for Pipe Networks *
 * Version 2.0 *

Input File: 1253-10 Scenario FFF + Full Dev. PH Flows Min Pressure.NET

Scenario 10

Firefighting flows imposed on the full peak hour flows for the full development with the reservoir level at RL 61.47m with the sole supply line being the 2250 water main from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
2	101	102	30	104.3
3	102	110	183	104.3
5	110	111	23	104.3
6	111	103	11	104.3
7	111	108	77	104.3
8	111	112	165	104.3
9	112	113	67	104.3
10	112	114	22	104.3
11	112	104	11	104.3
12	114	107	216	104.3
13	107	106	24	104.3
14	106	105	36	104.3
15	107	108	233	104.3
16	108	109	83	104.3
17	105	115	60	104.3
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
24	119	126	154	104.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
32	126	128	20	104.3
33	128	125	10	104.3
34	128	127	57	104.3
35	128	129	78	104.3
36	130	131	175	104.3
37	130	132	87	104.3

♀

Page 2 Scenario 10
 Link - Node Table: (continued)

Link ID	Start Node	End Node	Length m	Diameter mm
38	132	133	121	104.3

	1253-10 Scenario	FFF + Full Dev.	PH Flows	Min Pressure
39	132	134	79	104.3
40	134	135	145	104.3
41	134	136	25	104.3
42	136	137	65	104.3
43	137	147	80	104.3
44	137	138	81	104.3
45	138	139	13	104.3
46	138	145	85	104.3
47	139	140	86	104.3
48	139	143	15	104.3
49	140	141	173	104.3
50	141	115	90	104.3
51	142	143	167	202.2
52	143	144	241	104.3
53	143	145	76	202.2
54	145	146	149	202.2
55	147	148	216	104.3
56	109	110	157	48.4
57	105	108	288	48.4
21	117	119	15	225
22	101	151	210	152
59	151	119	96	152
60	151	118	982	152
61	50	150	1000	225
62	150	149	5	225
63	149	50	671	152
64	149	101	404	152.2
66	150	101	404	225
67	101	151	205	225
68	151	117	136	225

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0.00	44.06	40.56	0.00
102	0.66	43.57	40.17	0.00
103	0.66	40.74	36.94	0.00
104	0.66	40.41	37.01	0.00
105	0.80	40.32	36.22	0.00
106	0.73	40.33	36.23	0.00
107	0.44	40.36	35.96	0.00
108	0.44	40.57	36.57	0.00
109	0.80	40.57	33.47	0.00
110	0.58	40.99	37.59	0.00

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Page 3
Node Results: (continued)

Scenario 10

Node ID	Demand LPS	Head m	Pressure m	Quality
111	0.37	40.75	36.95	0.00
112	0.29	40.41	37.01	0.00
113	0.44	40.40	35.20	0.00
114	0.51	40.40	37.00	0.00
115	0.07	40.31	35.41	0.00
116	0.49	40.43	34.83	0.00
117	0.57	41.68	35.08	0.00
119	0.54	41.65	36.35	0.00
120	0.39	40.43	34.63	0.00

	1253-10	Scenario	FFF	+ Full	Dev.	PH	Flows	Min	Pressure
121		0.15	39.72		32.82		0.00		
122		0.15	39.82		32.92		0.00		
123		0.27	39.98		33.68		0.00		
124		0.73	39.96		33.26		0.00		
125		0.51	40.35		34.65		0.00		
126		0.29	40.49		34.79		0.00		
127		0.29	40.35		33.95		0.00		
128		0.07	40.35		34.55		0.00		
129		0.07	39.98		33.78		0.00		
130		0.51	37.31		30.26		0.00		
131		0.51	37.29		30.29		0.00		
132		0.37	36.23		29.23		0.00		
133		0.80	36.20		29.10		0.00		
134		0.22	35.54		30.24		0.00		
135		0.95	35.49		29.49		0.00		
136		0.49	35.40		30.80		0.00		
137		0.08	35.10		31.20		0.00		
138		0.22	38.48		33.28		0.00		
139		0.16	38.80		33.50		0.00		
140		0.32	39.11		32.31		0.00		
141		0.41	39.85		33.95		0.00		
142		0.56	39.67		32.57		0.00		
143		0.31	38.96		33.56		0.00		
144		0.58	38.93		33.73		0.00		
145		0.69	38.80		35.40		0.00		
146		12.41	38.61		35.61		0.00		
147		0.81	29.69		26.29		0.00		
148		16.22	16.34		11.24		0.00		
151		0.00	42.14		34.14		0.00		
118		8.03	39.93		35.83		0.00		
150		0.00	49.07		37.07		0.00		
149		0.00	49.07		37.07		0.00		
50		-56.62	61.47		0.00		0.00	Reservoir	

♀

Page 4
Link Results:

Scenario 10

Link ID	Flow LPS	Velocity m/s	Headloss m/km	Status
2	7.95	0.93	16.51	Open
3	7.29	0.85	14.06	Open
5	6.31	0.74	10.77	Open
6	0.66	0.08	0.16	Open
7	2.71	0.32	2.25	Open
8	2.57	0.30	2.04	Open
9	0.44	0.05	0.08	Open
10	1.18	0.14	0.48	Open
11	0.66	0.08	0.16	Open
12	0.67	0.08	0.17	Open
13	1.88	0.22	1.15	Open
14	1.15	0.14	0.46	Open
15	-1.65	0.19	0.90	Open
16	0.40	0.05	0.07	Open
17	0.57	0.07	0.13	Open
18	-3.74	0.44	4.08	Open
19	-4.23	0.50	5.14	Open
20	0.01	0.00	0.00	Open
23	30.10	0.94	6.47	Open
24	5.19	0.61	7.53	Open
25	5.17	0.61	7.44	Open

	1253-10	Scenario FFF + Full Dev.	PH Flows	Min Pressure
26	24.54	0.76	4.44	Open
27	-2.81	0.33	2.41	Open
28	7.84	0.92	16.06	Open
29	-2.96	0.35	2.65	Open
30	1.00	0.12	0.35	Open
31	0.73	0.09	0.20	Open
32	4.90	0.57	6.74	Open
33	0.51	0.06	0.10	Open
34	0.29	0.03	0.04	Open
35	4.03	0.47	4.70	Open
36	0.51	0.06	0.10	Open
37	6.82	0.80	12.41	Open
38	0.80	0.09	0.23	Open
39	5.65	0.66	8.75	Open
40	0.95	0.11	0.32	Open
41	4.48	0.52	5.69	Open
42	3.99	0.47	4.59	Open
43	17.03	1.99	67.64	Open
44	-13.12	1.54	41.75	Open
45	-9.78	1.14	24.22	Open
46	-3.56	0.42	3.73	Open
47	-3.51	0.41	3.63	Open
48	-6.43	0.75	11.14	Open
49	-3.83	0.45	4.26	Open
50	-4.24	0.50	5.15	Open
51	23.98	0.75	4.25	Open

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Link Results: (continued)

Scenario 10

Link ID	Flow LPS	Velocity m/s	Headloss m/km	Status
52	0.58	0.07	0.13	Open
53	16.66	0.52	2.17	Open
54	12.41	0.39	1.25	Open
55	16.22	1.90	61.80	Open
56	-0.40	0.22	2.72	Open
57	-0.22	0.12	0.88	Open
21	23.26	0.59	2.39	Open
22	0.00	0.00	0.00	Closed
59	12.57	0.69	5.16	Open
60	8.03	0.44	2.25	Open
61	56.62	1.42	12.40	Open
62	0.00	0.00	0.00	Open
63	0.00	0.00	0.00	Closed
64	0.00	0.00	0.00	Closed
66	56.62	1.42	12.40	Open
67	48.67	1.22	9.37	Open
68	28.06	0.71	3.38	Open

1253-11 Scenario FFF + Full Dev PH Flows Max Pressure
 Page 1 28-Jun-13 3:04:38 AM

 * E P A N E T *
 * Hydraulic and Water Quality *
 * Analysis for Pipe Networks *
 * Version 2.0 *

Input File: 1253-11 Scenario FFF + Full Dev PH Flows Max Pressure.NET

Scenario 11

Firefighting flows imposed on 2/3 peak hour flows for the full development with reservoir level at RL 68.88m with a 1500 and a 2250 supply lines from the reservoir to the intersection of Bonnie Doon and Cooya Beach Roads.

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
2	101	102	30	104.3
3	102	110	183	104.3
5	110	111	23	104.3
6	111	103	11	104.3
7	111	108	77	104.3
8	111	112	165	104.3
9	112	113	67	104.3
10	112	114	22	104.3
11	112	104	11	104.3
12	114	107	216	104.3
13	107	106	24	104.3
14	106	105	36	104.3
15	107	108	233	104.3
16	108	109	83	104.3
17	105	115	60	104.3
18	115	116	29	104.3
19	116	117	244	104.3
20	116	120	15	104.3
23	119	120	188	202.2
24	119	126	154	104.2
25	120	121	95	104.3
26	120	142	170	202.2
27	121	122	42	104.3
28	121	130	150	104.3
29	122	129	61	104.3
30	129	123	22	104.3
31	123	124	68	104.3
32	126	128	20	104.3
33	128	125	10	104.3
34	128	127	57	104.3
35	128	129	78	104.3
36	130	131	175	104.3
37	130	132	87	104.3
38	132	133	121	104.3

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Page 2
 Link - Node Table: (continued)

Scenario 11

Link ID	Start Node	End Node	Length m	Diameter mm
39	132	134	79	104.3

	1253-11	Scenario	FFF	+ Full	Dev	PH	Flo ws	Max	Pressure
40	134		135				145	104.	3
41	134		136				25	104.	3
42	136		137				65	104.	3
43	137		147				80	104.	3
44	137		138				81	104.	3
45	138		139				13	104.	3
46	138		145				85	104.	3
47	139		140				86	104.	3
48	139		143				15	104.	3
49	140		141				173	104.	3
50	141		115				90	104.	3
51	142		143				167	202.	2
52	143		144				241	104.	3
53	143		145				76	202.	2
54	145		146				149	202.	2
55	147		148				216	104.	3
56	109		110				157	48.	4
57	105		108				288	48.	4
21	117		119				15	225	
22	101		151				210	152	
59	151		119				96	152	
60	151		118				982	152	
61	50		150				1000	225	
62	150		149				5	225	
63	149		50				671	152	
64	149		101				404	152.	2
66	150		101				404	225	
67	101		151				205	225	
68	151		117				136	225	

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
101	0. 00	51. 47	47. 97	0. 00
102	0. 66	50. 98	47. 58	0. 00
103	0. 66	48. 15	44. 35	0. 00
104	0. 66	47. 82	44. 42	0. 00
105	0. 80	47. 73	43. 63	0. 00
106	0. 73	47. 74	43. 64	0. 00
107	0. 44	47. 77	43. 37	0. 00
108	0. 44	47. 98	43. 98	0. 00
109	0. 80	47. 98	40. 88	0. 00
110	0. 58	48. 40	45. 00	0. 00
111	0. 37	48. 16	44. 36	0. 00

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Node Results: (continued)

Scenario 11

Node ID	Demand LPS	Head m	Pressure m	Quality
112	0. 29	47. 82	44. 42	0. 00
113	0. 44	47. 81	42. 61	0. 00
114	0. 51	47. 81	44. 41	0. 00
115	0. 07	47. 72	42. 82	0. 00
116	0. 49	47. 84	42. 24	0. 00
117	0. 57	49. 09	42. 49	0. 00
119	0. 54	49. 06	43. 76	0. 00
120	0. 39	47. 84	42. 04	0. 00
121	0. 15	47. 13	40. 23	0. 00

	1253-11	Scenario	FFF + Full Dev PH Flows	Max Pressure
122		0.15	47.23	0.00
123		0.27	47.39	0.00
124		0.73	47.37	0.00
125		0.51	47.76	0.00
126		0.29	47.90	0.00
127		0.29	47.76	0.00
128		0.07	47.76	0.00
129		0.07	47.39	0.00
130		0.51	44.72	37.67
131		0.51	44.70	37.70
132		0.37	43.64	36.64
133		0.80	43.61	36.51
134		0.22	42.95	37.65
135		0.95	42.90	36.90
136		0.49	42.81	38.21
137		0.08	42.51	38.61
138		0.22	45.89	40.69
139		0.16	46.21	40.91
140		0.32	46.52	39.72
141		0.41	47.26	41.36
142		0.56	47.08	39.98
143		0.31	46.37	40.97
144		0.58	46.34	41.14
145		0.69	46.21	42.81
146		12.41	46.02	43.02
147		0.81	37.10	33.70
148		16.22	23.75	18.65
151		0.00	49.55	41.55
118		8.03	47.34	43.24
150		0.00	56.48	44.48
149		0.00	56.48	44.48
50		-56.62	68.88	0.00 Reservoir

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Link Results:

Scenario 11

Link ID	Flow LPS	Velocity m/s	Headloss m/km	Status
2	7.95	0.93	16.51	Open
3	7.29	0.85	14.06	Open
5	6.31	0.74	10.77	Open
6	0.66	0.08	0.16	Open
7	2.71	0.32	2.25	Open
8	2.57	0.30	2.04	Open
9	0.44	0.05	0.08	Open
10	1.18	0.14	0.48	Open
11	0.66	0.08	0.16	Open
12	0.67	0.08	0.17	Open
13	1.88	0.22	1.15	Open
14	1.15	0.14	0.46	Open
15	-1.65	0.19	0.90	Open
16	0.40	0.05	0.07	Open
17	0.57	0.07	0.13	Open
18	-3.74	0.44	4.08	Open
19	-4.23	0.50	5.14	Open
20	0.01	0.00	0.00	Open
23	30.10	0.94	6.47	Open
24	5.19	0.61	7.53	Open
25	5.17	0.61	7.44	Open
26	24.54	0.76	4.44	Open

	1253-11	Scenario	FFF	+ Full	Dev PH	Flows	Max	Pressure
27	-2.81	0.33	2.41		Open			
28	7.84	0.92	16.06		Open			
29	-2.96	0.35	2.65		Open			
30	1.00	0.12	0.35		Open			
31	0.73	0.09	0.20		Open			
32	4.90	0.57	6.74		Open			
33	0.51	0.06	0.10		Open			
34	0.29	0.03	0.04		Open			
35	4.03	0.47	4.70		Open			
36	0.51	0.06	0.10		Open			
37	6.82	0.80	12.41		Open			
38	0.80	0.09	0.23		Open			
39	5.65	0.66	8.75		Open			
40	0.95	0.11	0.32		Open			
41	4.48	0.52	5.69		Open			
42	3.99	0.47	4.59		Open			
43	17.03	1.99	67.64		Open			
44	-13.12	1.54	41.75		Open			
45	-9.78	1.14	24.22		Open			
46	-3.56	0.42	3.73		Open			
47	-3.51	0.41	3.63		Open			
48	-6.43	0.75	11.14		Open			
49	-3.83	0.45	4.26		Open			
50	-4.24	0.50	5.15		Open			
51	23.98	0.75	4.25		Open			

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Link Results: (continued)

Scenario 11

Link ID	Flow LPS	Velocity Unit m/s	Headloss m/km	Status
52	0.58	0.07	0.13	Open
53	16.66	0.52	2.17	Open
54	12.41	0.39	1.25	Open
55	16.22	1.90	61.80	Open
56	-0.40	0.22	2.72	Open
57	-0.22	0.12	0.88	Open
21	23.26	0.59	2.39	Open
22	0.00	0.00	0.00	Closed
59	12.57	0.69	5.16	Open
60	8.03	0.44	2.25	Open
61	56.62	1.42	12.40	Open
62	0.00	0.00	0.00	Open
63	0.00	0.00	0.00	Closed
64	0.00	0.00	0.00	Closed
66	56.62	1.42	12.40	Open
67	48.67	1.22	9.37	Open
68	28.06	0.71	3.38	Open