GMA Certification Group

BUILDING SURVEYORS

ACN 150 435 617

Leaders in Building Certification Services

PLANNING DIVISION

P: 0438 755 374 **E:** Patrick.c@gmacert.com.au P.O. Box 2760, Nerang Qld 4211

Our Ref: 20204033 Date: 10 March 2021

Department of Infrastructure Local Government and Planning State Assessment and Referral Agency (SARA) PO Box 2358 Cairns Qld 4870

Via Email: CairnsSARA@dilgp.qld.gov.au

Dear Sir.

RE: **RESPONSE** TO SARA INFORMATION REQUEST APPLICATION MATERIAL FOR CHANGE FUNCTION FACILITY, ON LAND LOCATED ΑT HIGHWAY, CAPTAIN COOK OAK **BEACH** RP742791)

SARA REF: 2012-20341 SRA

Reference is made to SARA's Information Request pursuant to section 12 of the Development Assessment Rules, dated 4 January 2021, in respect of the abovementioned application.

Specifically, the following information was requested to complete the assessment of the application:

 A Traffic Impact Assessment Report that demonstrates and confirms that the proposed Function Facility would not result in a worsening of the safety or operating conditions of Captain Cook Highway The Traffic Impact Assessment Report is to be prepared in accordance with the Guide to Traffic Impact Assessment (GTIA) by a qualified professional and certified by a Registered Professional Engineer of Queensland (RPEQ).

In accordance with section 13.2 of the Development Assessment Rules please find attached the following:

www.gmacert.com.au

BUILDING APPROVALS & INSPECTIONS BUILDING CERTIFICATION ENERGY EFFICIENCY ASSESSMENTS TOWN PLANNING

Gold Coast Caboolture Townsville Cairns Port Douglas Childers Kingscliff

GMA Certification Pty Ltd

 A Traffic Report, prepared by ARO Industries, that provides an assessment in accordance with the Guide to Traffic Impact Assessment and concludes that the 'proposed function facility will not adversely impact on the operational performance of the surrounding road network and the proposed access arrangements are considered adequate and suitable for the proposed use.'

We look forward to receiving your positive referral response in respect of the proposal as soon as possible; should you have any queries regarding this matter please do not hesitate to contact the undersigned on 0438 755 374 or by email patrick.c@gmacert.com.au

Kind Regards,

Patrick Clifton
PLANNING MANAGER
GMA CERTIFICATION GROUP

Page 2 of 2

ARO INDUSTRIES

'COTTONWOOD' OAK BEACH TRAFFIC REPORT





TABLE OF CONTENTS

1.	INTRODUCTION	2
2.	EXISTING USE OF SITE	
3.	ADJACENT DEVELOPMENT	2
4.	TRAFFIC ENVIRONMENT ON CAPTAIN COOK HIGHWAY	3
5.	TRAFFIC VOLUMES ON CAPTAIN COOK HIGHWAY	3
6.	TRAFFIC GENERATION FOR FUNCTION CENTRES	4
7.	ASSESSMENT CRITERIA	5
8.	ASSESSMENT RESULTS	
9.	ROAD SECTION ANALYSIS	5
10.	SIGHT DISTANCE	5
11.	PROPOSED ACCESS	6
12.	PARKING	
	CONCLUSION	
APP	ENDIX A – SITE PLAN	
APP	ENDIX B – TMR TRAFFIC DATA	
APP	ENDIX C – SIDRA RESULTS	



1. INTRODUCTION

This engineering report has been prepared by ARO Industries to assess the traffic impacts of a proposed change of use to enable conversion of an existing house 'Cottonwood' to operate as a function centre to cater for 80 persons.

The site is Lot 1, RP742791, 5146 Captain Cook Highway, Oak Beach. It is located approximately 14.5km south of Port Douglas and 29km north of Palm Cove and is shown in figure 1. The site is located within the jurisdiction of Douglas Shire Council and is subject to its planning controls.



Figure 1 – Locality Plan (Courtesy of Queensland Globe)

The traffic assessment has been commissioned to investigate vehicular access to the site, car parking requirements and the ability of the surrounding road network to absorb future traffic growth associated with development on the subject site.

2. EXISTING USE OF SITE

The site of the proposed change of use is an existing residential allotment containing a 3-bedroom residential house and a 2-bedroom cottage located on a large block of approximately 1.92ha in size. Cottonwood currently operates as a prestige vacation home rental. The block has direct privileged access onto Oak Beach.

The site is currently accessed from the Captain Cook Highway via an existing driveway. A site plan has been included in Appendix A.

3. ADJACENT DEVELOPMENT

There are numerous small lot residential developments along both sides of the Captain Cook Highway in the Oak Beach Area. To the North there is an eight-lot residential development, with access to the highway via an intersection with Toll Gate Road. Similarly, there subdivisions to the South (59 lots accessing the highway via Reynolds Road and another 32 lots off Oak Beach Road. The sites surrounding Cottonwood are typically large, with various uses.





Figure 2 – Site Location (Courtesy of Queensland Globe)

4. TRAFFIC ENVIRONMENT ON CAPTAIN COOK HIGHWAY

The Captain Cook Highway is a State Controlled Road aligned North-South between Cairns and Port Douglas. The highway links Cairns' northern beaches suburbs, Wangetti and Oak beach communities. The highway consists of a two-lane, undivided road, 7.0m wide with line marking and sealed shoulders. Some overtaking lanes and widening at intersections have been provided where warranted.

The existing speed zone on the Captain Cook Highway past the Oak Beach community is 80km/h and has flat gradients approaching the access to the development from both the north and the south. The site access is via a restricted access driveway. Whilst there is no intention of changing the access to the site, throughout the analysis it has priority-controlled t-intersection.

5. TRAFFIC VOLUMES ON CAPTAIN COOK HIGHWAY

Traffic data has been obtained from TMR counter ID No. 110022 located in Craiglie for the year 2019 and has been extrapolated for the year 2021. Due to Port Douglas being a popular local destination on the weekends both weekday and weekend traffic has been considered. The critical AM peak identified in the traffic data is 10am on both weekdays and weekends. The afternoon peak for weekdays occurs at 4pm and at 2pm on weekends. To predict the future volume of traffic on the Captain Cook Highway a linear growth rate of 2% has been applied to the existing traffic volumes. Based on the above growth rate and the AM and PM Peak background traffic volumes for 2021 and the design horizon of 10 years the traffic volumes were calculated. The backgrounds traffic volume om the Captain Cook Highway have been summarised in table 1, and the raw traffic data included in Appendix B.



Table 1: Traffic Volumes for the Captain Cook Highway

	20	21	20	31	Comments
	Weekday	Weekend	Weekday	Weekend	
AM Peak (two-way traffic)	539	652	642	777	Peak time = 10am (weekdays and weekends)
PM Peak (two-way traffic)	582	583	694	696	Peak time = 4pm weekdays Peak time = 2pm weekends
Daily (two-way traffic)	6870	6448	8191	7688	

6. TRAFFIC GENERATION FOR FUNCTION CENTRES

To analyse the impact of the development on the highway, it is necessary to assess the number of trips generated to and from the site and where they are likely to travel. Austroads Guides and RTA Guidelines have no specific traffic generation rates for function centres.

The potential development traffic generation from the site has been reviewed based on information provided by the applicant. This can be summarised as:

- The facility will provide space to accommodate a wedding function for 80 persons.
- On the day prior to the event, it is proposed a medium rigid (MR) truck will arrive to setup the equipment for the event and then leave.
- On the day of the event but prior to it is anticipated that vehicles will drop catering supplies and toilets to site and leave.
- Kitchen and food and beverage staff will arrive on the day of the event.
- Three Buses will be used to transport guests to the site.
 - One bus will carry a maximum capacity of 34 guests and two 28 persons per bus, Buses will arrive to site and drop off guests then depart, similarly, they will arrive to collect guests and return to either Port Douglas or Cairns.
- There would be three (3) additional staff required on the day including Celebrant, Entertainment and photographer is proposed onsite, arriving by private vehicle.
- The day after the event the MR truck and utes will return to collect the equipment and the toilets.

Therefore, the total trips generated for any one event will be 36 vehicles per event over a three-day period. The busiest day will be that of the event which will have 18 trips throughout the day. During this the peak will consist of seven (7) private vehicles and three (3) buses.

As part of the assessment, we have made a number of assumptions, which we believe are conservative, but provide confidence that the intersection is able to operate in the worst-case scenario without additional control(s). The assumptions include:

- The peak traffic volumes on the highway coincide with the peak traffic volumes with the event.
- That the traffic entering the site does so entirely from the South.
 - The right-hand turn treatment into the proposed development, when approaching from the South, is the critical and most adverse movement. Consequently, the assessment of the access driveway using the *most adverse movement*, has been undertaken. The findings of the investigation were that the access driveway operates safely and efficiently without any additional measures required to monitor or control the access, when assessed against the *most adverse movement*.
 - When considering the operation of the access against other, less adverse, movements the access performs equally well, if not better.
 - It should be explicitly noted, that the access to the property has been assessed as both an access driveway, and as a formal intersection. Assessing as an intersection is a more conservative approach but gives a higher degree of confidence as to the operation of the access.



7. ASSESSMENT CRITERIA

The performance of each leg of the priority-controlled intersection was analysed using SIDRA Intersection 9. SIDRA is an industry recognised analysis tool that estimates the capacity and performance of intersections based on input parameters, including geometry and traffic volumes, and provides estimates of an intersection's Degree of Saturation (DOS), queues and delays. Importantly it is noted that DOS is not the only performance indicator and that other measures such as critical delay and level of service (LOS) should also be considered when assessing the performance of an intersection.

8. ASSESSMENT RESULTS

The performance of the access was analysed using SIDRA Intersection 9 (SIDRA) which is an industry recognised analysis tool that estimates the capacity and performance of intersections based on input parameters, including geometry and traffic volumes, and provides estimates of an intersection's Degree of Saturation (DOS), queues and delays.

The analysis has been carried out with the projected traffic on the Captain Cook Highway in 2031 with the analysis results summarised in the table below.

Table 2: Assessment Results

		AM Peak		PM Peak				
	DOS	Delay (Sec)	95% Queue (m)	DOS	Delay (Sec)	95% Queue (m)		
Captain Cook Highway (North)	0.207	0.7		0.246	0.8			
Captain Cook Highway (South)	0.247	0.5	1	0.163	0.8	1		
Access	0.006	7.4	0	0.003	2.7	0		

Table 8.1 - SIDRA Results Summary

The results indicate the current form of the intersection / access operates within the typical performance thresholds for both peak periods, therefore, no mitigation measures are required.

9. ROAD SECTION ANALYSIS

The traffic generation and distribution from the site has been assessed and the impacts of the proposed development on the state-controlled road namely the Captain Cook Highway have been reviewed.

TMR's Guide to Traffic Impact Assessment (2018) states, in Section 6.4, that it is considered unreasonable to require quantification of the impacts on intersections and road links, unless the development creates an increase in traffic exceeding 5% of base traffic for any movement.

The annual average daily traffic volume (AADT) for 2031 for the Captain Cook Highway has been obtained from TMR and indicates traffic flows of 8,191 vehicles per weekday and 7688 vehicles on the weekend in the vicinity of the subject site.

The proposed development only generates an additional 18 trips per day maximum to the road network. This traffic impact is well below 5% of existing levels, therefore the impact on the state-controlled road network is considered insignificant, and detailed analysis is considered unnecessary.

10. SIGHT DISTANCE

Sight distance along Captain Cook Highway at the existing access to Cottonwood House are:

Looking Left (south) ≈ 195 metres
 Looking Right (north) ≈ 205 metres

AS/NZS 2890.1 – 2004 dictates the following sight distance requirements for access driveways at the development.

Frontage Road Speed	Minimum Sight Distance	Desirable Sight Distance			
80 km/hr	105m	111m			



Table 5.5 of the Austroads publication "Guide to Road Design Part 3: Geometric Design" indicates that the desirable minimum safe intersection sight distance for cars on sealed roads, with a driver reaction time of 2.0 seconds and level gradients is 181 metres.

Sight distance in both directions exceeds the desirable minimum value along the Captain Cook Highway at the existing access to Cottonwood.

There are frequent driveways along the Captain Cook Highway at individual properties and drivers should have an expectation that vehicles could be turning in or out of such properties.

11. PROPOSED ACCESS

The proposed change of use to provide a function centre at Cottonwood house will utilise the existing access driveway on the eastern side of the Captain Cook Highway. The existing access consists of a gravel access road approximately 4.0 metres wide with a gate located on the boundary with the Captain Cook Highway.

12. PARKING

Douglas Shire Council's Planning Scheme 2018 does not provide any specific guidelines for vehicle access and parking requirements for function centres. The RTA Guide to Traffic Generating Developments also does not provide any specific parking guidelines for function centres, therefore parking will be as required by negotiations with Douglas Shire Council Officers.

13. CONCLUSION

This report has assessed the impact of traffic generated by the proposed function centre on the external transport network. Consideration has been given to operational performance, road safety and access arrangements.

The proposed change of use will permit the use of existing houses at Cottonwood to operate as a Function Centre that will generate minimal traffic volumes, being outside weekday and weekend peak traffic volumes.

Results of the analysis indicate that the road network continues to operate with capacity and the impact of development traffic on the operational performance of the external road network is insignificant.

Therefore, the subject proposal is supported based on the following features;

- The site will generate an estimated 36 trips per event with a minimum number occurring within the weekday or Weekend AM and PM peak hours. This level of traffic has a negligible impact on the surrounding road network in terms of traffic flow efficiency.
- The proposed bus service to events will reduce the dependency on private vehicle trips.
- The traffic generation from the proposed Function Centre will be outside weekday peak traffic periods and will be right inwards prior to the commencement of a function and left outwards towards the end of a function.
- Traffic volumes turning right into Cottonwood's access are outside weekday or weekend peak periods and are unlikely to experience significant opposing southbound traffic on the Captain Cook Highway, therefore widening of the road is considered unnecessary.
- At the completion of the event the left turning traffic leaving Cottonwood and returning to Cairns will experience minimal delays from through traffic.
- The available sight distance enables drivers travelling along the Captain Cook Highway to adequately observe vehicles turning into Cottonwood's access.

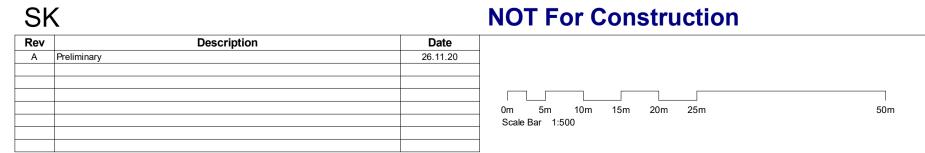
In conclusion, the proposed function facility will not adversely impact on the operational performance of the surrounding road network and the proposed access arrangements are considered adequate and suitable for the proposed use.



APPENDIX A Site Plan





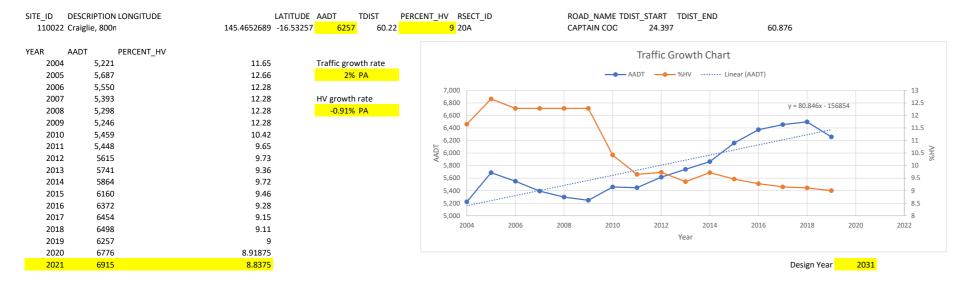


	5146R Captain Cook Hwy, Port Do	uglas	QLD	4877
	J:\CON00120 Oak Beach\4 NJA Documentation\5 CADD\1 SK\01 Model\CON00120_SK Model_Oak Beach_Central File.rvt	JOB No.		CON00120
TITLE		DATE		25.11.2020
	Site Plan	SCALE	A1 @	1 : 500
		DWG No.		
			SK.0	01 A

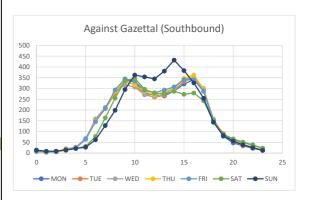


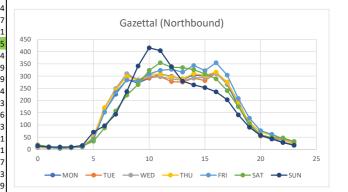
APPENDIX BTMR Traffic Data

Oak Beach Intersection Traffic Data



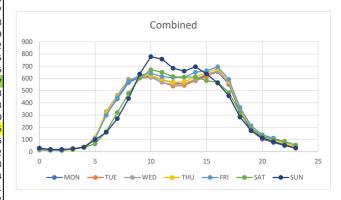
SITE_ID DESCRIPTION		SITE_DISTA LONGITUDI LATITUDE RSECT_ID	ROAD_NAME TD	DIST_START	TDIST_END GAZETTAL_DIRECTION	HOUR	MON	TUE	WED	THU	FRI	SAT	SUN	
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	()	7	7	6	6	7	12	1
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	1	1	5	5	4	4	5	6	
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	2	2	7	6	7	6	6	9	
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	3	3	16	20	15	12	17	16	1
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	4	4	27	25	24	27	25	21	2
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	Ę	5	67	64	63	66	66	31	2
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	6	5 1	L54	146	159	149	145	77	6
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	7	7 2	207	206	212	208	208	164	12
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	8	3 2	281	277	284	286	294	255	19
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	9	9 3	339	331	317	335	345	336	29
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	10) 3	331	317	306	317	334	347	36
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	11	1 2	279	273	270	285	291	296	35
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	17	2 2	269	259	260	270	280	278	34
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	13	3 2	265	268	274	284	293	275	38
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	14	4 2	288	290	300	301	308	286	43
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	15	53	321	332	343	347	341	273	38
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	16	5 3	341	351	350	362	340	279	32
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	17	7 2	286	288	300	298	289	243	25
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	18	3 1	L48	153	150	159	154	146	14
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	19	9	78	91	86	83	86	88	8
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	20)	47	55	56	61	62	66	
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	21	1	33	36	41	40	50	51	3
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	22	2	22	25	27	27	36	38	2
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 AGAINST GAZETTAL	23	3	12	12	12	15	22	22	1
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	()	9	7	7	11	14	19	1
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	1	1	7	7	6	7	9	11	1
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	2	2	4	5	4	5	5	7	1
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	3	3	6	7	7	6	7	9	1
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	4	4	10	10	11	11	11	14	:
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	ŗ	5	41	50	46	51	42	33	7
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	6	5 1	L54	167	171	171	153	88	9
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	7	7 2	226	246	249	243	233	157	14
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	8	3 2	285	309	310	298	284	222	23
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	9	9 2	272	279	285	284	280	265	34
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	10) 2	291	294	301	308	309	324	4:
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	11	1 3	309	298	298	308	324	355	40
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	12	2 2	299	278	289	296	327	336	33
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	13	3 2	288	275	283	289	316	335	27
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	14	4 3	304	291	293	310	343	326	26
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	15	5 3	300	281	294	303	322	309	25
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	16	5 3	314	312	310	316	355	289	23
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	17	7 2	267	275	269	275	304	241	20
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	18	3 1	L77	192	197	187	208	175	14
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	19		97		102	109	128	104	9
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	20		56	55	58	71	77	60	5
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	21	1	42	46	46	58	62	48	4
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	22		29	40	30	41	43	47	2
110022 Craiglie	800m South of Port Douglas Rd	60.22 145.4653 -16.53257 20A	CAPTAIN COOK HIGHWAY (CA	24.397	60.876 GAZETTAL	23		17	25	20	26	32	32	1





Oak Beach Intersection Traffic Data

COMBINED	0	16	15	14	17	21	31	29
COMBINED	1	12	12	10	11	14	17	19
COMBINED	2	11	11	11	11	11	16	19
COMBINED	3	22	27	22	19	25	25	24
COMBINED	4	37	35	35	38	36	35	37
COMBINED	5	108	114	109	117	108	64	98
COMBINED	6	308	314	330	320	298	165	159
COMBINED	7	433	451	461	451	441	321	272
COMBINED	8	567	585	594	584	578	477	435
COMBINED	9	610	610	603	619	625	601	636
COMBINED	10	622	611	608	625	642	671	777
COMBINED	11	588	570	568	593	615	651	758
COMBINED	12	568	537	549	567	608	614	683
COMBINED	13	553	543	557	573	609	610	660
COMBINED	14	591	582	593	611	651	613	696
COMBINED	15	621	614	637	650	663	582	636
COMBINED	16	655	663	660	678	694	568	562
COMBINED	17	553	563	569	573	593	484	458
COMBINED	18	325	345	347	346	362	321	284
COMBINED	19	175	187	187	192	213	192	171
COMBINED	20	103	109	114	131	139	125	113
COMBINED	21	76	82	87	98	112	99	82
COMBINED	22	51	64	57	68	79	86	53
COMBINED	23	30	37	32	41	55	55	30
·		7634.68	7683.04	7753.72	7933.52	8191.44	7422.64	7688





APPENDIX C SIDRA Results

DETAILED OUTPUT

∇ Site: 101 [Captain Cook Highway 2031 10AM + Arrive South]

New Site

Site Category: (None) Giveway / Yield (Two-Way)

OUTPUT TABLE LINKS



Sign Control Basic Parameters Gap Acceptance Parameters

Movements

Intersection Negotiation and Travel Data

Movement Capacity and Performance Parameters

Fuel Consumption, Emissions and Cost

Lanes

Lane Performance and Capacity Information

Lane, Approach and Intersection Performance

Driver Characteristics

Lane Delays

Lane Queues

Lane Queue Percentiles

Lane Stops

Îr Flow Rates

Origin-Destination Flow Rates (Total)

Origin-Destination Flow Rates by Movement Class

Lane Flow Rates

=Other

Parameter Settings Summary

Diagnostics

Sign Control

Sign Control Basic Parameters

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

Approach Control	Leg Geometry	App Dist m	Prop Queued Upstr Signal	Extra Bunching
South: CC So		30000	NA	0.0N
NorthEast: A	 Access Two Way	20	NA	0.0N
North: CC No		14000	NA	0.0N

NA Not Applicable (single Site analysis or unconnected Site in Network analysis).

Go to Table Links (Top)

N Program option resulted in zero value (single Site analysis or unconnected Site in Network analysis).

Gap Acceptance Parameters

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

Opd Lane	Dest	Opng Flow pcu/h	Intra Bunch Hdwy sec	Propn Bnchd	Entry HV Equiv	Critic Hdwy sec	al Gap Dist m	Foll-up Headway sec
South: 0	CC South NE	397	1.80	0.047	1.15	4.60	102.0	2.30
NorthEas 1 1	st: Access S N	396+ 863+	1.80 0.96	0.047 0.056	1.00	4.00 6.45	88.9 141.6	2.20

North: CC North

No opposed movements on this approach.

Values in this table are adjusted for movement classes in the entry stream. Use the Pedestrians and Priorities input dialogs to specify opposing pedestrian movements.

+ Percentage of exiting flow included in opposing vehicle flow

Go to Table Links (Top)

Movements

Intersection Negotiation and Travel Data
Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

TRAVEL SPEED, TRAVEL DISTANCE AND TRAVEL TIME

From To Approach Exit	Turn	Running Speed km/h	Travel Speed km/h	Travel Distance m		Dem Flows	el Distance Arv Flows veh-km/h	Time
South: CC South North		80.0 79.2	80.0 79.2	44010.0# 30030.0#		19225.4 316.1	19225.4 316.1	240.4
NorthEast: Acces Sout Nort	h L1	78.5 76.6	78.1 75.8			31.6 14.8	31.6 14.8	0.4
North: CC North NorthEas Sout		25.4 79.9	25.4 79.9	14030.0# 44010.0#		14.8 16770.1	14.8 16770.1	0.6
ALL VEHICLES:		79.9	79.9	43739.5#	1971.4#	36372.8	36372.8	455.4

[&]quot;Running Speed" is the average speed excluding stopped periods.

Travel Time values include cruise times and intersection delays including acceleration, deceleration and idling delays.

Travel Distance and Travel Time values include travel on the External Exit section based on the Exit Distance or user-specified Downstream Distance value as applicable.

INTERSECTION NEGOTIATION DATA

From To Approach Exit	Turn	Negn Radius m	Negn Speed km/h	Dist	App Dist m	Exit Dist m	Downstr Dist m
South: CC South North NorthEast		s 13.4	80.0		30000 30000	14000	NA NA
NorthEast: Access South North	L1 R3	7.0 5.0		10.0	20 20	30000 14000	NA NA
North: CC North NorthEast South	L3 T1	10.0 S	5.0 80.0	23.6		20	NA NA

NA Downstream Distance does not apply if:
- Exit is an internal leg of a network

- "Program" option was specified
- Distance specified was less than the Exit Negotiation Distance Distance specified was greater than the exit leg length

Some Negotiation Radius, Speed or Distance values are user specified.

MOVEMENT SPEEDS AND GEOMETRIC DELAY

		App. Sp			Speeds	Queue Move-up	Geom
Mov ID	Turn	Cruise km/h	Negn	Negn	Cruise	-	Delay
South:	CC S	outh					
2	T1	80.0	80.0	80.0	80.0	0.4	0.0
3a	R1	80.0	5.0	5.0	5.0	0.4	12.8
NorthE	ast:	 Access					
24a	L1	5.0	5.0	5.0	80.0	5.0	0.0
26b	R3	5.0	5.0	5.0	80.0	5.0	0.0
North:	CC N	orth					
7b	L3	80.0	5.0	5.0	5.0		20.8
8	Т1	80.0	80.0	80.0	80.0		0.0

Go to Table Links (Top)

Movement Capacity and Performance Parameters Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

MOVEMENT CAPACITY PARAMETERS

Mov ID	Turn	Mov Cl.	Arv Flow veh/h	Flow	ovement Adjust. Flow pcu/h	-	Prac. Deg. Satn xp	Prac. Spare Cap.	Deg. Satn x
South: 2	CC S T1 R1	South # #	437 11	0 382	0 397	1767 43	0.98 0.98	297 297	0.247* 0.247*
NorthE 24a 26b	ast: L1 R3	Acce # U3	ss 1 1	381 829	396 863	179 179	0.80	**** ****	0.006

North: CC North 7b L3 # 1 0 0 5 0.98 375 0.206 8 T1 # 381 0 0 1848 0.98 375 0.206

- * Maximum degree of saturation
- $\mbox{\#}$ Combined Movement Capacity parameters are shown for all Movement Classes.

MOVEMENT PERFORMANCE

Mov ID		_	Total Delay (pers-h/h	Aver. Delay)(sec)	Eff. Stop Rate	Total Stops	Tot.Trav. Distance (veh-km/h)	Time	Speed
South 2 3a		0.01	0.02 0.44	0.1 15.4	0.04		19225.4 316.1	240.4	80.0 79.2
24a	L1	Access 0.00 0.00	0.00 0.12		0.43	0.5 0.5	 	0.4 0.2	78.1 75.8
North 7b 8	CC L3 T1	North 0.01 0.07	0.01	21.5	0.01		14.8 16770.1	0.6 209.8	25.4 79.9

Go to Table Links (Top)

Fuel Consumption, Emissions and Cost Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov Turn ID		Total	Total	CO Total kg/h	Total	Total
	8490.38 195.68					8.008 0.135
	8686.06	1550.2	3707.4	7.13	0.361	8.143
NorthEast: Acces 24a L1 26b R3	23.59			0.01		
	38.73	5.3	13.4	0.02	0.002	0.042
	8.12 3567.06					
	3575.17	1325.1	3197.7	6.13	0.302	6.946
INTERSECTION:	12299.97	2880.6	6918.5	13.28	0.665	15.132

FUEL CONSUMPTION, EMISSIONS AND COST (RATE)

								-
Mov	Turn	Cost	Fuel	CO2	CO	HC	NOX	
ID		Rate	Rate	Rate	Rate	Rate	Rate	
		\$/km	$L/100\mathrm{km}$	g/km	g/km	g/km	g/km	

South: CC South

2 T1 3a R1		7.9 8.0		0.36 0.37	0.018 0.019	
	0.44	7.9	189.7	0.36	0.018	0.417
NorthEast: Access						
24a L1 26b R3	0.75 1.03		154.6 574.8			
	0.84	11.4	288.4	0.42	0.041	0.907
North: CC North						
7b L3 8 T1		7.9 7.9	187.2 190.5			
	0.21	7.9	190.5	0.37	0.018	0.414
INTERSECTION:	0.34	7.9	190.2	0.37	0.018	0.416

Go to Table Links (Top)

Lanes

Lane Performance and Capacity Information Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

LANE PERFORMANCE

Lane	Flow veh/h	Cap veh/h	Deg. Satn x	Aver. Delay sec	Stop	Q u e 95% Ba veh	ck	Lane Length m
South: C			0.247	0.5	0.04	0.2	1.1	30000.0
NorthEas			0.006	7.4	0.43	0.0	0.2	20.0
North: C		-	0.206	0.7	0.01			14000.0

LANE FLOW AND CAPACITY INFORMATION

	Total Arv Flow veh/h	Cap	Cap	Satn	Util
South:	CC South	225	1810	0.247	100
NorthE	ast: Acces		358	0.006	100
	CC North 382	382	1853	0.206	100

The capacity values of Continuous Lanes are obtained by adjusting the basic saturation flow for lane width, grade, movement class and turning vehicle effects. Saturation flow scale applies if specified.

Go to Table Links (Top)

Lane, Approach and Intersection Performance Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

No.	Flow	%HV	Basic	Sat	Delay	Longest Queue m	Length
South:	CC South						
1	447	9		0.247		1	***
	447	9		0.247	0.5		
NorthE	ast: Acce	ss					
1	2	50		0.006	7.4	0	20
	2	50		0.006	7.4	0	
North:	CC North						
1	382	8	1950	0.206	0.7		***
	382	8		0.206	0.7		
ALL VE	====== HICLES						
	Total				Aver.		
	Flow				Delay		
======	832 ======	8 =====		U.Z4/ ======	0.6	 	

Peak flow period = 15 minutes.

Queue values in this table are 95% queue (metres)

Note: Basic Saturation Flows at roundabouts or sign-controlled intersections apply only to continuous lanes.

Go to Table Links (Top)

Driver Characteristics

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

Lane No.	_	Satn Flow veh/h	_	Satn Spacing m	~	Driver Response Time sec
South:	CC Sout	h jor Road	Moveme	nt		
NorthEa	ast: Acc		3.20	4.44	10.00	-4.00
North:	CC Nort	h ntinuous	Moveme	nt		

Saturation Flow and Saturation Headway are derived from follow-up headway.

Go to Table Links (Top)

Lane Delays

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

LANE DELAYS

No.	Satn x	During Green	Prog. Factor	Min Del dm	Stop 1st d1	-line 2nd d2	Delay Total dSL	Acc. Dec. dn	Quei Total dq	ing MvUp dqm	(Idle) di	Geom dig	Control dic
South:	CC Sou 0.247	th NA	NA	0.1	0.2	0.0	0.2	0.2	0.0	0.0	0.0	0.3	0.5
NorthE	ast: Ac 0.006	cess NA	NA	6.8	7.4	0.0	7.4	0.7	6.7	0.0	6.7	0.0	7.4
North:	CC Nor					0.6							
1 0.206 0.6 0.1 0.7 SIDRA Standard Delay Model is used. Control Delay is the sum of Stop-line Delay and Geometric Delay. dm: Minimum delay for gap acceptance cases dSL: Stop-line delay (=d1+d2) dn: Average stop-start delay for all vehicles queued and unqueued dq: Queuing delay (the part of the stop-line delay that includes stopped delay and queue move-up delay) dqm: Queue move-up delay di: Stopped delay (stopped (idling) time at near-zero speed) dig: Geometric delay													

Go to Table Links (Top)

Lane Queues

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

BACK OF QUEUE (VEHICLES)

No. x Green South: CC South	ctor Queue No	Nb1	Nb2	Nb 	95%		5io 95% 	Block	%
	ND 0 0								
1 0.247 NA N	N A A								
	1411	0.1	0.0	0.1	0.2	0.00	0.00	0.0	NA
NorthEast: Access									
1 0.006 NA 1	NA 0.0	0.0	0.0	0.0	0.0	0.00	0.01	0.0	NA

BACK OF QUEUE (DISTANCE)

Lane	Deg. Satn	% Arv During	Prog. Factor	Ovrfl.		~	ueue (m)		Queue Rat	Stor.	Prob. Block	
No.	Х	Green		No	Nb1	Nb2	Nb	95%	Av.	95%	%	8
	: CC S	outh NA	NA	0.0	0.5	0.0	0.5	1.1	0.00	0.00	0.0	NA
	East: 0.006	Access NA	NA 	0.0	0.1	0.0	0.1	0.2	0.00	0.01	0.0	NA

North: CC North

OTHER QUEUE RESULTS (VEHICLES)

Lane	_	% Arv During	_	Queue		
No.	Х	Green		No	Nc	95%
	: CC S		NA	0.0	0.0	0.0
		Access NA	NA	0.0	0.0	0.0
North	: CC N	orth				

OTHER QUEUE RESULTS (DISTANCE)

Lane	_	During	Prog. Factor		Cyc-Av.	
	: CC S		NA	0.0	0.2	0.3
	East: 0.006	Access NA	NA	0.0	0.0	0.1
North	: CC N	orth				

Go to Table Links (Top)

Lane Queue Percentiles

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

LANE QUEUE PERCENTILES (VEHICLES)

	Deg.		Perce	ntile B	ack of	 Queue	(veh)	
No.	Satn x	50%	70%	85%	90%	95%	98%	100%
	: CC Sou		0.1	0.1	0.1	0.2	0.2	0.2
North 1	East: Ac		0.0	0.0	0.0	0.0	0.0	0.0
North	: CC Nor	th						

LANE QUEUE PERCENTILES (DISTANCE)

T _i ane	Deg. Satn		Perce	ntile	Back of	Queue	(metres)	
No.	X	50%	70%	85%	90%	95%	98%	100%
	CC So	uth 0.5	0.6	0.8	1.0	1.1	1.3	1.3

NorthEast: Access

1 0.006 0.1 0.1 0.1 0.2 0.2 0.2 0.2

North: CC North

Go to Table Links (Top)

Lane Stops

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

Lane No.	Deg. Satn x	% Arv During Green	Prog. Factor	he1	he2	Geom. hig	h	Н	Queue Move-up Rate hqm	Total Queue Move-ups Hqm	Prop. Queued pq	Aver. Num. of Cycles to Depart
	: CC So											
1	0.247	NA	NA		0.00				0.00	0.0	0.04	0.04
North	East: A	ccess										
1	0.006	NA	NA	0.43	0.00	0.00	0.43	0.9	0.00	0.0	0.59	0.59
North	: CC No	rth										
1	0.206	NA	NA			0.01	0.01	3.1				

hig is the average value for all movements in a shared lane

hqm is average queue move-up rate for all vehicles queued and unqueued

Go to Table Links (Top)

Flow Rates

Origin-Destination Flow Rates (Total)
Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

 ${\tt Give-Way \ Sign \ Controlled \ Intersection}$

TOTAL FLOW RATES for All Movement Classes (veh/h)

From SOUTH To: Turn: Flow Rate %HV (all designations)	N T1 436.8 8.0	NE R1 10.5 30.0	TOT 447.4 8.5
From NORTHEAST To: Turn: Flow Rate %HV (all designations)	S L1 1.1 0.0	N R3 1.1 100.0	TOT 2.1 50.0
From NORTH To: Turn: Flow Rate %HV (all designations)	NE L3 1.1 0.0	S T1 381.1 8.0	TOT 382.1 8.0

Flow rates shown above are Arrival Flow Rates (veh/h) based on the following input specifications: Unit Time for Volumes = 60 minutes

Peak Flow Period = 15 minutes

Effects of Volume Factors (Peak Flow Factor, Flow Scale, Growth Rate) are included. Arrival Flow Rates may be less than Demand Flow Rates if capacity constraint applies in network analysis.

Go to Table Links (Top)

Origin-Destination Flow Rates by Movement Class Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

FLOW RATES for Light Vehicles (veh/h)

From SOUTH To:	N	NE	TOT
Turn:	T1	R1	
Flow Rate	401.9	7.4	409.3
Mov Class %	92.0	70.0	91.5
Flow Scale	1.00	1.00	-
Peak Flow Factor	0.95	0.95	-
Residual Demand	0.0	0.0	0.0
From NORTHEAST To:	S	N	TOT
Turn:	L1	R3	
Flow Rate	1.1	0.0	1.1
Mov Class %	100.0	0.0	50.0
Flow Scale	1.00	1.00	-
Peak Flow Factor	0.95	0.95	-
Residual Demand	0.0	0.0	0.0
From NORTH To:	NE	S	TOT
Turn:	L3	T1	
Flow Rate	1.1	350.6	351.6
Mov Class %	100.0	92.0	92.0
Flow Scale	1.00	1.00	-
Peak Flow Factor	0.95	0.95	-
Residual Demand	0.0	0.0	0.0

FLOW RATES for Heavy Vehicles (veh/h)

From SOUTH To:	N	NE	TOT
Turn:	T1	R1	
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand	34.9 8.0 1.00 0.95 0.0	0.0 0.0 1.00 0.95 0.0	34.9 7.8 - 0.0
From NORTHEAST To:	S	N	TOT
Turn:	L1	R3	
Flow Rate	0.0	0.0	0.0
Mov Class %	0.0	0.0	0.0
Flow Scale	1.00	1.00	-
Peak Flow Factor	0.95	0.95	-
Residual Demand	0.0	0.0	0.0
From NORTH To:	NE	S	TOT
Turn:	L3	T1	
Flow Rate	0.0	30.5	30.5
Mov Class %	0.0	8.0	
Flow Scale	1.00	1.00	
Peak Flow Factor	0.95	0.95	
Residual Demand	0.0	0.0	

FLOW RATES for User Class 3 (veh/h) $\,$

From SOUTH To:	N	NE	TOT
Turn:	T1	R1	
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand	0.0 0.0 1.00 0.95 0.0	3.2 30.0 1.00 0.95 0.0	3.2 0.7 - 0.0
From NORTHEAST To:	S	N	TOT
Turn:	L1	R3	
Flow Rate	0.0	100.0	1.1
Mov Class %	0.0		50.0
Flow Scale	1.00		-
Peak Flow Factor	0.95		-
Residual Demand	0.0		0.0
From NORTH To:	NE	S	TOT
Turn:	L3	T1	
Flow Rate	0.0	0.0	0.0
Mov Class %	0.0	0.0	
Flow Scale	1.00	1.00	
Peak Flow Factor	0.95	0.95	
Residual Demand	0.0	0.0	

Flow rates shown above are Arrival Flow Rates (veh/h) based on the following input specifications: Unit Time for Volumes = 60 minutes Peak Flow Period = 15 minutes Effects of Volume Factors (Peak Flow Factor, Flow Scale, Growth Rate) are included. Arrival Flow Rates may be less than Demand Flow Rates if capacity constraint applies in

network analysis.

Go to Table Links (Top)

Lane Flow Rates

Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

LANE FLOW RATES AT STOP LINE (veh/h)

From SOUTH To: Turn:	N T1	NE R1	TOT
Lane 1 LV HV U3 Total	401.9 34.9 *	7.4 * 3.2 10.5	409.3 34.9 3.2 447.4
Approach	436.8	10.5	447.4
From NORTHEAST Turn:	To: S L1	N R3	TOT
Lane 1 LV U3 Total	1.1 * 1.1	1.1	1.1 1.1 2.1
Approach	1.1	1.1	2.1
From NORTH To: Turn:	NE L3	S T1	TOT
Lane 1 LV	1.1	350.6	351.6

HV	*	30.5	30.5
Total	1.1	381.1	382.1
Approach	1.1	381.1	382.1

* Movement not allocated to the lane

EXIT LANE FLOW RATES

Movemen	nt Class:	LV	HV	U3	TOT
	SOUTH 1	351.6 351.6	30.5 30.5	* *	382.1 382.1
	NORTHEAST 1	8.4 8.4	*	3.2 3.2	11.6
	NORTH 1	401.9 401.9	34.9 34.9	1.1	437.9 437.9

^{*} Movement not allocated to the lane

DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV	U3	TOT
Exit: SOUTH Lane: 1 Total	351.6 351.6	30.5 30.5	* *	382.1 382.1
Exit: NORTHEAST Lane: 1 Total	8.4	*	3.2 3.2	11.6 11.6
Exit: NORTH Lane: 1 Total	401.9 401.9	34.9 34.9	1.1 1.1	437.9 437.9

^{*} Movement not allocated to the lane

Flow rates shown above are Arrival Flow Rates (veh/h) based on the following input specifications: Unit Time for Volumes = 60 minutes

Peak Flow Period = 15 minutes

Effects of Volume Factors (Peak Flow Factor, Flow Scale, Growth Rate) are included.

Arrival Flow Rates may be less than Demand Flow Rates if capacity constraint applies in network analysis.

Go to Table Links (Top)

Other

Parameter Settings Summary Site: Captain Cook Highway 2031 10AM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

* Basic Parameters:

Intersection Type: Unsignalised - Give Way Driving on the left-hand side of the road Input data specified in Metric units Model Defaults: Standard Left Peak Flow Period (for performance): 15 minutes Unit time (for volumes): 60 minutes.

```
SIDRA Standard Delay model used
SIDRA Standard Queue model used
Level of Service based on: Delay (SIDRA)
Queue percentile: 95%
```

Go to Table Links (Top)

Diagnostics

Site: Captain Cook Highway 2031 10AM + Arrive South

```
Site ID: 101
Give-Way Sign Controlled Intersection

Lane Flow-Capacity Iterations:

Site Model Variability Index (Iterations 3 to N): 0.0%
Number of Iterations: 3 (Maximum: 10)

Other Diagnostic Messages (if any):
```

Go to Table Links (Top)

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com

Processed: Thursday, 25 February 2021 9:59:49 AM

Project: Not Saved

DETAILED OUTPUT

Site: 101 [Captain Cook Highway 2031 2PM + Arrive South]

New Site

Site Category: (None) Giveway / Yield (Two-Way)

OUTPUT TABLE LINKS



Sign Control Basic Parameters Gap Acceptance Parameters

Movements

Intersection Negotiation and Travel Data

Movement Capacity and Performance Parameters

Fuel Consumption, Emissions and Cost

Lanes

Lane Performance and Capacity Information

Lane, Approach and Intersection Performance

Driver Characteristics

Lane Delays

Lane Queues

Lane Queue Percentiles

Lane Stops

Îr Flow Rates

Origin-Destination Flow Rates (Total)

Origin-Destination Flow Rates by Movement Class

Lane Flow Rates

=Other

Parameter Settings Summary

Diagnostics

Sign Control

Sign Control Basic Parameters

Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

Approach Control	Leg Geometry	App Dist m	Prop Queued Upstr Signal	Extra Bunching %
South: CC So Major Road		30000	NA	0.0n
NorthEast: A		20	NA	0.0N
North: CC No Major Road		14000	NA	0.0n

NA Not Applicable (single Site analysis or unconnected Site in Network analysis).

Go to Table Links (Top)

N Program option resulted in zero value (single Site analysis or unconnected Site in Network analysis).

Gap Acceptance Parameters Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

Opd Lane	Dest	Opng Flow pcu/h	Intra Bunch Hdwy sec	Propn Bnchd	Entry HV Equiv	Critica Hdwy sec	-	Foll-up Headway sec
South: (CC South NE	474	1.80	0.058	1.15	4.60	102.0	2.30
NorthEas 1 1	st: Acces: S N	s 473+ 775+	1.80 1.05	0.058 0.055	1.00	4.00 4.30	88.9 94.3	2.20

North: CC North

No opposed movements on this approach.

Values in this table are adjusted for movement classes in the entry stream. Use the Pedestrians and Priorities input dialogs to specify opposing pedestrian movements.

+ Percentage of exiting flow included in opposing vehicle flow

Go to Table Links (Top)

Movements

Intersection Negotiation and Travel Data Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

TRAVEL SPEED, TRAVEL DISTANCE AND TRAVEL TIME

From To Approach Exit	Turn	,	,	Travel Distance m	s	Dem Flows	el Distance Arv Flows veh-km/h	Tot.Trav. Time veh-h/h
South: CC South North NorthEast	T1 R1	80.0 79.2	80.0 79.2	44010.0# 30030.0#	1981.1#	12230.2 316.1	12230.2 316.1	152.9 4.0
NorthEast: Access South North	L1 R3	78.5 76.9	78.4 76.6	30030.9# 14030.9#		31.6 14.8	31.6 14.8	0.4
North: CC North NorthEast South	L3 T1	25.4 79.9	25.4 79.9	14030.0# 44010.0#		14.8 20013.0	14.8 20013.0	
ALL VEHICLES:		79.8	79.8	43708.6#	1970.8#	32620.4	32620.4	408.6

[&]quot;Running Speed" is the average speed excluding stopped periods.

Travel Time values include cruise times and intersection delays including acceleration, deceleration and idling delays.

Travel Distance and Travel Time values include travel on the External Exit section based on the Exit Distance or user-specified Downstream Distance value as applicable.

INTERSECTION NEGOTIATION DATA

From To Approach Exit	Turn	Negn Radius m	Negn Speed km/h	Dist	App Dist m	Exit Dist m	Downstr Dist m
South: CC South North NorthEast		s 13.4	80.0 5.0		30000 30000	14000	NA NA
NorthEast: Access South North	ı L1	7.0 5.0		10.0	20 20	30000 14000	NA NA
North: CC North NorthEast South		10.0 S	5.0 80.0	23.6	14000	20	NA NA

NA Downstream Distance does not apply if:
- Exit is an internal leg of a network

- "Program" option was specified
- Distance specified was less than the Exit Negotiation Distance Distance specified was greater than the exit leg length

Some Negotiation Radius, Speed or Distance values are user specified.

MOVEMENT SPEEDS AND GEOMETRIC DELAY

Mov ID	Turn	App. Sp Cruise km/h	 Negn	 Negn	Cruise	Queue Move-up Speed km/h	Delay
2		80.0				0.5 0.5	
24a	L1					5.0 5.0	
		80.0 80.0					20.8

Go to Table Links (Top)

Movement Capacity and Performance Parameters Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

MOVEMENT CAPACITY PARAMETERS

Mov	Turn	Mov Cl.	Arv Flow	Flow		-	Prac. Deg. Satn	Prac. Spare Cap.	Deg. Satn
			veh/h	veh/h	pcu/h	ven/n	хр	용	X
South:	: CC :	south							
2	Т1	#	278	0	0	1709	0.98	503	0.163
3a	R1	#	11 	456 	474	65 	0.98	503	0.163
North	East:	Acce	ss						
24a	L1	#	1	455	473	371	0.80	****	0.003
26b	R3	#	1	744	775	371	0.80	****	0.003

North: CC North 7b L3 # 1 0 0 4 0.98 298 0.246* 8 T1 # 455 0 0 1849 0.98 298 0.246*

- * Maximum degree of saturation
- $\mbox{\#}$ Combined Movement Capacity parameters are shown for all Movement Classes.

MOVEMENT PERFORMANCE

Mov ID		Total Delay reh-h/h)	Total Delay (pers-h/h	Aver. Delay	Eff. Stop Rate		Tot.Trav. Distance (veh-km/h)		Speed
South 2 3a		0.02 0.05	0.02	0.2 15.8	0.06	153.11 4.13	12230.2 316.1	152.9 4.0	80.0 79.2
	L1	Access 0.00 0.00	0.00		0.31	 6.33	31.6 14.8	0.4 0.2	78.4 76.6
North 7b 8		0.01 0.10	0.01	21.6	0.01	 0.19	14.8 20013.0	0.6 250.5	25.4 79.9

Go to Table Links (Top)

Fuel Consumption, Emissions and Cost Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

FUEL CONSUMPTION, EMISSIONS AND COST (TOTAL)

Mov ID	Turn		Total		CO Total kg/h		
2		5732.40 207.70					5.112 0.137
		5940.10	997.7	2386.3	4.57	0.235	5.249
NorthE 24a 26b		2.17		4.7	0.01		
		3.48	3.0	6.9	0.02	0.001	0.002
	CC North L3 T1	8.12 4257.50					0.006 8.282
		4265.62	1581.0	3815.2	7.31	0.361	8.288
INTER	SECTION:	10209.21	2581.7	6208.5	11.90	0.596	13.539

FUEL	CONSUMPTION,	EMISSIONS	AND	COST	(RATE)

								•
Mov	Turn	Cost	Fuel	CO2	CO	HC	NOX	
ID		Rate	Rate	Rate	Rate	Rate	Rate	
		\$/km	$L/100\mathrm{km}$	g/km	g/km	g/km	g/km	

South: CC South

2 T1 3a R1	0.47 0.66					
	0.47	8.0	190.2	0.36	0.019	0.418
NorthEast: Access 24a L1 26b R3	0.07				0.017 0.018	
	0.08	6.4	149.8	0.38	0.017	0.044
North: CC North 7b L3 8 T1	0.55 0.21	7.9 7.9				
	0.21	7.9	190.5	0.36	0.018	0.414
INTERSECTION:	0.31	7.9	190.3	0.36	0.018	0.415

Go to Table Links (Top)

Lanes

Lane Performance and Capacity Information Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

LANE PERFORMANCE

Lane	Flow veh/h	Cap veh/h	Deg. Satn x	Aver. Delay sec	Stop	Q u e 95% Ba veh	ck	
South: C			0.163	0.8	0.06	0.2	1.2	30000.0
NorthEas 1			0.003	2.7	0.31	0.0	0.1	20.0
North: C		-	0.246	0.8	0.01			14000.0

LANE FLOW AND CAPACITY INFORMATION

	Total Arv Flow veh/h	Cap	Cap	Satn	Util
South:	CC South 288	151	1774	0.163	100
NorthEa	ast: Acces		742	0.003	100
North:	CC North 456	456	1853	0.246	100

The capacity values of Continuous Lanes are obtained by adjusting the basic saturation flow for lane width, grade, movement class and turning vehicle effects. Saturation flow scale applies if specified.

Go to Table Links (Top)

Lane, Approach and Intersection Performance Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

No.	Flow	%HV	Basic	Sat	Delay	Longest Queue m	Length
	CC South	9			0.8	1	***
	288	9			0.8		
NorthE 1	ast: Acces	ss O		0.003	2.7	0	20
	2	0		0.003	2.7	0	
	CC North		1950	0.246	0.8		****
	456	8		0.246	0.8		
ALL VE	====== HICLES						
	Total Flow 746			X	Aver. Delay 0.8	Queue	

Peak flow period = 15 minutes.

Queue values in this table are 95% queue (metres)

Note: Basic Saturation Flows at roundabouts or sign-controlled intersections apply only to continuous lanes.

Go to Table Links (Top)

Driver Characteristics

Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

Lane No.	-	Satn Flow veh/h	-	Satn Spacing m	~	Response
South:	CC Sout NA - Ma	h jor Road	Moveme	ent		
NorthE	ast: Acc 5.0		2.50	3.47	7.00	-2.54
North:	CC Nort NA - Co	h ntinuous 	Moveme	ent		

Saturation Flow and Saturation Headway are derived from follow-up headway.

Go to Table Links (Top)

Lane Delays

Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

LANE DELAYS

	Satn	During	Prog. Factor	Min Del	Stop 1st	-line 2nd	Delay Total	Acc. Dec.	Quei Tota	uing L MvUp	-	Geom	Control
1			NA	0.2	0.3	0.0	0.3	0.3	0.0	0.0	0.0	0.5	0.8
NorthE	ast: Ac	cess	NA	2.5	2.7	0.0	2.7	0.6	2.1	0.0	2.1	0.0	2.7
	CC Nor					0.8					0.0	0.8	
and Gdm: MdSL: Gdn: Adq: Qdq: Qddi: Sddi: Sddig: Gdig:	eometri inimum Stop-li verage ueuing topped Queue m topped	c Delay. delay fo ne delay stop-sta delay (t delay an ove-up d delay (s ic delay	r gap acc (=d1+d2) rt delay he part o d queue r elay topped (2	ceptand for all of the nove-up	ce case l veh: stop- delag	es icles line (queued delay t	l and u hat in	inqueue icludes	ed	-line D	elay	

Go to Table Links (Top)

Lane Queues

Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

BACK OF QUEUE (VEHICLES)

	Deg. Satn	% Arv During	Prog. Factor	Ovrfl. Oueue		_	ueue (v	,	~	Stor.	Prob. Block	
No.	Х	Green	140001	No	Nb1	Nb2	Nb	95%		95%	8	8
South:	cc s	outh										
1 0	.163	NA	NA	0.0	0.1	0.0	0.1	0.2	0.00	0.00	0.0	NA
NorthE	Tast:	Access										
1 0	0.003	NA	NA	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA

BACK OF QUEUE (DISTANCE)

Lane	Deg. Satn	% Arv During	Prog. Factor	Ovrfl.		~	ueue (m)		Queue Rat		Prob. Block	
No.	Х	Green		No	Nb1	Nb2	Nb	95%	Av.	95%	%	8
	: CC S	outh NA	NA	0.0	0.5	0.0	0.5	1.2	0.00	0.00	0.0	NA
	East: 0.003	Access NA	NA	0.0	0.0	0.0	0.0	0.1	0.00	0.00	0.0	NA

North: CC North

OTHER QUEUE RESULTS (VEHICLES)

Lane	_	During	_	Queue	Cyc-Av.	
	: CC S		NA	0.0	0.0	0.0
	East: 0.003	Access NA	NA	0.0	0.0	0.0
North	: CC N	orth				

OTHER QUEUE RESULTS (DISTANCE)

	_	During	Prog. Factor		Cyc-Av.	Queue 95%
	: CC S		NA	0.0	0.2	0.3
	East: 0.003		NA	0.0	0.0	0.0
North	: CC N	orth				

Go to Table Links (Top)

Lane Queue Percentiles

Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

LANE QUEUE PERCENTILES (VEHICLES)

Lane	Deg. Satn		Perce	ntile B	ack of	Queue	(veh)	
No.	X	50%	70%	85%	90%	95%	98%	100%
	: CC Sou 0.163		0.1	0.1	0.1	0.2	0.2	0.2
	East: Ac 0.003		0.0	0.0	0.0	0.0	0.0	0.0
North	: CC Nor	 th 						

LANE QUEUE PERCENTILES (DISTANCE)

Lane	Deg. Satn		Perce	ntile	Back of	Queue	(metres)	
No.	X	50%	70%	85%	90%	95%	98%	100%
South:	: CC Soi	uth 0.5	0.6	0.9	1.0	1.2	1.3	1.4

NorthEast: Access

1 0.003 0.0 0.0 0.0 0.1 0.1 0.1 0.1

North: CC North

Go to Table Links (Top)

Lane Stops

Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

Lane No.	Deg. Satn x	During Green	Prog. Factor	he1	he2	Geom. hig	Rate Overall h	Stops H	Queue Move-up Rate hqm	Total Queue Move-ups Hqm	Prop. Queued pq	Aver. Num. of Cycles to Depart
South 1	: CC So	uth NA	NA	0.00	0.00	0.06	0.06	16.7	0.00	0.0	0.06	0.06
	East: A		NA	0.31	0.00	0.00	0.31	0.7	0.00		0.49	0.49
North 1	: CC No	rth NA	NA			0.01	0.01	3.1				

hig is the average value for all movements in a shared lane

hqm is average queue move-up rate for all vehicles queued and unqueued

Go to Table Links (Top)

Flow Rates

Origin-Destination Flow Rates (Total)
Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

 ${\tt Give-Way \ Sign \ Controlled \ Intersection}$

TOTAL FLOW RATES for All Movement Classes (veh/h)

From SOUTH To: Turn: Flow Rate %HV (all designations)	N T1 277.9 8.0	NE R1 10.5 30.0	TOT 288.4 8.8
From NORTHEAST To: Turn: Flow Rate %HV (all designations)	S L1 1.1 0.0	N R3 1.1 0.0	TOT 2.1 0.0
From NORTH To: Turn: Flow Rate %HV (all designations)	NE L3 1.1 0.0	S T1 454.7 8.0	TOT 455.8 8.0

Flow rates shown above are Arrival Flow Rates (veh/h) based on the following input specifications: Unit Time for Volumes = 60 minutes

Peak Flow Period = 15 minutes

Effects of Volume Factors (Peak Flow Factor, Flow Scale, Growth Rate) are included. Arrival Flow Rates may be less than Demand Flow Rates if capacity constraint applies in network analysis.

Go to Table Links (Top)

Origin-Destination Flow Rates by Movement Class Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

FLOW RATES for Light Vehicles (veh/h)

From SOUTH To: Turn:	N T1	NE R1	TOT
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand	255.7 92.0 1.00 0.95 0.0	70.0 1.00	263.0 91.2 - - 0.0
From NORTHEAST To: Turn:	S L1	N R3	TOT
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand	1.1 100.0 1.00 0.95 0.0	100.0	
From NORTH To: Turn:	NE L3	S T1	TOT
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand		418.4 92.0 1.00 0.95 0.0	

FLOW RATES for Heavy Vehicles (veh/h)

From SOUTH To:	N	NE	TOT
Turn:	T1	R1	
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand	22.2 8.0 1.00 0.95 0.0	0.0 0.0 1.00 0.95 0.0	22.2 7.7 - 0.0
From NORTHEAST To:	S	N	TOT
Turn:	L1	R3	
Flow Rate	0.0	0.0	0.0
Mov Class %	0.0	0.0	
Flow Scale	1.00	1.00	
Peak Flow Factor	0.95	0.95	
Residual Demand	0.0	0.0	
From NORTH To:	NE	S	TOT
Turn:	L3	T1	
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand	0.0 0.0 1.00 0.95 0.0	36.4 8.0 1.00 0.95 0.0	36.4 8.0 - 0.0

FLOW RATES for User Class 3 (veh/h)

From SOUTH To:	N	NE	TOT
Turn:	T1	R1	
Flow Rate Mov Class % Flow Scale Peak Flow Factor Residual Demand	0.0 0.0 1.00 0.95 0.0	3.2 30.0 1.00 0.95 0.0	3.2 1.1 - 0.0
From NORTHEAST To:	S	N	TOT
Turn:	L1	R3	
Flow Rate	0.0	0.0	0.0
Mov Class %	0.0	0.0	0.0
Flow Scale	1.00	1.00	-
Peak Flow Factor	0.95	0.95	-
Residual Demand	0.0	0.0	0.0
From NORTH To:	NE	S	TOT
Turn:	L3	T1	
Flow Rate	0.0	0.0	0.0
Mov Class %	0.0	0.0	
Flow Scale	1.00	1.00	
Peak Flow Factor	0.95	0.95	
Residual Demand	0.0	0.0	

Flow rates shown above are Arrival Flow Rates (veh/h) based on the following input specifications: Unit Time for Volumes = 60 minutes

Peak Flow Period = 15 minutes

Effects of Volume Factors (Peak Flow Factor, Flow Scale, Growth Rate) are included.

Arrival Flow Rates may be less than Demand Flow Rates if capacity constraint applies in network analysis.

Go to Table Links (Top)

Lane Flow Rates

Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101

Give-Way Sign Controlled Intersection

LANE FLOW RATES AT STOP LINE (veh/h)

From SOUTH To: Turn:	N T1	NE R1	TOT
Lane 1 LV HV U3 Total	255.7 22.2 * 277.9	7.4 * 3.2 10.5	263.0 22.2 3.2 288.4
Approach	277.9	10.5	288.4
From NORTHEAST To	o: S L1	N R3	TOT
Lane 1 LV Total	1.1	1.1	
Approach	1.1	1.1	2.1
From NORTH To: Turn:	NE L3	S T1	TOT
Lane 1 LV HV	1.1	418.4 36.4	

Total	1.1	454.7	455.8
Approach	1.1	454.7	455.8

* Movement not allocated to the lane

EXIT LANE FLOW RATES

Movemen	nt Class:	LV	HV	U3	TOT
	SOUTH 1	419.4 419.4	36.4 36.4	*	455.8 455.8
Exit: Lane: Total	NORTHEAST 1	8.4 8.4	*	3.2 3.2	11.6 11.6
Exit: Lane: Total	NORTH 1	256.7 256.7	22.2	*	278.9 278.9

^{*} Movement not allocated to the lane

DOWNSTREAM LANE FLOW RATES FOR EXIT ROADS

Movement Class:	LV	HV	U3	TOT
Exit: SOUTH Lane: 1 Total	419.4 419.4		*	455.8 455.8
Exit: NORTHEAST Lane: 1 Total	8.4 8.4	*	3.2 3.2	11.6 11.6
Exit: NORTH Lane: 1 Total	256.7 256.7		*	278.9 278.9

^{*} Movement not allocated to the lane

Flow rates shown above are Arrival Flow Rates (veh/h) based on the following input specifications: Unit Time for Volumes = 60 minutes

Peak Flow Period = 15 minutes

Effects of Volume Factors (Peak Flow Factor, Flow Scale, Growth Rate) are included.

Arrival Flow Rates may be less than Demand Flow Rates if capacity constraint applies in network analysis.

Go to Table Links (Top)

Other

Parameter Settings Summary Site: Captain Cook Highway 2031 2PM + Arrive South

Site ID: 101 Give-Way Sign Controlled Intersection

* Basic Parameters:

Intersection Type: Unsignalised - Give Way Driving on the left-hand side of the road Input data specified in Metric units Model Defaults: Standard Left Peak Flow Period (for performance): 15 minutes Unit time (for volumes): 60 minutes. SIDRA Standard Delay model used

```
SIDRA Standard Queue model used
Level of Service based on: Delay (SIDRA)
Queue percentile: 95%
```

Go to Table Links (Top)

Diagnostics

Site: Captain Cook Highway 2031 2PM + Arrive South

```
Site ID: 101
Give-Way Sign Controlled Intersection

Lane Flow-Capacity Iterations:

Site Model Variability Index (Iterations 3 to N): 0.0%
Number of Iterations: 3 (Maximum: 10)

Other Diagnostic Messages (if any):
```

Go to Table Links (Top)

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com

Processed: Thursday, 25 February 2021 9:59:50 AM

Project: Not Saved